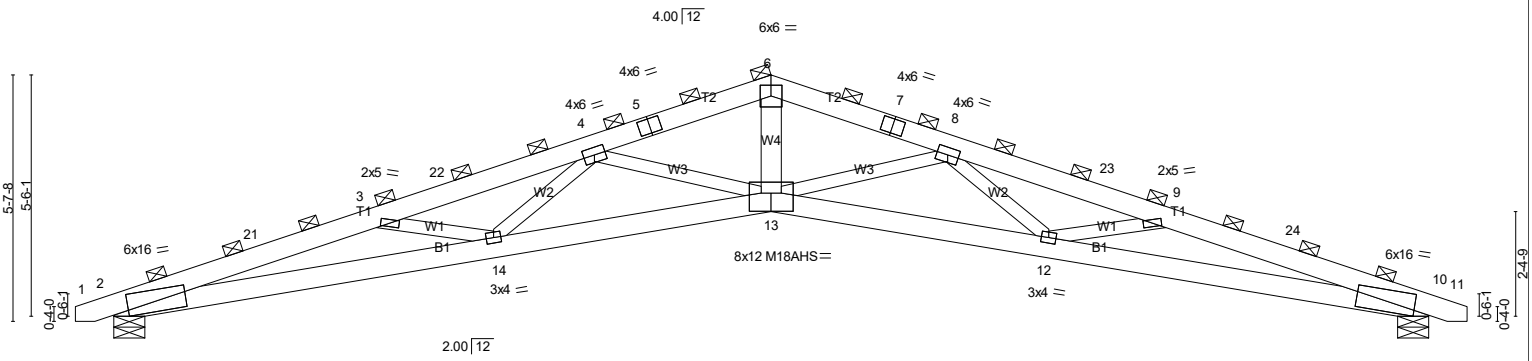
Job	Truss	Truss Type	Qty	Ply		Attachment 08
25080283B	S30	SCISSORS	16	1	Westwood C.C. 400733-012 Job Reference (optional)	

UFP Site Built, LLC, UFP 20/20

Run: 8.830 s Apr 11 2025 Print: 8.830 s Apr 11 2025 MiTek Industries, Inc. Wed Aug 6 12:07:12 2025 Page 1
ID:ZHHB8ESp8faLeAKcYUvvt6z26yE-lE1k8ocvcdHAgb_3Opyej6VfZmJueKeLKfhW0Yyqb?D

-0-10-8	6-3-10	10-11-10	15-0-0	19-0-6	23-8-6	30-0-0	30-10-8
0-10-8	6-3-10	4-8-0	4-0-6	4-0-6	4-8-0	6-3-10	0-10-8

Scale = 1:52.6



8-8-0	15-0-0	21-4-0	30-0-0
8-8-0	6-4-0	6-4-0	8-8-0

Plate Offsets (X,Y)-- [13:0-6-0,0-5-0]

LOADING (psf)		**SPACING-**	4-0-0	**CSI.**		**DEFL.**	in	(loc)	l/defl	L/d	**PLATES**	**GRIP**
TCLL	30.0	Plate Grip DOL	1.15	TC	0.80	Vert(LL)	-0.90	13-14	>399	240	MT20	197/144
TCDL	5.0	Lumber DOL	1.15	BC	0.78	Vert(CT)	-1.17	13-14	>307	180	M18AHS	142/136
BCLL	0.0	Rep Stress Incr	NO	WB	0.78	Horz(CT)	0.57	10	n/a	n/a		
BCDL	5.0	Code IBC2021/TPI2014		Matrix-MS							Weight: 177 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.1 *Except*
T1: 2x6 SP 2400F 2.0E
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SPF No.2 *Except*
W4: 2x6 SP No.1

BRACING-
TOP CHORD 2-0-0 oc purlins (2-6-10 max.).
BOT CHORD Rigid ceiling directly applied or 5-8-14 oc bracing.

REACTIONS. (lb/size) 2=2716/0-8-8, 10=2716/0-8-8
Max Horz 2=-175(LC 15)
Max Uplift2=-764(LC 10), 10=-764(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-21=-10937/3569, 3-21=-10790/3577, 3-22=-10088/3233, 4-22=-10027/3246, 4-5=-7905/2580, 5-6=-7800/2590, 6-7=-7800/2590, 7-8=-7905/2580, 8-23=-10027/3246, 9-23=-10088/3233, 9-24=-10790/3577, 10-24=-10937/3569, 10-11=0/32
BOT CHORD 2-14=-3268/10380, 13-14=-2795/9241, 12-13=-2795/9241, 10-12=-3268/10380
WEBS 6-13=-1333/4353, 3-14=-872/482, 4-14=-149/728, 4-13=-2429/741, 8-13=-2429/748, 8-12=-162/728, 9-12=-872/497

NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=3.0psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-7-13 to 3-4-3, Interior(1) 3-4-3 to 10-11-10, Exterior(2R) 10-11-10 to 19-0-6, Interior(1) 19-0-6 to 26-7-13, Exterior(2E) 26-7-13 to 30-7-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Pf=33.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.20
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 33.6 psf on overhangs non-concurrent with other live loads.
6) Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
7) All plates are MT20 plates unless otherwise indicated.
8) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
9) Bearing at joint(s) 2, 10 considers parallel to grain value using ANSI/TPI 1 angle to grain formula. Building designer should verify capacity of bearing surface.
10) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 764 lb uplift at joint 2 and 764 lb uplift at joint 10.
11) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
12) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Job	Truss	Truss Type	Qty	Ply	
25080283B	T30	COMMON	2	1	Westwood C.C. 400733-012 Job Reference (optional)

UFP Site Built, LLC, UFP 20/20

Run: 8.830 s Apr 11 2025 Print: 8.830 s Apr 11 2025 MiTek Industries, Inc. Wed Aug 6 12:07:13 2025 Page 1
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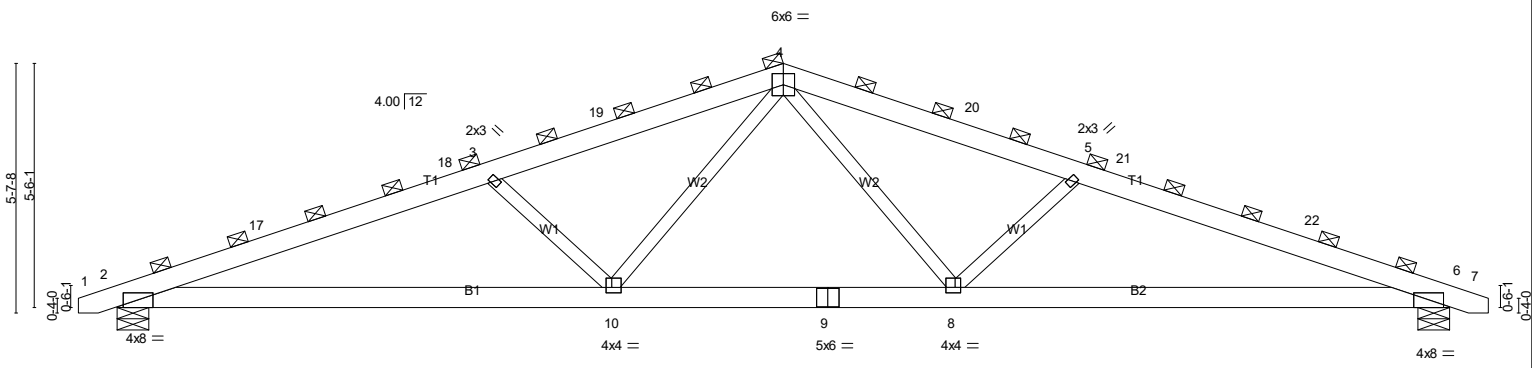
-0-10-8 | 8-6-0 | 15-0-0 | 21-6-0 | 30-0-0 | 30-10-8
0-10-8 | 8-6-0 | 6-6-0 | 6-6-0 | 8-6-0 | 0-10-8

Scale = 1:51.9



11-1-10 | 18-10-6 | 30-0-0
11-1-10 | 7-8-13 | 11-1-10

Plate Offsets (X,Y)-- [2:0-1-10,Edge], [6:0-1-10,Edge], [8:0-1-8,0-1-8], [10:0-1-8,0-1-8]

LOADING (psf)		SPACING-	4-0-0	CSI.		DEFL.	in	(loc)	l/defl	L/d	PLATES	GRIP
TCLL	30.0	Plate Grip DOL	1.15	TC	0.98	Vert(LL)	-0.33	10-13	>999	240	MT20	197/144
TCDL	5.0	Lumber DOL	1.15	BC	0.60	Vert(CT)	-0.46	10-13	>779	180		
BCLL	0.0	Rep Stress Incr	NO	WB	0.46	Horz(CT)	0.11	6	n/a	n/a		
BCDL	5.0	Code IBC2021/TPI2014		Matrix-MS							Weight: 171 lb	FT = 20%

LUMBER-
TOP CHORD 2x6 SP No.1
BOT CHORD 2x6 SP 2400F 2.0E
WEBS 2x4 SPF No.2

BRACING-
TOP CHORD 2-0-0 oc purlins (2-1-0 max.).
BOT CHORD Rigid ceiling directly applied or 7-7-7 oc bracing.

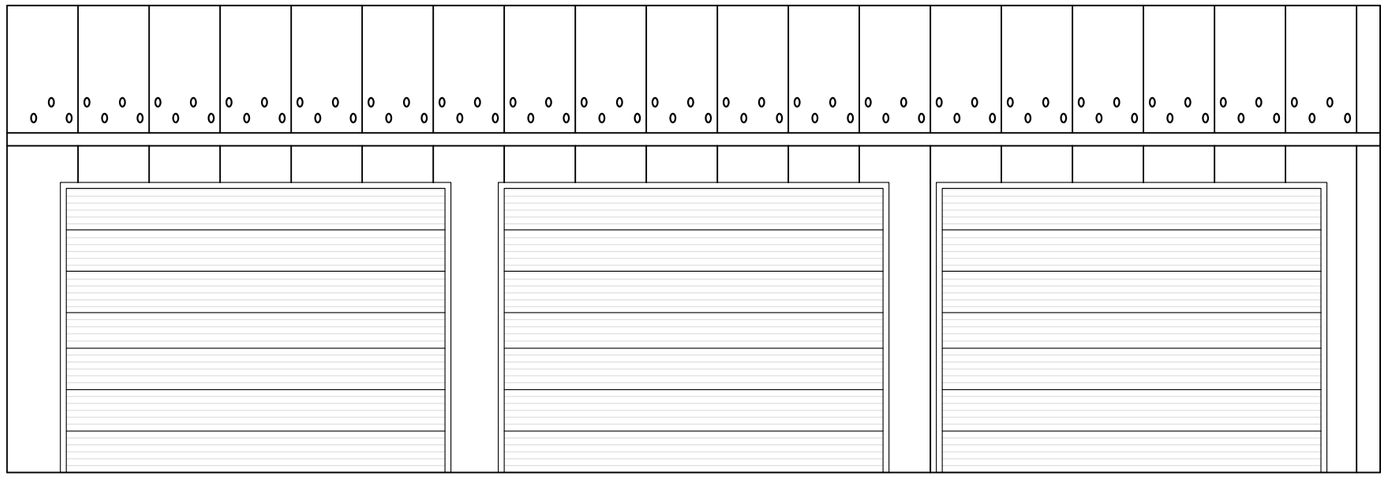
REACTIONS. (lb/size) 2=2716/0-8-8, 6=2716/0-8-8
Max Horz 2=-175(LC 15)
Max Uplift2=-765(LC 10), 6=-765(LC 11)

FORCES. (lb) - Maximum Compression/Maximum Tension
TOP CHORD 1-2=0/32, 2-17=-6201/2085, 17-18=-6101/2088, 3-18=-5928/2104, 3-19=-5299/1831, 4-19=-5114/1853, 4-20=-5114/1853, 5-20=-5299/1830, 5-21=-5928/2104, 21-22=-6101/2088, 6-22=-6201/2085, 6-7=0/32
BOT CHORD 2-10=-1819/5788, 9-10=-1117/3954, 8-9=-1117/3954, 6-8=-1819/5788
WEBS 3-10=-1685/622, 4-10=-453/1798, 4-8=-454/1798, 5-8=-1685/622

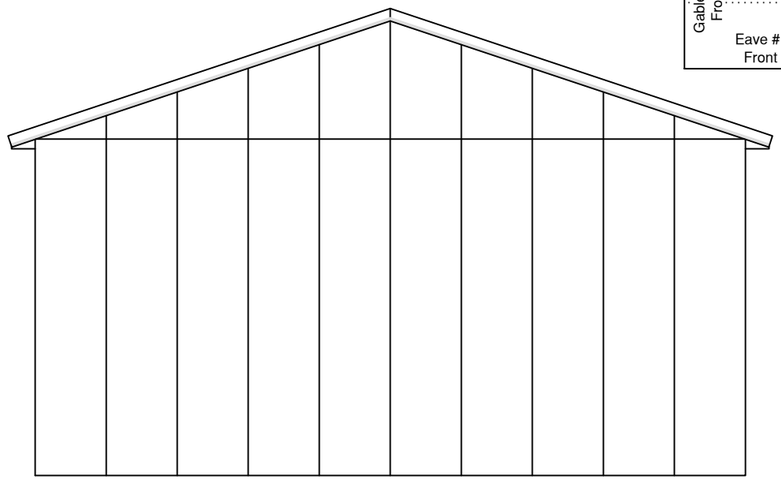
NOTES-
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-16; Vult=115mph (3-second gust) Vasd=91mph; TCDL=3.0psf; BCDL=3.0psf; h=20ft; Cat. II; Exp C; Enclosed; MWFRS (envelope) gable end zone and C-C Exterior(2E) -0-7-13 to 3-4-3, Interior(1) 3-4-3 to 11-0-0, Exterior(2R) 11-0-0 to 19-0-0, Interior(1) 19-0-0 to 26-7-13, Exterior(2E) 26-7-13 to 30-7-13 zone;C-C for members and forces & MWFRS for reactions shown; Lumber DOL=1.60 plate grip DOL=1.60
3) TCLL: ASCE 7-16; Pr=30.0 psf (roof LL: Lum DOL=1.15 Plate DOL=1.15); Pg=40.0 psf; Pf=33.6 psf (Lum DOL = 1.15 Plate DOL = 1.15); Is=1.0; Rough Cat C; Partially Exp.; Ce=1.0; Cs=1.00; Ct=1.20
4) Unbalanced snow loads have been considered for this design.
5) This truss has been designed for greater of min roof live load of 12.0 psf or 1.00 times flat roof load of 33.6 psf on overhangs non-concurrent with other live loads.
6) Dead loads shown include weight of truss. Top chord dead load of 5.0 psf (or less) is not adequate for a shingle roof. Architect to verify adequacy of top chord dead load.
7) This truss has been designed for a 10.0 psf bottom chord live load nonconcurrent with any other live loads.
8) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 765 lb uplift at joint 2 and 765 lb uplift at joint 6.
9) See Standard Industry Piggyback Truss Connection Detail for Connection to base truss as applicable, or consult qualified building designer.
10) Graphical purlin representation does not depict the size or the orientation of the purlin along the top and/or bottom chord.

LOAD CASE(S) Standard

Contract # 400733-012



EAVE #1



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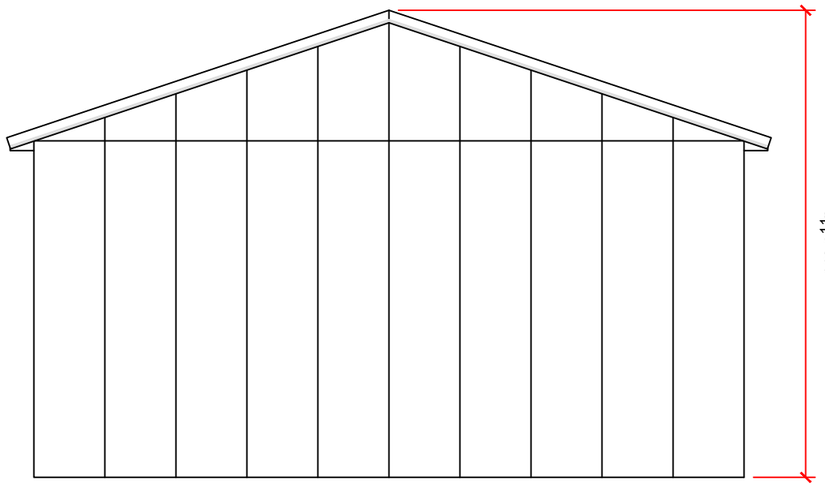

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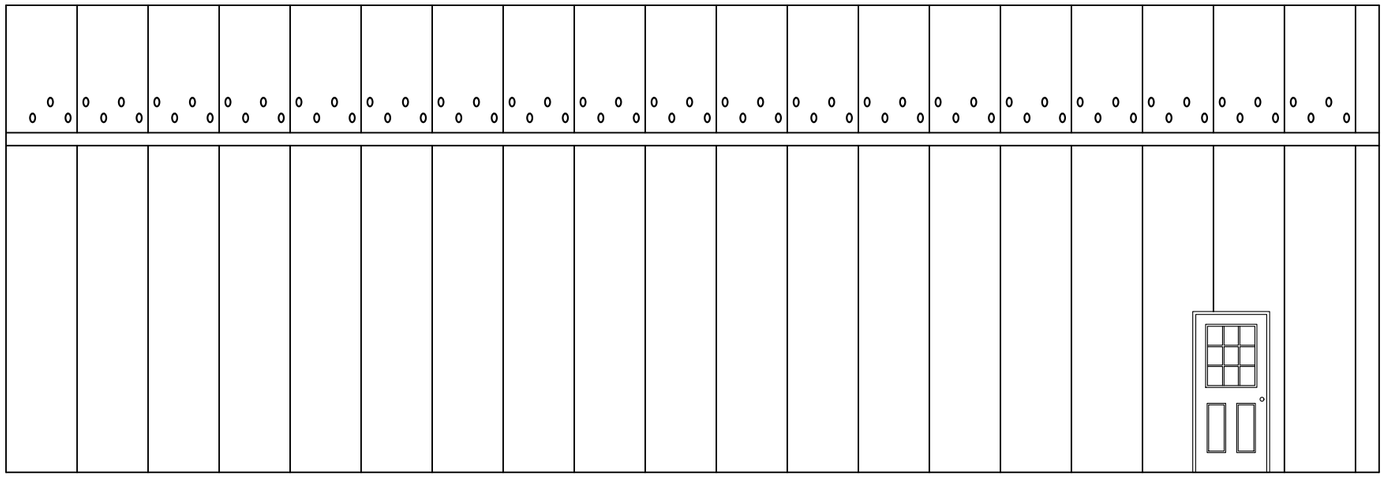

EAVE #2

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: Elevations

James A. Koppenhaver, P.E.
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CUSTOMER ADDRESS:
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800 MAPLE AVE E
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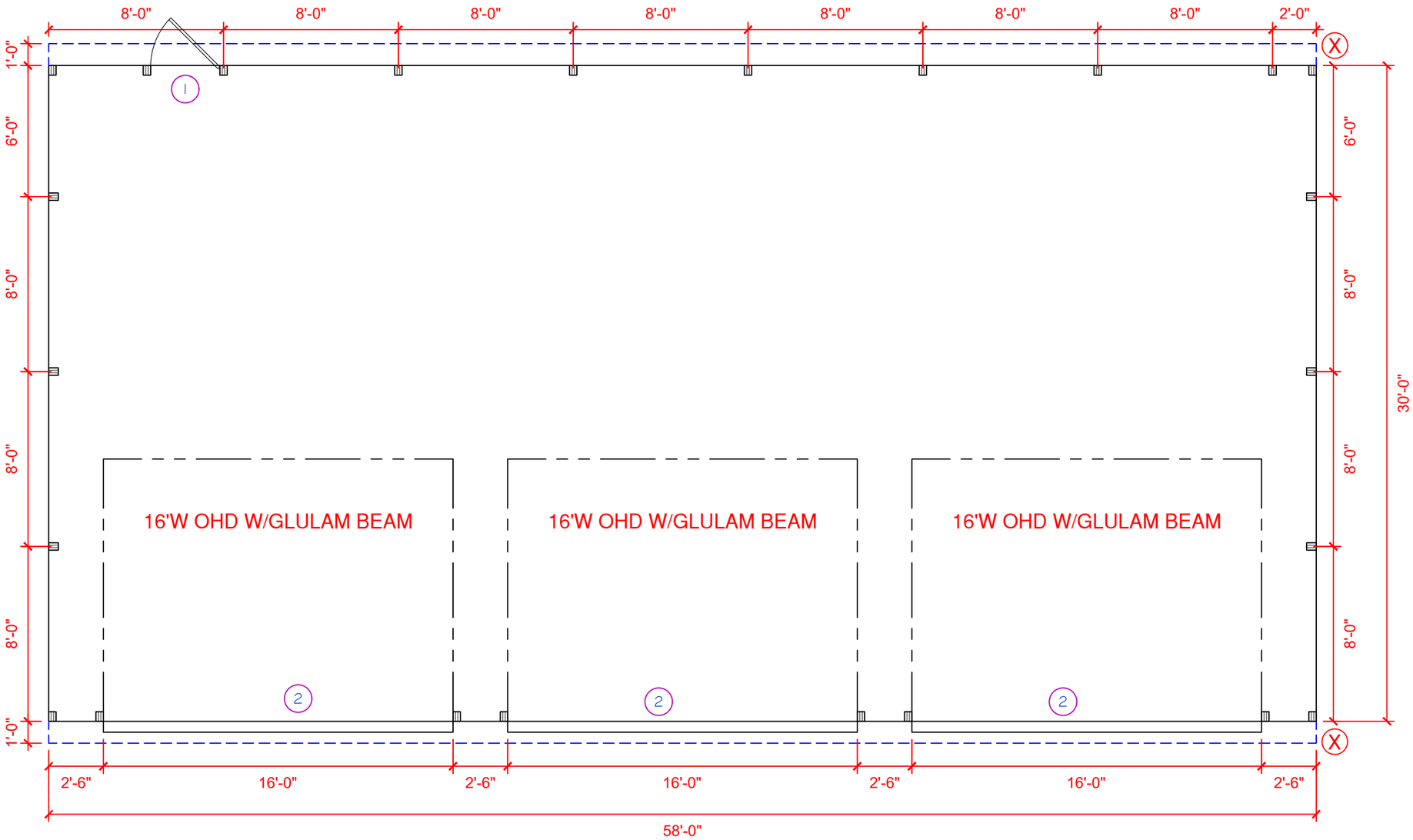
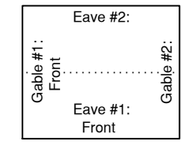
703-755-5363 (H)
954-260-2011 (M)

Contract # 400733-012



RANGE



X = DOWNSPOUT

WINDOW & DOOR SCHEDULE

SYM.	TYPE OF WINDOW / DOOR	NOMIN.	R.O.
1	ENTRY DOOR 9-LITE	3068	38" x 82½"
2	OVERHEAD DOOR	16X12	16'-0" x 12'-4"

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: Pole Plan

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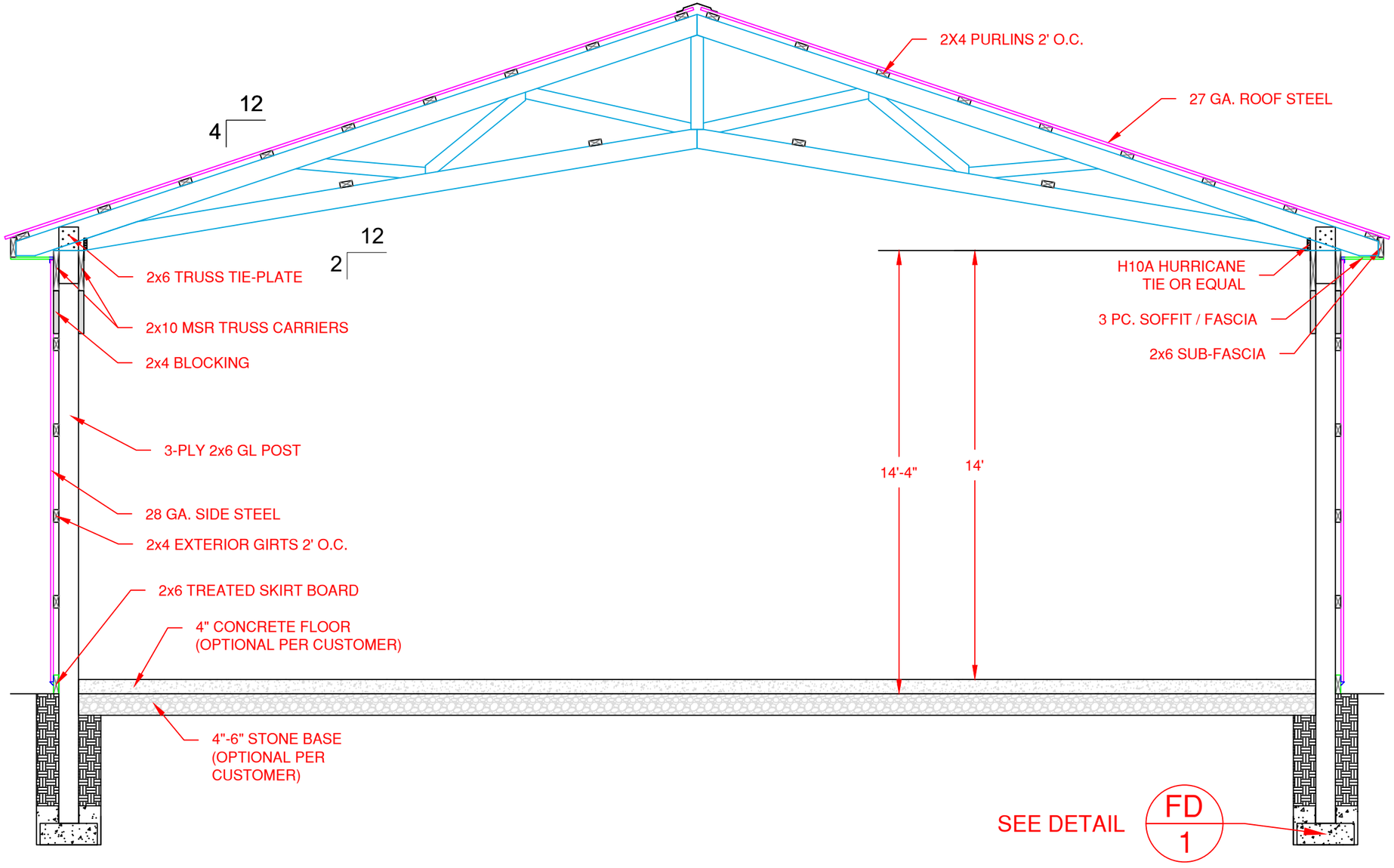
954-260-2011 (M)

Contract # 400733-012



TYP. CROSS SECTION

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: TYP Cross Section

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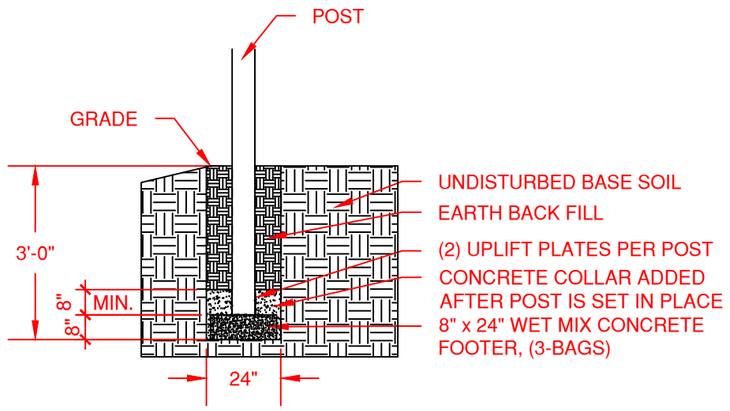
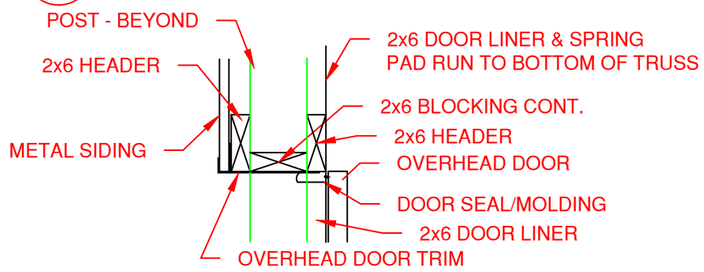
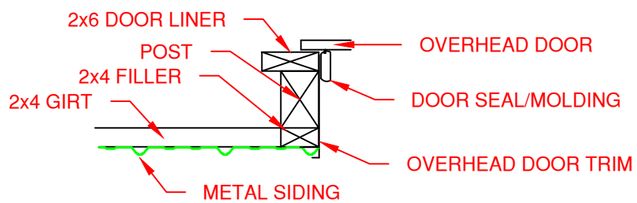
Contract # 400733-012



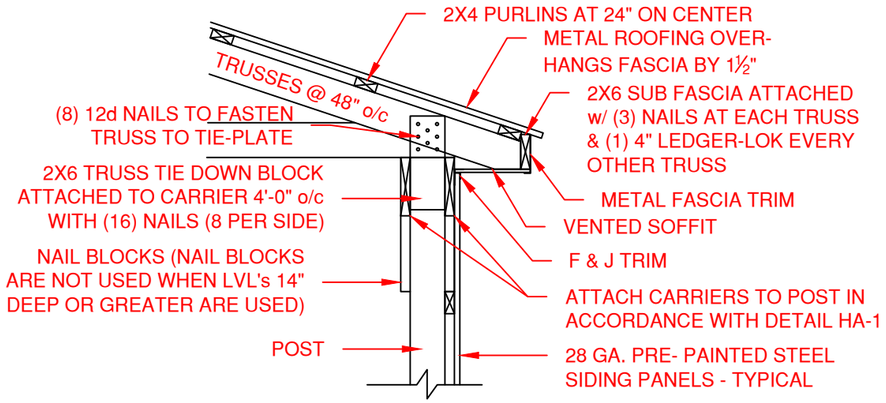
FD 1 FOOTER DETAIL



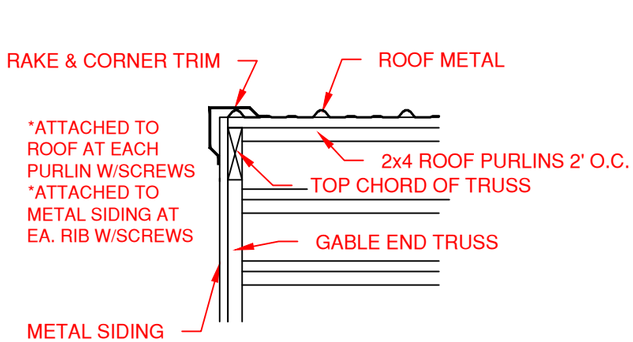
OD 1 O.H. DOOR HEADER DETAIL



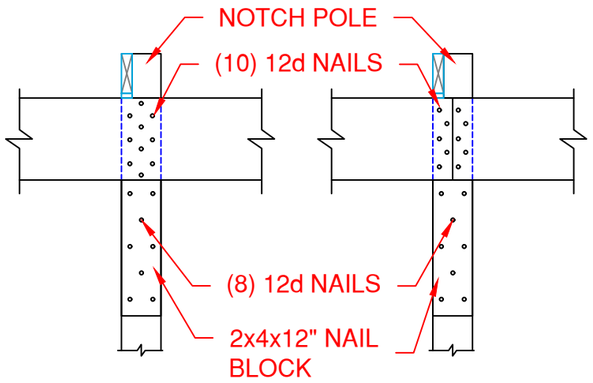
OD 2 O.H. DOOR JAMB DETAIL



RE 1 STEEL ROOF EDGE DETAIL

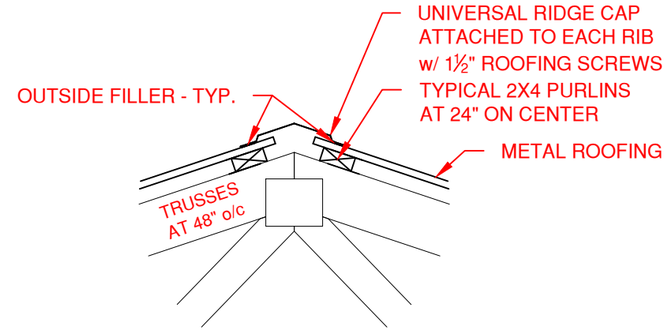


GD 2 FLUSH GABLE DETAIL



CONTINUOUS ***SPLICED***

HA 1 2X10 MSR OR BETTER HEADER ATTACHMENT



SR 1 STEEL ROOF RIDGE DETAIL

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: Detail A

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800 MAPLE AVE E
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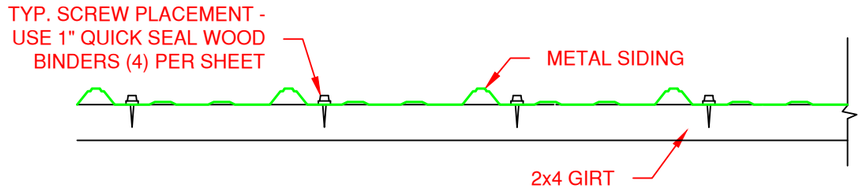
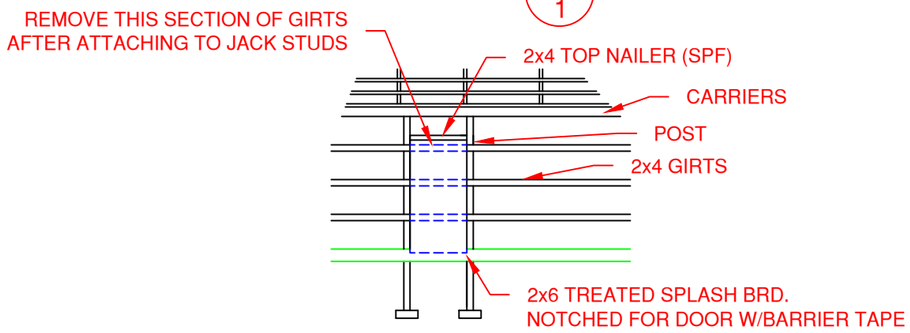
703-755-5363 (H)
954-260-2011 (M)

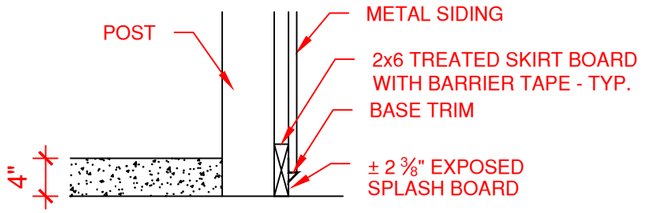
Contract # 400733-012



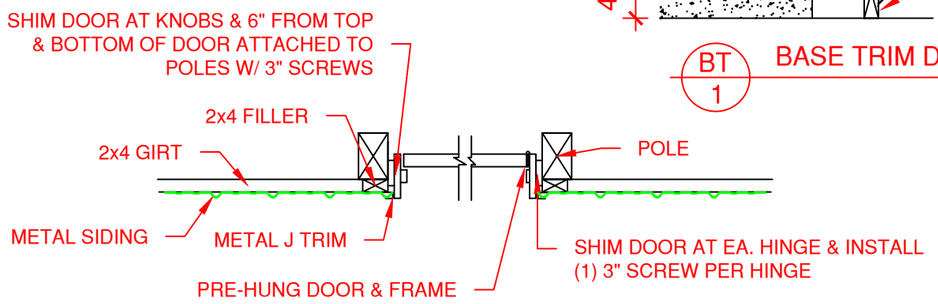
SS 1 SIDING SCREW PATTERN DETAIL



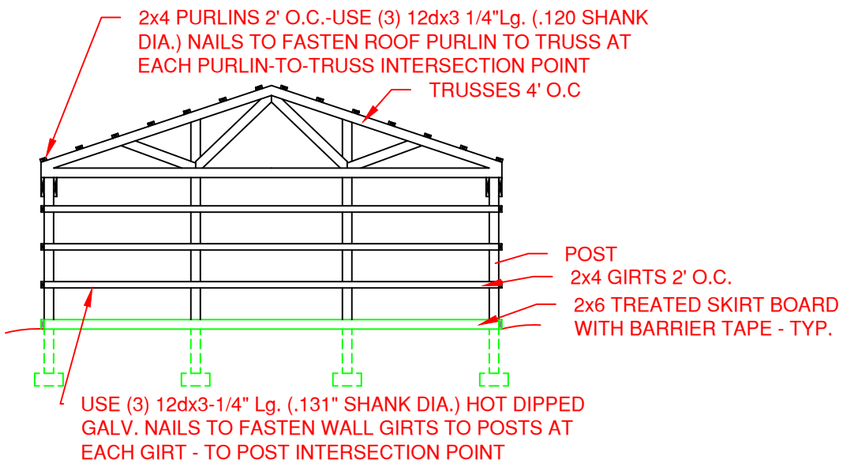
SD 1 SERVICE DOOR FRAMING DETAIL



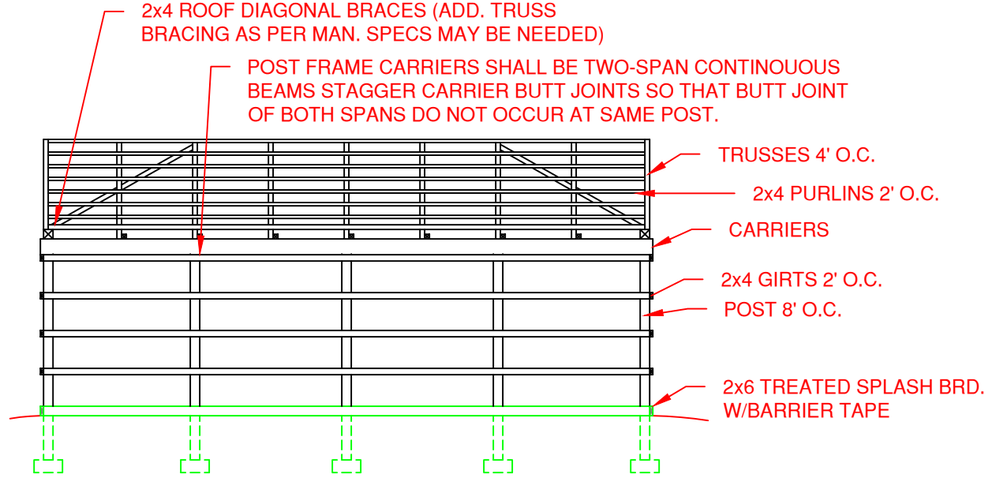
BT 1 BASE TRIM DETAIL



SD 2 SERVICE DOOR JAMB DETAIL



GF 1 GENERAL GABLE VIEW DETAIL



EF 1 GENERAL EAVE VIEW DETAIL

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: Detail B

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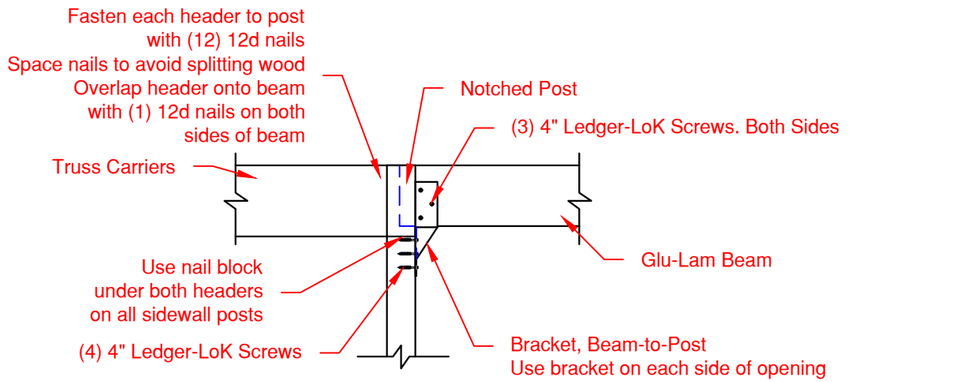
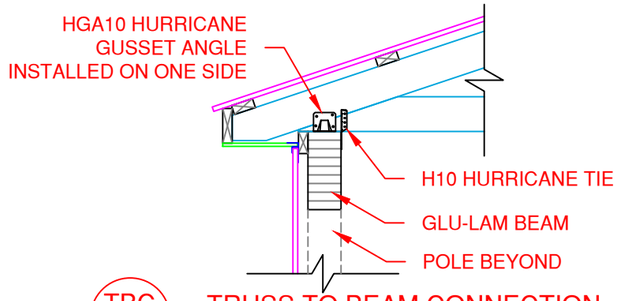
Contract # 400733-012



BB / 1 BEAM BRACKET DETAIL



TBC / 1 TRUSS TO BEAM CONNECTION

Revisions:

8-6-2025: (HLK) #012 BLDG SIZE

BUILDING SIZE: 30x58x14'-4"

DRAWN BY: HEIDI KLOCK

CHECKED BY:

Job Number: WESTWOOD-012

DATE: 1-20-2025

SHEET: Detail C

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BRIAN STONE & DAVID McGREGOR
800 MAPLE AVE E
VIENNA, VA 22180
703-755-5363 (H)
954-260-2011 (M)

Contract # 400733-012

General Notes:

Foundations:

A. Bottom of all exterior footings shall be minimum of 3'-0" below finished grade. Minimum size 8" high x 24" round or as noted.
B. Assumed design soil bearing pressure = 2,000 PSF U.N.O.
C. Minimum concrete footing strength to be 3,500 PSI at 28 days.

Concrete Floor:

A. The floor thickness shall be a minimum of 4", or per plans
B. A minimum of 4,000 PSI concrete W/ Fibermesh reinforcement or equivalent reinforcement
C. Expansion and control joints as needed
D. 6-mill poly vapor barrier shall be installed between the floor slab and base material, joints lapped 6" minimum
D.1. The vapor barrier is not required in R-3 detached unheated buildings, or for exposed exterior flat work

Metal Cladding:

A. Steel siding and roofing panels shall be fabricated from 27 & 28 gauge, grade e 180 KSI structural quality steel conforming to ASTM A-446 with a hot dipped galvanized coating conforming to ASTM A-525 or with an aluminum-zinc alloy coating conforming to ASTM A-792 (plain products only).
B. Paint Finish: All panels when required shall receive a factory applied polyester coating conforming to the manufacturer's specifications.
C. Flashings: All flashings shall be shop fabricated from material that is the same gauge and finish as the wall/roof panels to which they are attached.
D. Closures: Shall be pre-molded neoprene to match the configuration of the wall/roof panel and shall be in lengths as supplied by the panel manufacturer.
E. Fasteners: All screw fasteners shall have a combination steel and neoprene washer. Nails shall have a Fabriseal washer or equivalent. Fastener selection and installation shall be as recommended by the cladding manufacturer.

Wood Trusses:

A. Trusses are to be designed and fabricated in accordance with the published standards of the National Forest Products Association and the Truss Plate Institute's "Design Specifications for Light, Metal Plate Connected Wood Trusses" (TPI-XX) Latest Edition.
B. The web configuration plate sizes, chord sizes and lateral bracing shall be designed by a licensed professional engineer. The truss manufacturer shall provide the contractor with shop drawings of each truss design bearing the engineers seal. Shop drawings shall be approved by the contractor before fabrication.
C. All trusses shall be designed for the loading, spacing and geometry shown on the plan.
D. The contractor shall install the bracing of the wood trusses in accordance with the manufacturer's design. Minimum lateral bracing of web and bottom chord members shall be as required by truss design.

Lumber:

A. All lumber shall comply to the requirements of the American Institute of Timber Construction and the National Forest Products Association's "National Design Specification for Wood Construction".
B. All lumber for posts and beams shall be #2 or better southern yellow pine grade stamped by a SPIB approved mill, surfaced at a maximum moisture content of 19% treated .6 pcf CCA, .23 pcf MCA, or equal.
C. All lumber for headers shall be SYP #1 or Better, grade stamped by a SPIB approved mill, surfaced at a maximum moisture content of 19%.
D. All lumber exposed to ground contact or insect infestation shall be treated according to the American Wood preservers' Association Standards, .15 pcf MCA or equal.

Connections:

A. All wood connection to be made according to the "National Design Specification for Wood Construction". The minimum connection to be two 12 penny nails. Other connection as per plan or as controlled by standard construction practices.
B. It is acceptable for 2x4 wind girt spacing to vary from 18" to 30", when the span of the girt is 10' or less. Horizontal spacing of fasteners for the metal wall panels shall be in accordance with the panel manufacturer's instructions. The wind girt spacing up to 30" conforms to the rigid diaphragm design for post frame walls.

Cautionary Notes:

1. Structural components such as posts, beams, trusses or fasteners and attachment brackets should NOT be modified, notched or cut in any manner without proper review and approval of the building design professional.

2. Rainwater and melt water should be directed away from post foundation locations.

3. On enclosed buildings with large doors (that is buildings designed as completely enclosed) the doors should be closed during periods high wind and/or stormy weather to reduce uplift forces on the building.

4. Do NOT lean heavy materials against posts or girts unless the building has been designed for those types of loads. Do NOT store loose material against walls unless building has been designed for side thrust loads and any moisture contained in the loose materials.

5. Do NOT use the roof trusses for storing material unless the building and roof trusses have been designed for those loads.

6. Concentrated loads such as ceiling-mounted furnaces, wet sprinkler systems, ventilation hoods, etc. SHALL NOT be attached to the roof trusses without the prior review and written approval of Pioneer Pole Buildings, Inc. and the building design professional.

7. Do NOT install hardware that would maintain snow cover on the roof of buildings without the prior review and written approval of Pioneer Pole Buildings, Inc. and the building design professional.

8. Do NOT attach additional buildings or lean-to enclosed areas to pole barn buildings unless the building has been designed for the additional loads created by these building additions and needs the written approval of Pioneer Pole Buildings, Inc. and the building design professional.

9. Door openings should NOT be added to the building walls after the building has been constructed without review and approval of the building design professional.

Misc. Notes:

These plans are designed in accordance with the 2021 IBC
Construction Class VB

TRUSS CARRIERS USED TO BE EQUAL TO OR BETTER THAN 2x10 MSR 2400f - 2.0e
GLULAM BEAM SIZE= 5 $\frac{1}{4}$" x 13 $\frac{3}{4}$"

HURRICANE TIES USED = RT16A (USP CONNECTORS)
5K GUTTER W/DOWNSPOUT
DBL BUBBLE VAPOR BARRIER - ROOF & WALLS
R-21 INSULATION W/LINER - WALLS
R-38 INSULATION W/CEILING LINER
UNIVERSAL RIDGE VENT
IBC USE GROUP UTILITY
SNOW GUARDS INSTALLED ON EACH EAVE

DESIGN CRITERIA:
Ground Snow Loads:
Ground Snow Load (psf) = 40
Wind Speed:
Wind Speed = 115 mph
Truss Loads:
Top Chord Live (psf) = 30
Top Chord Dead (psf) = 5
Bottom Chord Live (psf) = 0
Bottom Chord Dead (psf) = 5

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