444 MAPLE AVENUE WEST

SITE PLAN TOWN OF VIENNA, VIRGINIA

DEPARTMENT OF PUBLIC WORKS NOTES

LESS THAN THREE-FOURTHS INCH (3/4") SIZE.

1) WATERLINES, ALL WATERLINE INSTALLATION SHALL SUBSCRIBE TO THE TOWN OF VIENNA'S CONSTRUCTION STANDARDS AND SPECIFICATIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING:

a. WATERLINES SHALL HAVE A MINIMUM COVER OF FOUR FEET (4'), UNLESS OTHERWISE SHOWN ON THE DRAWING, MEASURED FROM THE TOP OF THE PIPE TO THE FINISHED GROUND ELEVATION. IN NO EVENT SHALL THE DEFLECTION PER JOINT OF LENGTH EXCEED THE RECOMMENDED MAXIMUM DEFLECTION AS SET FORTH BY THE PIPE MANUFACTURER.

- b. FIRE HYDRANTS SHALL BE PLACED A MINIMUM OF EIGHTEEN INCHES (18") OR A MAXIMUM OF THIRTY—SIX INCHES (36") FROM THE FACE OF THE CURB, MEASURED FROM THE FACE OF THE CURB TO THE CENTER OF THE BARREL, AT A POINT ON THE HYDRANT IMMEDIATELY BELOW THE SWIVEL FLANGE. THE PUMPER NOZZLE SHALL BE EIGHTEEN INCHES (18") ABOVE THE CURB.
- c. FIRE HYDRANTS SHALL BE SO PLACED THAT THEY DO NOT FALL WITHIN THE SIDEWALK, UNLESS SO DIRECTED BY THE INSPECTOR FOR THE TOWN OF VIENNA. d. A SUITABLE SUMP MUST BE PROVIDED TO ALLOW DRAINING OF THE HYDRANT. FIRE HYDRANTS SHALL NOT TO BE PLACED IN
- e. HYDRANTS SHALL BE PLACED ON FIRM FOUNDATION TO SUPPORT THE UNDERBASE AND A CONCRETE BLOCK BRACE SHALL BE
- POURED AGAINST FIRM UNDISTURBED GROUND TO KEEP THE HYDRANT FROM BLOWING OFF. f. GRAVEL SHALL BE PLACED AROUND CIRCUMFERENCE OF THE HYDRANT BASE FOR A DISTANCE OF TWELVE INCHES (12") AND A HEIGHT OF EIGHTEEN INCHES (18") TO ASSURE POSITIVE DRAINAGE OF HYDRANT BARREL. GRAVEL SHALL BE CLEAN AND NOT
- g. POLYETHYLENE ENCASEMENT SHALL BE PROVIDED AROUND ALL WATER LINES, ENCASEMENT SHALL BE SEAMLESS AND IN ACCORDANCE WITH ANSI/AWWA STANDARD CLOS CLASS C, AND SHALL BE INSTALLED IN ACCORDANCE WITH PROCEDURES DESCRIBED IN ANSI/AWWA STANDARD CLOS AROUND THE WATER MAIN, INCLUDING ALL PIPE, FITTINGS, VALVES, HYDRANTS AND BRANCH CONNECTIONS WHERE SHOWN ON THE PLANS. THERE SHALL BE TWO FEET OF OVERLAP BETWEEN THE CUT SECTIONS OF POLYETHYLENE. ALL OVERLAPPING SECTIONS SHALL BE COMPLETELY TAPED IN ORDER TO HOLD THEM IN POSITION DURING BACKFILLING. ALL HOLES, SLITS, OR OPENINGS OF ANY SIZE SHALL BE REPAIRED IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION SUCH THAT THE INTEGRITY OF THE POLYETHYLENE ENCASEMENT IS COMPLETELY RESTORED TO THE SATISFACTION OF THE TOWN. POLYETHYLENE ENCASEMENT WILL NOT BE REQUIRED FOR WATER MAIN INSTALLED INSIDE OF
- 2) SEWER LINES: ALL SEWER LINE INSTALLATION SHALL SUBSCRIBE TO THE TOWN OF VIENNA'S CONSTRUCTION STANDARDS AND
- SPECIFICATIONS INCLUDING BUT NOT LIMITED TO THE FOLLOWING: a. PVC SEWER PIPE SHALL BE SCHEDULE 35 AND SHALL CONFORM TO THE A.S.T.M. DESIGNATION D3034.
- b. INFILTRATION TEST, THE INFILTRATION INTO THE COMPLETED SEWER SHALL NOT EXCEED A RATE OF TWO-HUNDRED (200) GALLONS PER INCH OF DIAMETER, PER MILE, PER DAY FOR EIGHT INCH (8") UP TO AND INCLUDING TWENTY-FOUR INCH (24") DIAMETER AND NOT MORE THAN FIVE-THOUSAND (5,000) GALLONS PER MILE, PER DAY FOR ALL SIZED OVER TWENTY-FOUR INCH (24") DIAMETER. FOR SHORT STRETCHES OF LESS THAN FIVE-HUNDRED FEET (500'), RATES UP TO FIFTY PERCENT (50%) MORE THAN THE ABOVE AMOUNTS WILL BE PERMITTED PROVIDED THAT THE TOTAL LEAKAGE IN THE COMPLETED CONTRACT DOES NOT EXCEED THE ABOVE LIMITS.
- c. TESTS FOR INFILTRATION SHALL BE MADE UNDER THE DIRECTION OF THE TOWN AND AT LOCATIONS SELECTED BY HIM. ALL LABOR, TEMPORARY EQUIPMENT, AND MATERIALS INCLUDING WEIRS NECESSARY FOR SUCH TESTS SHALL BE FURNISHED BY THE CONTRACTOR WITHOUT ADDITIONAL COMPENSATION. SUFFICIENT WEIR MEASUREMENTS SHALL BE MADE IN MANHOLES TO FURNISH THE NECESSARY INFORMATION. WHERE WEIR MEASUREMENTS ARE NOT SUITABLE IN THE TOWN'S OPINION, OTHER METHODS OF MEASUREMENTS AS HE SHALL DETERMINE MAY BE ADOPTED. WHERE PRACTICABLE, SEWERS SHALL BE TESTED IN LENGTHS BETWEEN MANHOLES OF NOT MORE THAT FIFTEEN-HUNDRED FEET (1,500').
- d. SEWERS WILL NOT BE TESTED FOR INFILTRATION UNTIL AT LEAST TWO (2) WEEKS AFTER INSTALLATION AND PRIOR TO ANY SERVICE CONNECTIONS UNLESS OTHERWISE APPROVED BY THE TOWN. SEWERS WHICH FAIL TO MEET TESTS SHALL BE REPAIRED UNTIL NECESSARY REQUIREMENTS OF THIS SPECIFICATION, AS EVIDENCED BY SUBSEQUENT TESTS, ARE COMPLIED WITH.
- 3) STORMWATER LINES: ALL STORMWATER LINE INSTALLATION SHALL SUBSCRIBE TO THE TOWN OF VIENNA'S CONSTRUCTION STANDARDS AND SPECIFICATIONS.
- 4) THE OWNER MUST PHYSICALLY DISCONNECT EXISTING WATER, SEWER LATERALS THAT ARE TO BE ABANDONED AT THEIR CONNECTION
- 5) THE CONTRACTOR SHALL FIELD VERIFY THE LOCATION OF EXISTING UNDERGROUND UTILITIES PRIOR TO INSTALLATION OF PROPOSED UTILITIES. A MINIMUM OF 18 INCHES VERTICAL AND FIVE FEET HORIZONTAL CLEARANCE SHALL BE MAINTAINED FROM ANY UTILITIES
- AND PUBLIC WATER AND SEWER MAINS. 6) ALL WATER AND SEWER FACILITIES SHALL BE INSTALLED BY A LICENSED CONTRACTOR IN THE STATE OF VIRGINIA.
- 7) ALL WATER AND SEWER CONSTRUCTION IS INSPECTED AND TESTED AS PER VIENNA STANDARDS PRIOR TO FINAL ACCEPTANCE BY THE TOWN.
- 8) THE VIENNA SEWER DEPARTMENT SHALL BE NOTIFIED AT A MINIMUM 48 HOURS (MONDAY THROUGH FRIDAY) PRIOR TO COMMENCING ANY WORK, TESTING, AND PRIOR TO MAKING ANY CONNECTIONS TO EXISTING SEWER LINES OR MANHOLES.
- 9) THERE SHALL BE NO UNMETERED CONNECTIONS TO THE TOWN'S WATER SYSTEM, INCLUDING CONNECTIONS BYPASSING METERS FOR TESTING ON-SITE PLUMBING OR FOR OBTAINING CONSTRUCTION WATER.
- 10) PRESSURE TESTING AGAINST VALVES WILL NOT BE ALLOWED.
- 11) A NOMINAL FOUR FEET OF COVER IS REQUIRED FOR ALL WATER MAINS AT FINAL GRADE.
- 12) THE USE OF A FIRE HYDRANT AS A WATER SOURCE IS PROHIBITED UNLESS A METER HAS BEEN OBTAINED FROM VIENNA FOR USE OF A SPECIFIC HYDRANT(S).
- 13) ABANDONMENTS OF EXISTING WATER AND SEWER CONNECTIONS ARE CLEARLY INDICATED ON THE PLANS WITH APPROPRIATE NOTES DESCRIBING HOW THE SERVICES ARE TO BE ABANDONED AT THE MAINS. INDIVIDUAL SERVICES MUST BE CUT AND THE REMAINING SERVICE LINE CRIMPED NOT LESS THAN 12" FROM THE MAIN AND NOT MORE THAN 3'. THE TOWN IS TO BE NOTIFIED 48 HOURS IN ADVANCE OF DISCONNECTION AND MAY INSPECT THE DISCONNECTION AT ITS CHOOSING SO AS TO PREVENT SLOW LEAKS, INCOMPLETELY TURN OFF CORPORATIONS AND OR FUTURE CONSTRUCTION ISSUES WITH TRENCHING THROUGH NONE CUT SERVICE
- 14) PER SECTION TOWN OF VIENNA CONSTRUCTION SPECIFICATIONS SECTION I.B.10: SUPPORTS SHALL BE CONSTRUCTED BEHIND ALL BENDS, TEES, CAPS, PLUGS AND HYDRANTS. THE USE OF SUPPORTS BEHIND FITTINGS REFERS TO FITTINGS IN THE VERTICAL PLANE AS WELL AS THOSE USED IN THE HORIZONTAL PLAN." THEY SHALL BE DESIGNED TO CARRY THE LOAD THAT WILL REASONABLY BE REQUIRED OF THEM UNDER MAXIMUM HEAD AND A REASONABLE ALLOWANCE FOR WATER HAMMER."
- 15) POLYETHYLENE ENCASEMENT IS REQUIRED FOR ALL LINES IN VIENNA IN ACCORDANCE WITH ANSI/AWWA STANDARD C105 CLASS C, PER SECTION1.B.14.B OF THE VIENNA STANDARD CONSTRUCTION SPECIFICATIONS.

TOWN OF VIENNA - NOTES

- 1. A PRE-CONSTRUCTION MEETING MUST BE HELD PRIOR TO THE START OF CONSTRUCTION. CALL 703-255-6384 TO SCHEDULE THE PRE-CONSTRUCTION MEETING.
- 2. ALL CONSTRUCTION GENERATED DEBRIS MUST BE HAULED AWAY BY THE CONTRACTOR OR OWNER.
- PRIOR TO THE REMOVAL OF ANY TOWN TREES (TREES WITHIN THE RIGHT OF WAY), THE APPLICANT OR THEIR REPRESENTATIVE SHALL CONTACT THE TOWN OF VIENNA ARBORIST AT 703-255-6360 TO COORDINATE HAVING THE TOWN ARBORIST ONSITE DURING ALL TOWN TREE REMOVAL.
- 4. TREE PROTECTION FOR ANY TOWN TREE, AS SHOWN ON PLAN, MUST BE INSTALLED PRIOR TO ANY SITE WORK.
- 5. IT IS UNLAWFUL TO PERFORM ANY CONSTRUCTION ABOVE FOUNDATION CORNERS PRIOR TO APPROVAL OF SETBACKS. WORK COMPLETED IN VIOLATION OF THIS REQUIREMENT IS SUBJECT TO DEMOLITION.
- 6. ALL DUMPSTERS/PODS ARE TO BE PLACED ON PRIVATE PROPERTY.
- 7. FRONT ELEVATION CHECKS ARE REQUIRED.

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- 8. WALL CHECK SURVEYS ARE REQUIRED AND MUST BE SUBMITTED PRIOR TO CONSTRUCTION ABOVE FOUNDATION CORNERS.
- 9. A CERTIFICATE OF OCCUPANCY IS REQUIRED PRIOR TO OCCUPANCY. ALL REQUIRED DOCUMENTATION AND INSPECTION MUST BE SUBMITTED/COMPLETED BEFORE THE TOWN OF VIENNA WILL ISSUE A CERTIFICATE OF OCCUPANCY.
- 10. EXISTING SANITARY SEWER LATERALS ARE TYPICALLY CAPPED AT OR NEAR THE PROPERTY LINE. THE REUSE OF THE PORTION OF THE EXISTING SANITARY SEWER LATERAL BETWEEN THE TOWN OWNED SEWER MAIN AND THE CAPPED END MAY BE ALLOWED PROVIDING THAT A LICENSED PLUMBER CERTIFIES THAT THE EXISTING PIECE OF PIPE IS GRADED PROPERLY AND IN LIKE NEW CONDITION. THE REUSE OF A PORTION OF THE EXISTING LATERAL DOES NOT IMPLY THAT THE TOWN IS WARRANTING THE CONDITION IN ANY WAY.

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CONSTRUCTION NOTES

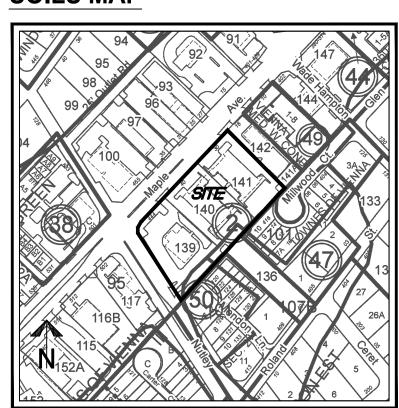
CONTRACTOR AND DEVELOPER ARE ADVISED THAT ANY ELECTRONIC FILES ASSOCIATED WITH THE PREPARATION OF THESE PLANS WILL NOT BE RELEASED TO OTHERS FOR USE IN CONSTRUCTION STAKEOUT OR RELATED SERVICES.

- 1. THE EXISTING UNDERGROUND UTILITIES SHOWN HEREON ARE BASED UPON AVAILABLE INFORMATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF ALL UTILITIES BEFORE COMMENCING WORK AND FOR ANY DAMAGES WHICH MAY OCCUR BY HIS FAILURE TO LOCATE OR PRESERVE THESE UNDERGROUND UTILITIES. IF DURING CONSTRUCTION OPERATIONS THE CONTRACTOR SHOULD ENCOUNTER UTILITIES OTHER THAN IN THOSE SHOWN ON THE PLANS. HE SHALL IMMEDIATELY NOTIFY THE ENGINEER AND TAKE NECESSARY AND PROPER STEPS TO PROTECT THE FACILITY AND ASSURE THE CONTINUANCE OF SERVICE.
- 2. THE CONTRACTOR SHALL DIG TEST PITS AS REQUIRED FOLLOWING NOTIFICATION AND MARKING OF ALL EXISTING UTILITIES BY MISS UTILITY TO VERIFY THE LOCATION AND DEPTH OF EXISTING UTILITIES. TEST HOLES TO BE PERFORMED AT LEAST 30 DAYS PRIOR TO START OF CONSTRUCTION. ANY DISCREPANCIES ARE TO BE REPORTED IMMEDIATELY TO THE OWNER AND ENGINEER. REDESIGN AND APPROVAL BY REVIEWING AGENCIES SHALL BE OBTAINED IF THIS INSTANCE OCCURS.
- 3. THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE OWNER AND THE ENGINEER OF ANY CHANGES OR CONDITIONS ATTACHED TO PERMITS OBTAINED FROM ANY AUTHORITY ISSUING PERMITS.
- 4. THE CONTRACTOR SHALL VISIT THE SITE AND SHALL VERIFY EXISTING CONDITIONS PRIOR TO STARTING CONSTRUCTION.
- 5. THE CONTRACTOR SHALL CLEAR THE SITE OF ALL TREES, BUILDINGS, FOUNDATIONS, ETC. WITHIN THE LIMITS OF CONSTRUCTION UNLESS OTHERWISE SPECIFIED, AND SHALL BE RESPONSIBLE FOR CAUSING EXISTING UTILITIES TO BE DISCONNECTED
- 6. THE DEVELOPER SHALL PROVIDE OVER-LOT GRADING TO PROVIDE POSITIVE DRAINAGE AND PRECLUDE PONDING OF WATER.
- 7. FINISHED GRADES SHOWN FOR FINISHED TOP OF CURB GRADES ON EXISTING ROADS SHALL BE FIELD ADJUSTED AS REQUIRED TO CONFORM TO THE INTENT OF THE TYPICAL SECTION USING THE EXISTING EDGE OF PAVEMENT AS THE CONTROL POINT. A SMOOTH GRADE SHALL BE MAINTAINED FROM THE CENTERLINE OF THE EXISTING RIGHT-OF-WAY TO THE FACE OF CURB TO PRECLUDE THE FORMING OF FALSE GUTTERS AND/OR THE PONDING OF WATER ON THE ROADWAY. THE EXISTING PAVEMENT SHALL BE RECAPPED AND/OR REMOVED AND REPLACED AS REQUIRED TO ACCOMPLISH THIS REQUIREMENT. CURB FORMS SHALL BE INSPECTED AND APPROVED FOR HORIZONTAL AND VERTICAL ALIGNMENT BY CITY OF FALLS CHURCH INSPECTORS PRIOR TO PLACING OF CONCRETE. CONTRACTOR SHALL REFER TO THE ARCHITECTURAL AND STRUCTURAL DRAWINGS FOR FINISHED GRADES ON TOP OF STRUCTURED PARKING DECK.
- 8. AREAS ON OR OFF-SITE, WHICH ARE DISTURBED BY THIS CONSTRUCTION AND WHICH ARE NOT PAVED OR BUILT UPON, SHALL BE ADEQUATELY STABILIZED TO CONTROL EROSION AND SEDIMENTATION. THE MINIMUM ACCEPTABLE STABILIZATION SHALL CONSIST OF PERMANENT GRASS, SEED MIXTURE TO BE AS RECOMMENDED BY THE CITY AGENT. ALL SLOPES 3:1 AND GREATER SHALL BE SODDED AND PEGGED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE TOWN OF VIENNA.
- 9. EXISTING WELLS SHALL BE PERMANENTLY ABANDONED IN ACCORDANCE WITH VIRGINIA STATE WATER CONTROL BOARD REQUIREMENTS.
- 10. ALL OVER HEAD POLE LINES SHALL BE RELOCATED OR REMOVED AS SHOWN BY THE OWNING UTILITY COMPANIES. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING ALL ARRANGEMENTS AND COORDINATING ALL WORK REQUIRED FOR THE
- 11. SUBBASE MATERIAL SHOWN ON THE TYPICAL STREET SECTION SHALL CONFORM TO VDOT SPECIFICATIONS SECTION 209. PAVEMENT THICKNESS AS SHOWN ON THE PLAN 31. ALL STREET CUTS TO MEET VDOT STREET RESTORATION DETAILS ARE BASED ON AN ASSUMED SOIL SUPPORT VALUES (S.S.V.) OF 10 UNLESS OTHERWISE NOTED. A QUALIFIED SOILS TESTING FIRM SHALL BE ENGAGED BY THE CONTRACTOR TO DETERMINE THE ACTUAL S.S.V. IN ACCORDANCE WITH "A DESIGN GUIDE FOR SUBDIVISION PAVEMENTS IN VIRGINIA" BY N.K. VASWANI, OCTOBER 1973. VHRC 73-821 AS AMENDED. SOIL SUPPORT VALUES SHALL BE OBTAINED AT EACH CHANGE IN SUBGRADE SOILS AND AT A MAXIMUM SPACING OF 500 FEET WHERE SUBGRADE SOILS REMAIN CONSTANT. S.S.V. SHALL BE FURNISHED TO THE ENGINEER AND THE ENGINEER SHALL REVISE THE PAVEMENT DESIGN THICKNESS TO SHOW THE ACTUAL DEPTH OF PAVEMENT MATERIAL REQUIRED AND SHALL SUBMIT THE REVISION TO THE CITY OF FALLS CHURCH FOR REVIEW AND APPROVAL. THE CONTRACTOR IS ADVISED NOT TO BRING THE AREA SUBJECT TO THIS REVISION TO FINISHED GRADE UNTIL AFTER THE REVISED PAVEMENT SECTION IS APPROVED.
- 12. PRIOR TO BEGINNING CONSTRUCTION, CONTRACTOR SHALL VERIFY FROM THE ARCHITECTURAL DRAWINGS ALL DIMENSION, DETAILS, AND TREATMENTS FOR THE PROPOSED BUILDINGS, WALKWAYS, AND OTHER PROPOSED CONSTRUCTION WHERE INDICATED ON THE PLANS. ANY DISCREPANCIES SHALL IMMEDIATELY BE REPORTED TO DESIGN ENGINEER.
- 13. THE CONTRACTOR IS TO VERIFY INVERT, SIZE AND LOCATION OF BUILDING UTILITY CONNECTIONS WITH THE MECHANICAL PLANS PRIOR TO PLACEMENT OF UNDERGROUND UTILITIES.
- 14. NO UNDERGROUND SOILS INVESTIGATION HAS BEEN PERFORMED BY WALTER L. PHILLIPS, INC. ALL SOILS INFORMATION PRESENTED AS PART OF THIS SITE PLAN HAS BEEN PREPARED BY OTHERS AND IS INCLUDED AS REQUIRED FOR CITY OF FALLS SITE PLAN APPROVAL.
- 15. THE CONTRACTOR SHALL REMOVE EXISTING BUILDINGS, FENCES AND OTHER EXISTING PHYSICAL FEATURES AS REQUIRED.

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- 16. ALL PROPOSED SIDEWALK, CG-6, CG-2 OR CG-6R IS TO BE CONSTRUCTED WITH A MINIMUM 6" AGGREGATE BASE. SEE SHEET C-0202 AND C-0203 FOR DETAILS
- 17. EXISTING CONSTRUCTION SHALL BE REMOVED TO NEAREST JOINT. NEW CONSTRUCTION SHALL BE PROVIDED AS SHOWN AND ANY DAMAGED AREA SHALL BE REPAIRED TO MATCH CONDITIONS EXISTING PRIOR TO CONSTRUCTION.
- 18. DAMAGE TO ANY EXISTING ENTRANCES, CURB AND GUTTER, PAVEMENT OR OTHER EXISTING STRUCTURES NOT PROPOSED TO BE DISTURBED WITH THIS DEVELOPMENT, WILL BE THE RESPONSIBILITY OF THE CONTRACTOR AND MUST BE REPAIRED TO THE SATISFACTION OF THE TOWN OF VIENNA, THE VIRGINIA DEPARTMENT OF TRANSPORTATION AND ANY ADJOINING OWNERS THAT MAY BE AFFECTED.
- 19. THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING A SMOOTH TRANSITION TO EXISTING CURB.
- 20. ALL PRIVATE BUILDING CONNECTIONS ARE TO BE INSTALLED IN ACCORDANCE WITH THE CURRENT PLUMBING CODE.
- 21. SEE ARCHITECTURAL AND/OR LANDSCAPE DRAWINGS FOR DIMENSIONS AND DETAILS FOR ALL RETAINING WALLS. ALL ON-SITE RETAINING WALLS ARE SUBJECT TO A SEPARATE BUILDING PERMIT TO BE OBTAINED BY OWNER. THIS PLAN IS FOR APPROXIMATE LOCATION AND PROPOSED GRADING ONLY. GEOTECHNICAL AND STRUCTURAL DESIGN TO BE ACCOMPLISHED BY OTHERS. RETAINING WALLS SHOWN ON THIS PLAN ARE FOR THE PURPOSES OF DEMONSTRATING THE PROPOSED TOP AND BOTTOM ELEVATIONS AND LOCATION OF THE WALLS ONLY. RETAINING WALLS ARE TO BE MAINTAINED BY THE PROPERTY OWNERS.
- 22. TOPS OF EXISTING STRUCTURES WHICH REMAIN IN USE ARE TO BE ADJUSTED IN ACCORDANCE WITH THE GRADING PLAN. ALL PROPOSED STRUCTURE TOP ELEVATIONS ARE TO BE VERIFIED BY THE CONTRACTOR WITH THE SITE GRADING PLANS. IN CASE OF CONFLICT, THE GRADING PLAN SHALL SUPERSEDE PROFILE ELEVATIONS. MINOR ADJUSTMENTS TO MEET FINISHED GRADE ELEVATIONS MAY BE REQUIRED.
- 23. SEE LANDSCAPE PLAN FOR ALL ON-SITE SIDEWALK, PLANTING AND IRRIGATION
- 24. THE DESIGN, CONSTRUCTION, FIELD PRACTICES AND METHODS SHALL CONFORM TO THE REQUIREMENTS SET FORTH BY THE TOWN OF VIENNA AND ITS CURRENT ZONING ORDINANCE AND CONSTRUCTION STANDARDS MANUAL, FAILURE TO COMPLY WITH THE CODE, APPLICABLE MANUALS, PROVISIONS OF THE CONSTRUCTION AND ESCROW AGREEMENTS OR THE PERMITS SHALL BE DEEMED A VIOLATION.
- 25. THE APPROVAL OF THESE PLANS SHALL IN NO WAY RELIEVE THE OWNER/DEVELOPER OR HIS AGENT OF ANY LEGAL RESPONSIBILITIES WHICH MAY BE REQUIRED BY THE CODE OF VIRGINIA OR ANY ORDINANCE ENACTED BY THE GOVERNING AGENCY.
- 26. A MINIMUM PERMISSIBLE GRADE OF 1.00% IS REQUIRED FOR PAVEMENT TO ASSURE POSITIVE DRAINAGE. IF THERE IS EXISTING PAVEMENT WHICH IS TO REMAIN DISTURBED DURING CONSTRUCTION AND IS LESS THAN 1.00%, THEN THE CONTRACTOR IS TO CHECK TO MAKE SURE THE SITE AREA WILL HAVE ADEQUATE DRAINAGE.
- 27. NO EVIDENCE OF GRAVES OR BURIAL SITES HAS BEEN FOUND ON THIS PROPERTY.
- 28. CONSTRUCTION STAKEOUT SHALL BE UNDER THE DIRECT SUPERVISION OF A LICENSED LAND SURVEYOR IN THE COMMONWEALTH OF VIRGINIA.
- 29. CONTRACTOR TO COORDINATED TIE BACK LOCATIONS WITH EXISTING AND PROPOSED UTILITIES AND OBTAIN APPROVALS REQUIRED BY THE TOWN OF VIENNA.
- 30. ALL SITE WORK TO MEET TOWN OF VIENNA STANDARDS AND SPECIFICATIONS PER THE CURRENT PUBLIC INFRASTRUCTURE MANUAL (PIM). IF A STANDARD IS NOT INCLUDED IN THE PIM REFER TO THE FAIRFAX COUNTY PUBLIC FACILITIES MANUAL (PFM).
- 32. RELOCATED STORM SEWER TO BE INSTALLED AND BACKFILLED PER TOWN OF VIENNA STANDARDS.

SOILS MAP



SOILS DATA

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SOIL #'S	SERIES NAME	HYDROLOGIC SOIL GROUP	PROBLEM CLASS	FOUNDATION SUPPORT	SUBSURFACE DRAINAGE	ERODABIL
95	URBAN LAND	N/A	IVB	N/A	N/A	N/A
107B	WHEATON- MEADOWVILLE COMPLEX	D	IVB	FAIR	MARGINAL	MEDIUM

STRUCTURAL ENGINEER TO DETERMINE LEVEL OF REQUIRED INVESTIGATION FOR BUILDING DESIGN PURPOSES.

SCALE: 1"=300

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> NOTE: NO SITE PLAN MODIFICATIONS REQUESTED

FIRE FLOW INFORMATION SEE SHEET C-1302, FIRE MARSHAL APPROVAL

WILL BE OBTAINED PRIOR TO TOWN APPROVAL. FILE # 79830

FIRE MARSHAL APPROVAL - MICHAEL PARUTI DATE 01/21/2021

GENERAL NOTES:

PROPERTY ADDRESS: 430,440,444 MAPLE AVENUE WEST SITE AREA: 120,091 SF OR 2.76 ACRES TYPE OF PLAN: MAPLE AVENUE REZONING APPROVED 10/29/2018 EXISTING ZONE: RS-16/C-1 PROPOSED/APPROVED ZONE: MAC (SE CLERK LETTER SHEET C-0201)

see sheet **T-ogwyn ao f**on**iv i pornaton** OWNER INFORMATION OF THE NA DEVELOPMENT AS DICITIES, LLC C/C HEKEMIAN & COMPANY 505 MAIN STREET SHITE SW2021 HACKENSACK, NJ 07601

COMMUNITY CONTACT:

CHRIS BELL CHRIS@HEKEMIAN.COM (410) 626 9607

SCALE: 1"=2000

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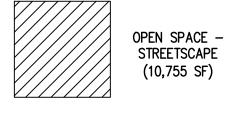
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ATTN: CHRIS BELL (410) 626 9607

OPEN SPACE CALCULATION

LOT AREA:		120,091 SF (2.76 AC)
MIN OPEN SPACE REQUIRED	15% OF LOT AREA	18,014 SF MIN.
OPEN SPACE PROVIDED	20.0% OF LOT AREA	24,042 SF



OPEN SPACE – SIDE AND REAR (13,287)

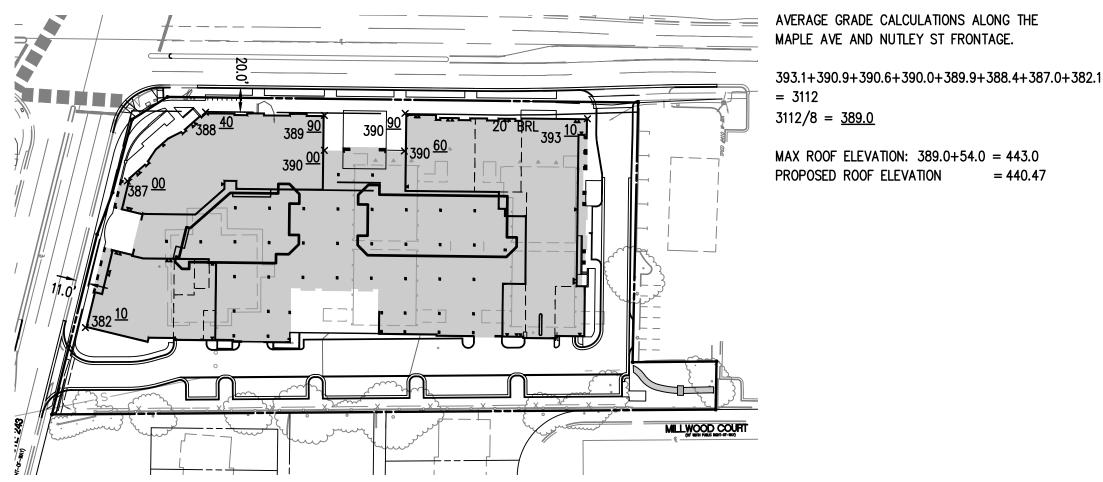
ZONING TABULATION

PROPOSED ZONE: MAPLE AVENUE COMMERCIAL (MAC)

	MAC ZONING REQUIREMENTS	PROVIDED
LOT AREA	NONE	120,091 SF (2.76 AC)
LOT WIDTH	NONE	392.28'
BUILDING WIDTH	NONE	196'
BUILDING HEIGHT**	54'	51.47'
YARD REQUIREMENTS		
FRONT ON MAPLE	20' MINIMUM TO FACE OF CURB	20.1'
FRONT OF NUTLEY	15' MINIMUM TO FACE OF CURB	15.7'
SIDE (EAST PL)	WITH WALLS OR DOORS = 8' IF ABUTS A COMMERCIAL USE	40.3'
REAR (SOUTH PL)	20'	58.1'
MAX IMPERVIOUS SURFACE	80% OF THE LOT + 5% INCENTIVE BONUS = 85% (102,077 SF)	84.7% (101,664 SF)
OPEN SPACE	15% OF LOT AREA (18,014 SF)	20.0% (24,042 SF)
LOADING		
LOADING SPACE DEPTH	25'	38' MINIMUM
LOADING SPACE WIDTH	15'/50' OF BUILDING WIDTH = 60' TOTAL	25',35' (60' TOTAL)
LOADING SPACE HEIGHT	15'	17' RES.; 19' RETAIL
**AVERAGE GRADE = 389.0		

SCALE: 1" = 80'

FINISHED LOT GRADE CALCULATION



PARKING TABULATION

9	6.0%
	0.070
84	56.0%
57	38.0%

PARKING SUMMARY		
PARKING REQUIRED	REQUIREMENT	REDUCTION
RESIDENTIAL	249	212
RETAIL	168	144
TOTAL PARKING REQUIRED	417	356
PARKING PROVIDED	ACTUAL	BONUS**
RESIDENTIAL (BASEMENT)	181	226
RETAIL (GROUND FLOOR / SURFACE)	127	148
TOTAL PARKING PROVIDED	308	374

*15% REDUCTION BASED ON VIENNA ZONING CODE SECTION 18-95.19
**25% BONUS BASED ON VIENNA ZONING CODE SECTION 18-95.10 A
****Assumed 1 seat per 15 SF
****PARKING TOTALS EXCLUDE COMPACT SPACES

PARKING DETAILS

PARKING REQUIRED					
RESIDENTIAL	COUNT	RATIO	REQUIREMENT	REDUCTIO)N*
STUDIO	9	1 / UNIT	9	x0.85	8
1 BEDROOM	84	1.5 / UNIT	128	x0.85	109
2 BEDROOM	57	2 / UNIT	112	x0.85	95
TOTAL	150		249		212
RETAIL					
RETAIL	11,052 SF	5 / 1000 SF	56	x0.85	48
RESTAURANT	368 SEATS***	1 / 4 SEATS	92	x0.85	79
OUTDOOR SEATING	79 SEATS***	1 / 4 SEATS	20	x0.85	17
TOTAL			168		144
TOTAL PARKING REQUIRED			417		356
PARKING PROVIDED					
	GARAGE (G)	SURFACE (S)	TOTAL	BONUS**	
RESIDENTIAL PARKING	181	0	181	Gx1.25+S	226
RETAIL PARKING	84	43	127	Gx1.25+S	148
TOTAL PARKING PROVIDED	265	43	308		374

COMPACT PARKING RATIO			5.2%
RESIDENTIAL PARKING PROVIDED	ACTUA	L	BONUS**
STANDARD (100% OF REQUIRED SPACES)	181	Gx1.25+S	228
COMPACT (0% OF REQUIRED SPACES)	16	Gx1.25+S	21
TOTAL	197		249
RESIDENTIAL PARKING REQUIRED AFTER RED	UCTION*		211
SURPLUS PARKING AFTER BONUS AND REDUCTION * **		38	

RESIDENTIAL (EXTERIOR)
RETAIL
TOTAL PARKING PROVIDED

Town of Department of Planning and Zoning MAC ZONE DESIGNATION APPLICATION INCENTIVES SHEET Please check boxes for all incentives being requested Type of Incentive Requested From Schedule A From Schedule B A five percent increase in the maximum 2 2 2

ed .	Minimum Number of Features to be Provided			
Type of Incentive Requested	From Schedule A	From Schedule B		
A five percent increase in the maximum impervious surface	2	2	(X)	
A ten percent increase in the maximum impervious surface	2	3	0	
A 7.5 percent reduction in the minimum number of parking spaces required	1	2	0	
A 15 percent reduction in the minimum number of parking spaces required	2	3	(X)	
Inclusion of a mezzanine level for a commercial use which shall not count as an additional story.	1	1	0	

Please check boxes for all incentive features being used in MAC project

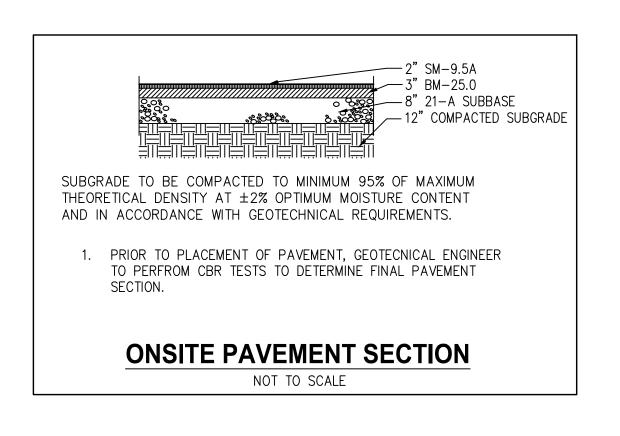
Schedule	Type of Feature	
GREEN BU	LDING CERTIFICATION	
AA	Designed to Earn the Energy Star certification (EPA program)	O
В	Structure design that can accommodate the installation and operation of solar photovoltaic panels or solar thermal heating devices (including appropriate wiring and water transport systems)	⊗
BBB	Construction of the principal structure to meet or exceed LEED Silver certification standards or another equivalent green building certification program	0
ВВ	Construction of the principal structure to Earn the Design for Energy Star certification or similar program	0
WATER CO	NSERVATION AND QUALITY PROTECTION	
AA	Configuration of the principal structure's roof so that at least 50 percent of the roof is a "green" roof intended to capture and hold rain water	0
Α	Inclusion of rain water capture and re-use devices such as cisterns, rain filters, and underground storage basins with a minimum storage capacity of 500 gallons	0
Α	Provision of open space set-asides at a rate 200 percent or more beyond the minimum required	0
В	Provision of rain gardens or other appropriate stormwater infiltration system(s) of at least two percent of the total site area.	⊗
В	Use of xeriscape landscaping techniques without irrigation	
SITE CONF	IGURATION	
AA	Inclusion of underground parking or parking structures sufficient to accommodate 51 percent or more of the off-street parking requirements	\otimes
AA	Closure of one existing accessway, vehicular entrance or driveway on Maple Avenue	
Α	Provision of public art with a value meeting at least one percent of the total construction cost	
Α	Provision of all required long-term bicycle parking spaces in weather-protected rooms or cages with controlled access for bicycle users, and that also contain racks for the socure attachment of bicycles using a lock or other similar device.	n of Vienn
ВВ	Provision of transit facilities (e.g., designated park-and-ride parking spaces, t us shelters*, bicycle share facilities or similar features)	8
В	Inclusion of showering and dressing facilities in nonresidential development: for employees alternative forms of transportation	pproved
В	Provision of at least one enclosed recycling station per building suitable for s orage and collection of recyclable generated on-site	06/09/2021
BBB *The loo	Provision of public parking in an amount to exceed 110 percent of the required parking for the development	00/07/2021

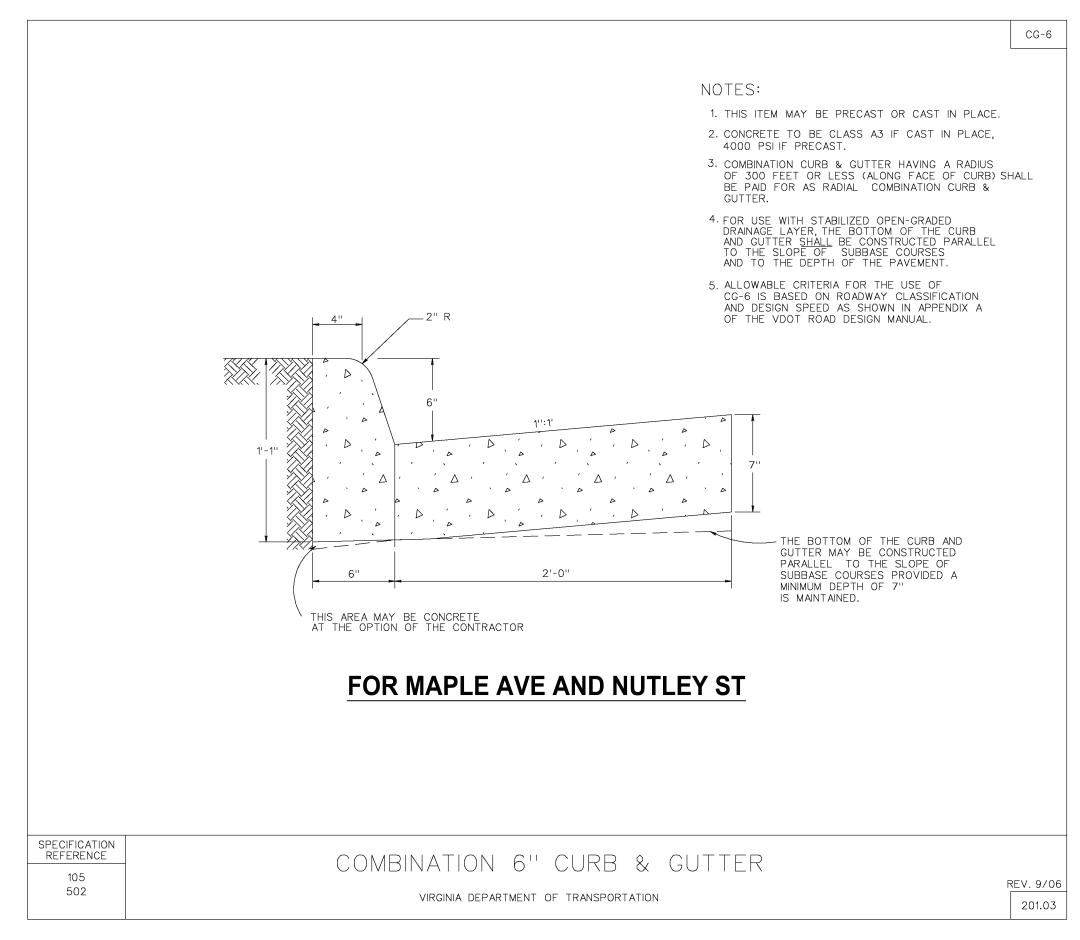
127 Center Street, South • Vienna, Virginia 22180 p: (703) 255-6341 • f: (703) 255-5729 • TTY711 www.viennava.gov

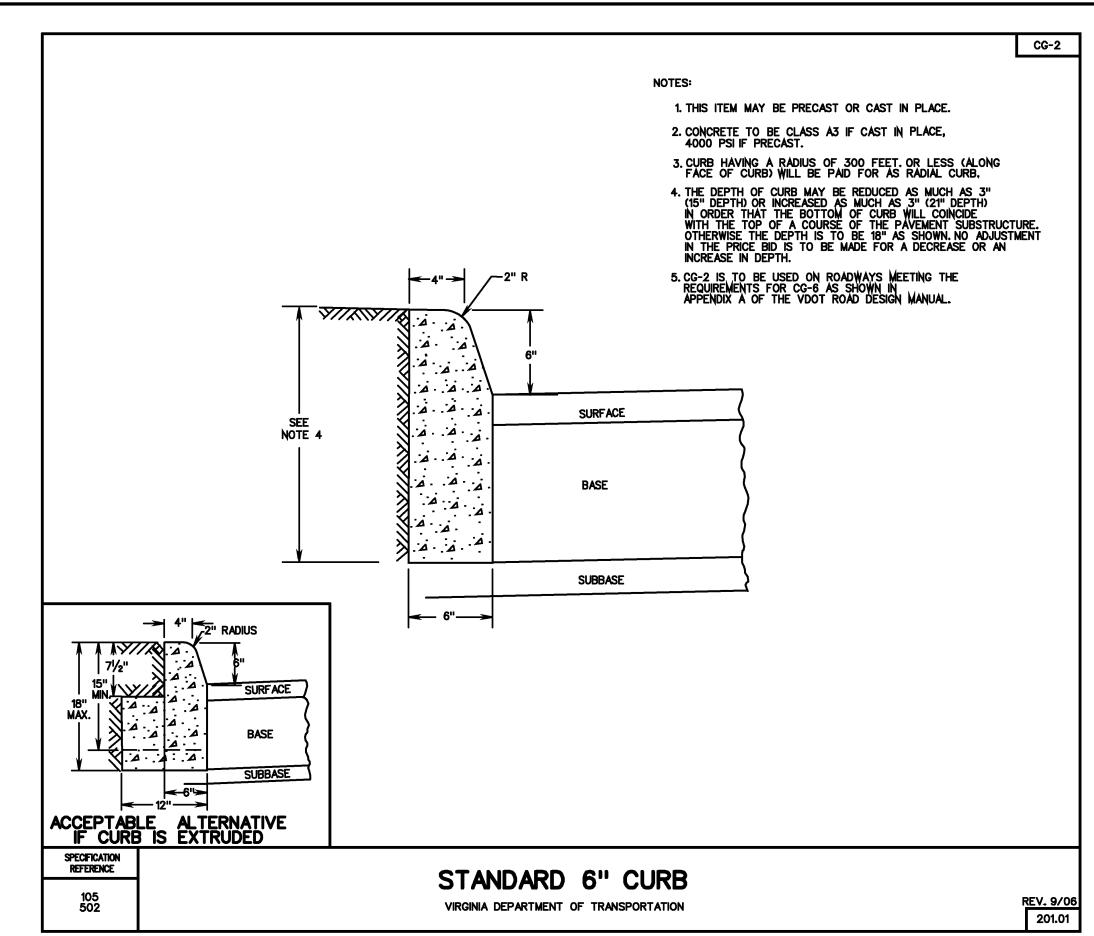
ZONING TABULATI

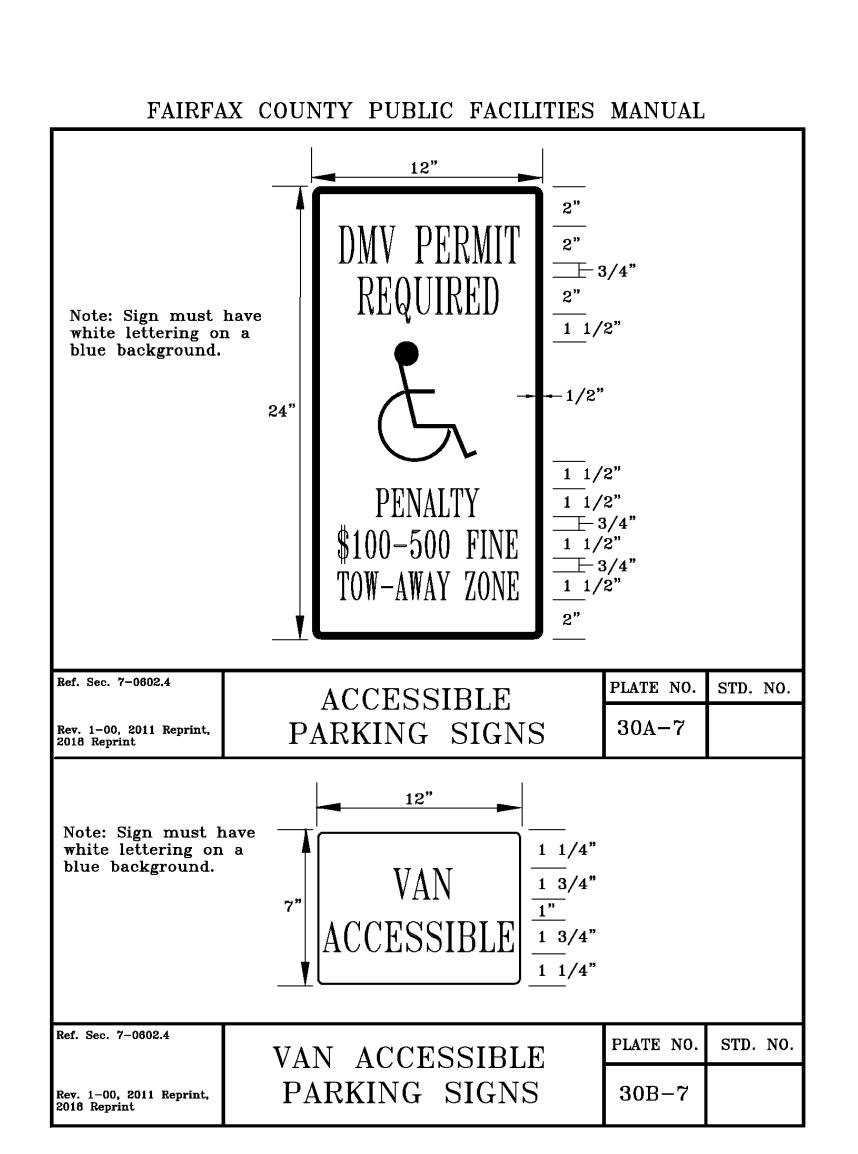
44 MAPLE AVENUE W

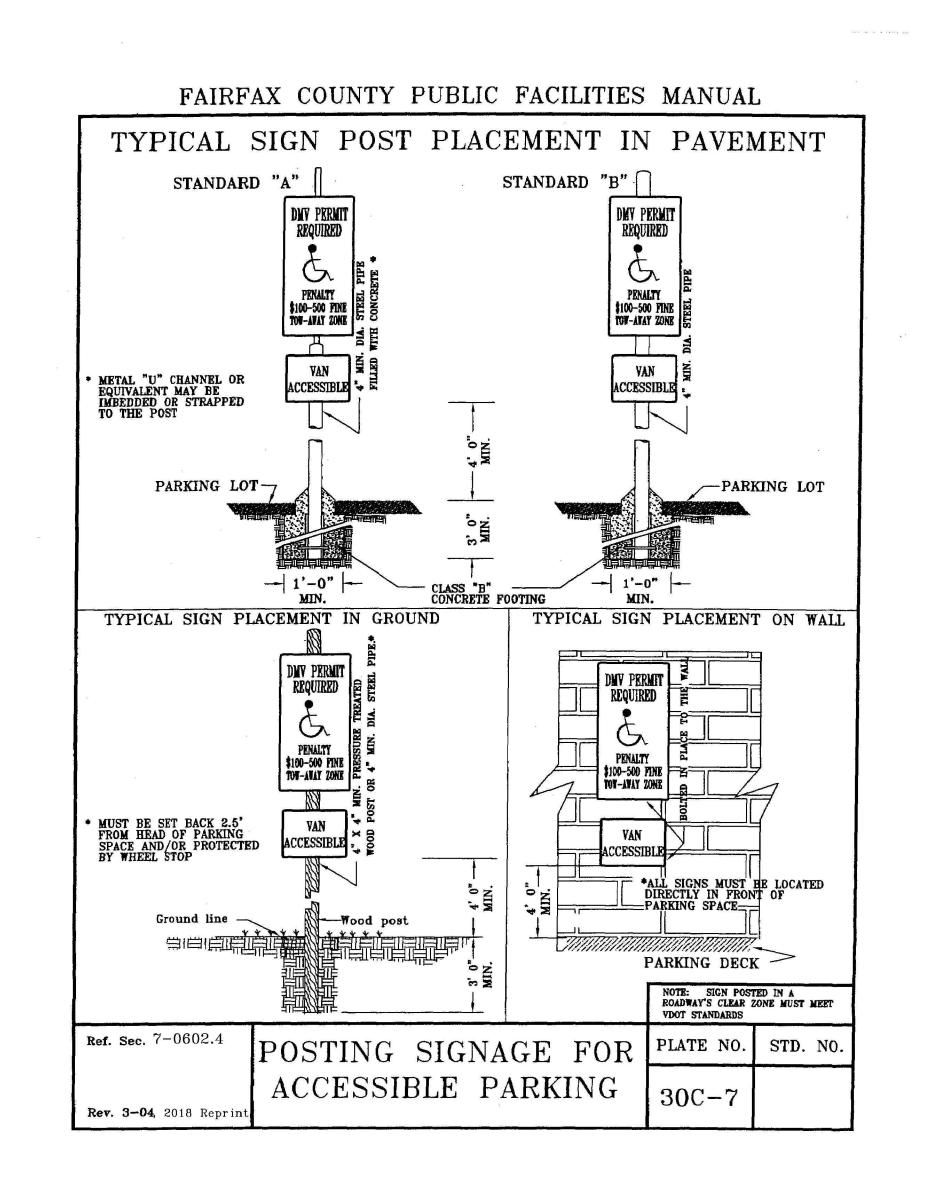
EXISTING IMPERVIOUS AREA = 100,766 SF (83.9%)

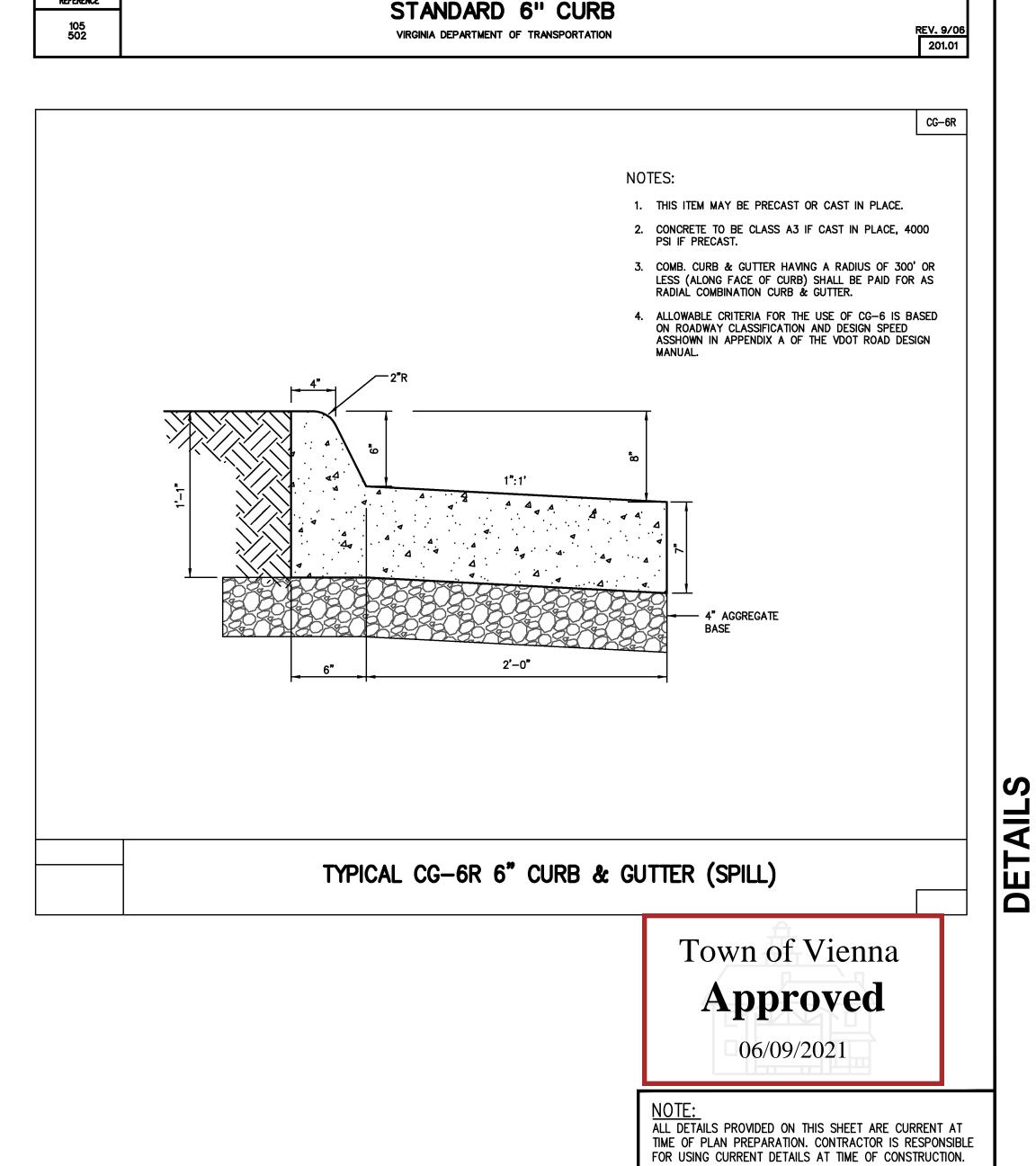


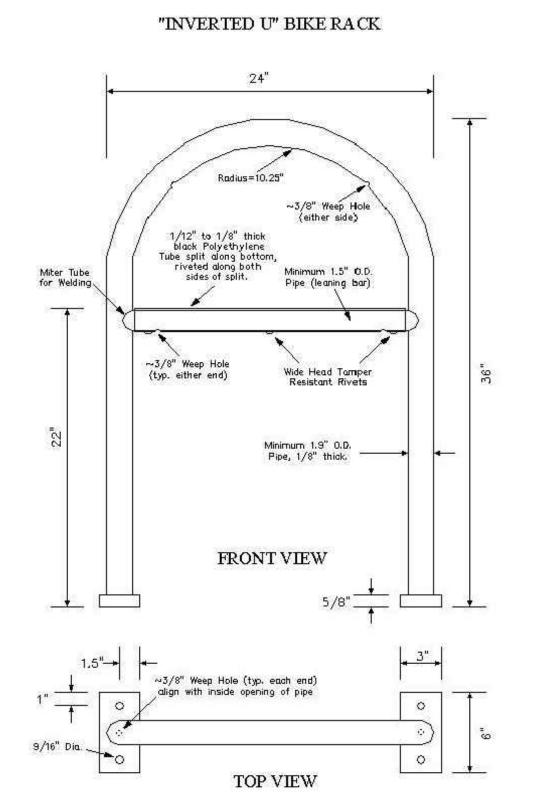








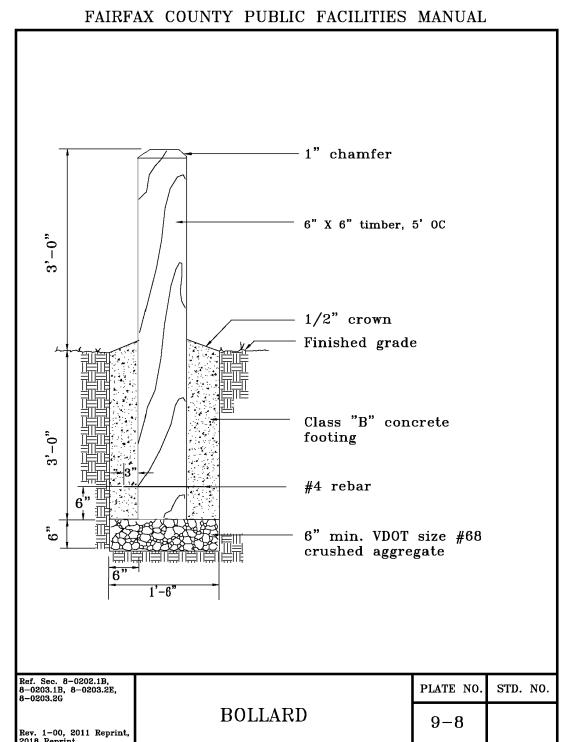


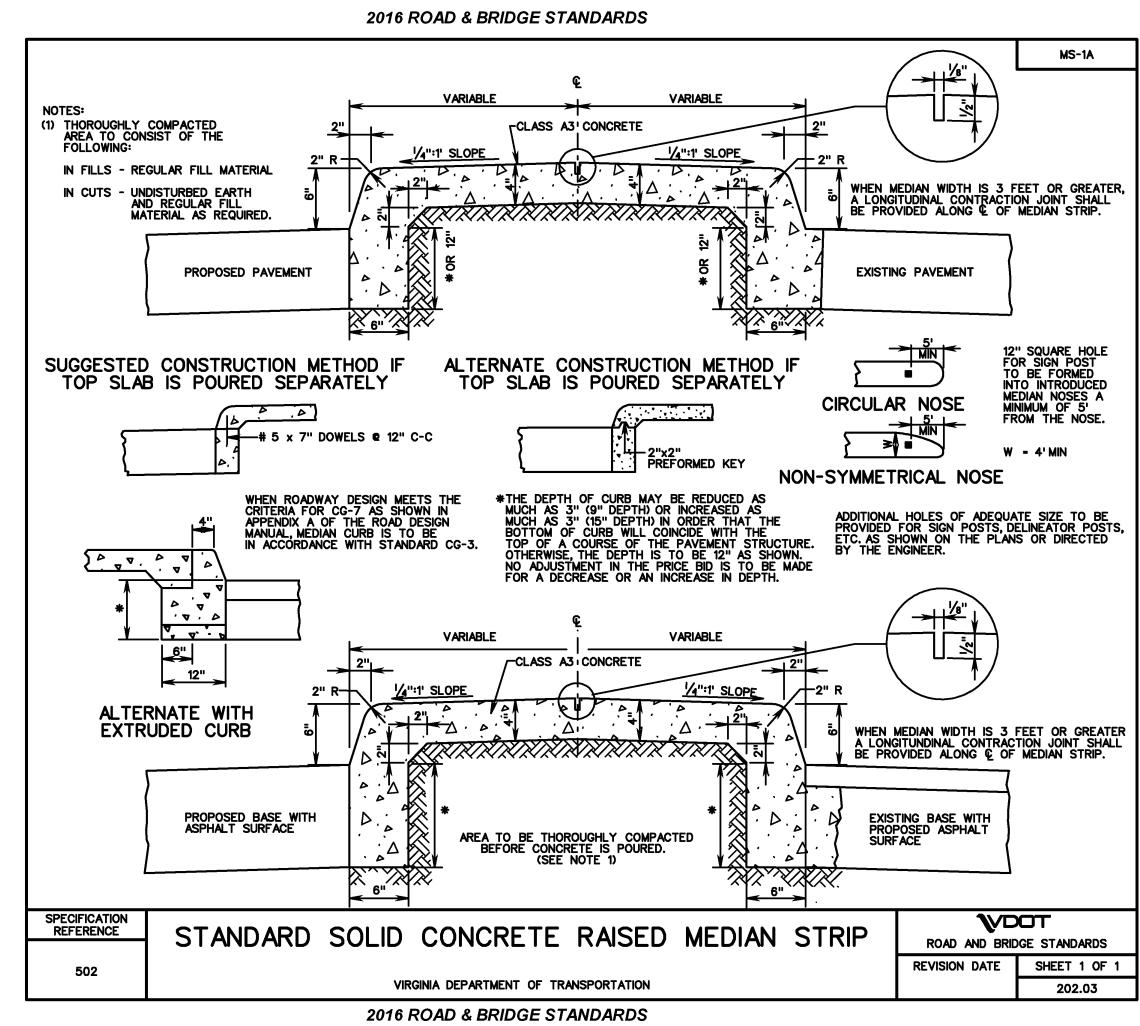


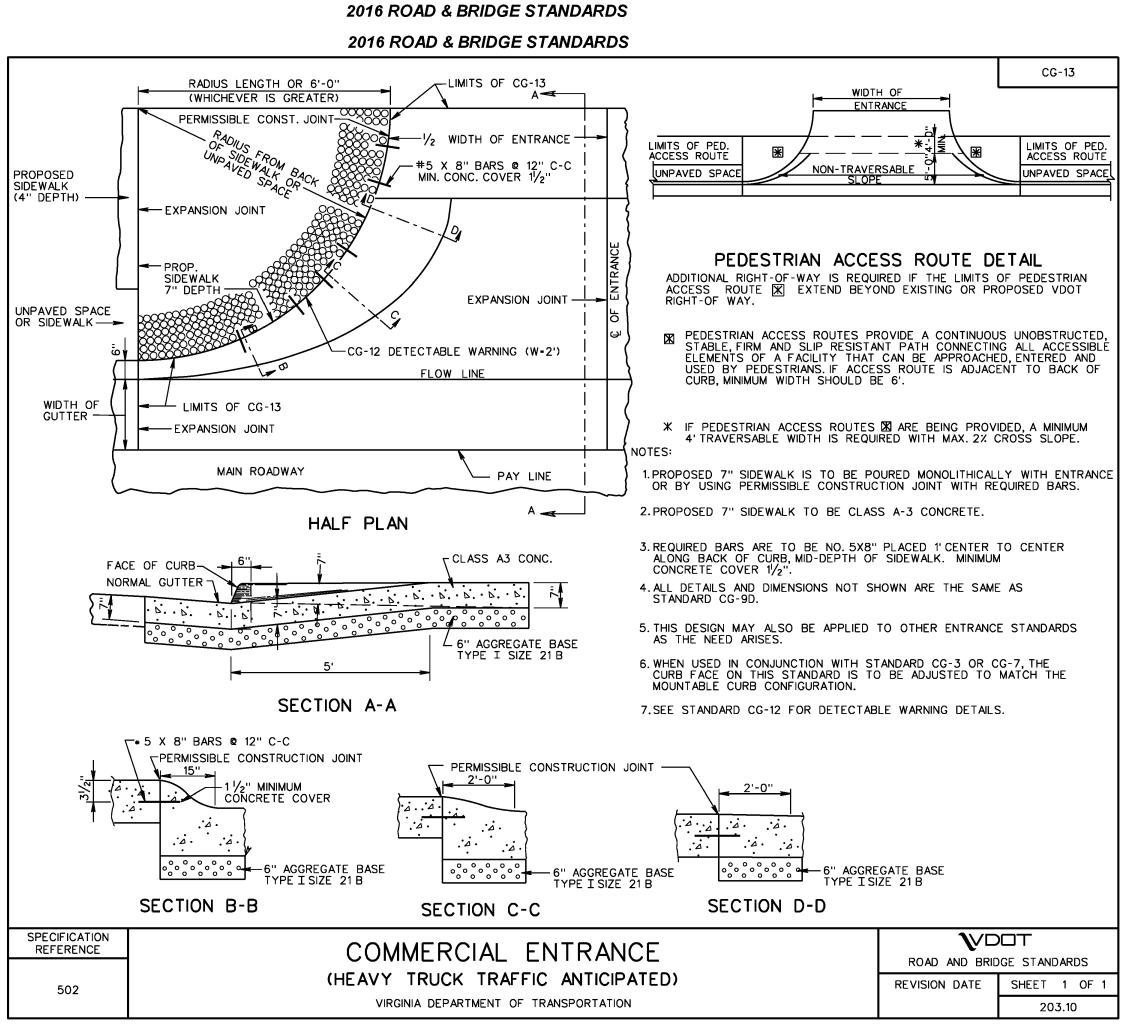
ACCESSIBILITY NOTES

- ALL GRADES/SLOPES SHOWN ON THIS PLAN WERE DESIGNED AT OR BELOW MAXIMUMS ALLOWED BY ADA. IT IS THE CONTRACTOR'S RESPONSIBILITY TO FAMILIARIZE THEMSELVES WITH ADA ACCESSIBILITY GUIDELINES, AND OTHER APPLICABLE ACCESSIBILITY GUIDELINES. IN THE EVENT THAT A DESIGN QUESTION SHOULD ARISE, OR A FIELD CONDITION PRESENT ITSELF THAT IS DIFFERENT FROM THOSE SHOWN ON THESE PLANS, WORK SHOULD CEASE AND THE ENGINEER SHOULD BE NOTIFIED SO THAT AN ACCEPTABLE SOLUTION CAN BE DETERMINED
- THE CONTRACTOR IS ADVISED TO CAREFULLY CHECK ALL THE PHASES OR WORK RELATING TO ADA ACCESSIBILITY FOR THIS PROJECT. SINCE THE CODE DOES NOT ALLOW FOR CONSTRUCTION TOLERANCE, ANY CONSTRUCTION THAT EXCEEDS MAXIMUM OR MINIMUM DIMENSIONS AND SLOPES AS REQUIRED BY ANSI ARE SUBJECT TO REJECTION AND MAY BE REQUIRED TO BE REMOVED AND REPLACED AT THE CONTRACTOR'S EXPENSE.
- 3. SINCE THE CIVIL ENGINEER OR SURVEYOR CANNOT CONTROL THE EXACT METHODS OR MEANS USED BY THE GENERAL CONTRACTOR OR THEIR SUBCONTRACTORS DURING GRADING AND CONSTRUCTION OF THE PROJECT, THE CIVIL ENGINEER OR SURVEYOR ASSUMES NO RESPONSIBILITY FOR THE FINAL ACCEPTANCE OF ADA ACCESSIBILITY RELATED ITEMS (OR OTHER ACCESSIBILITY CODES) BY THE STATE AND COUNTY, ANY OTHER AUTHORITY, OR OTHER AFFECTED PARTIES.

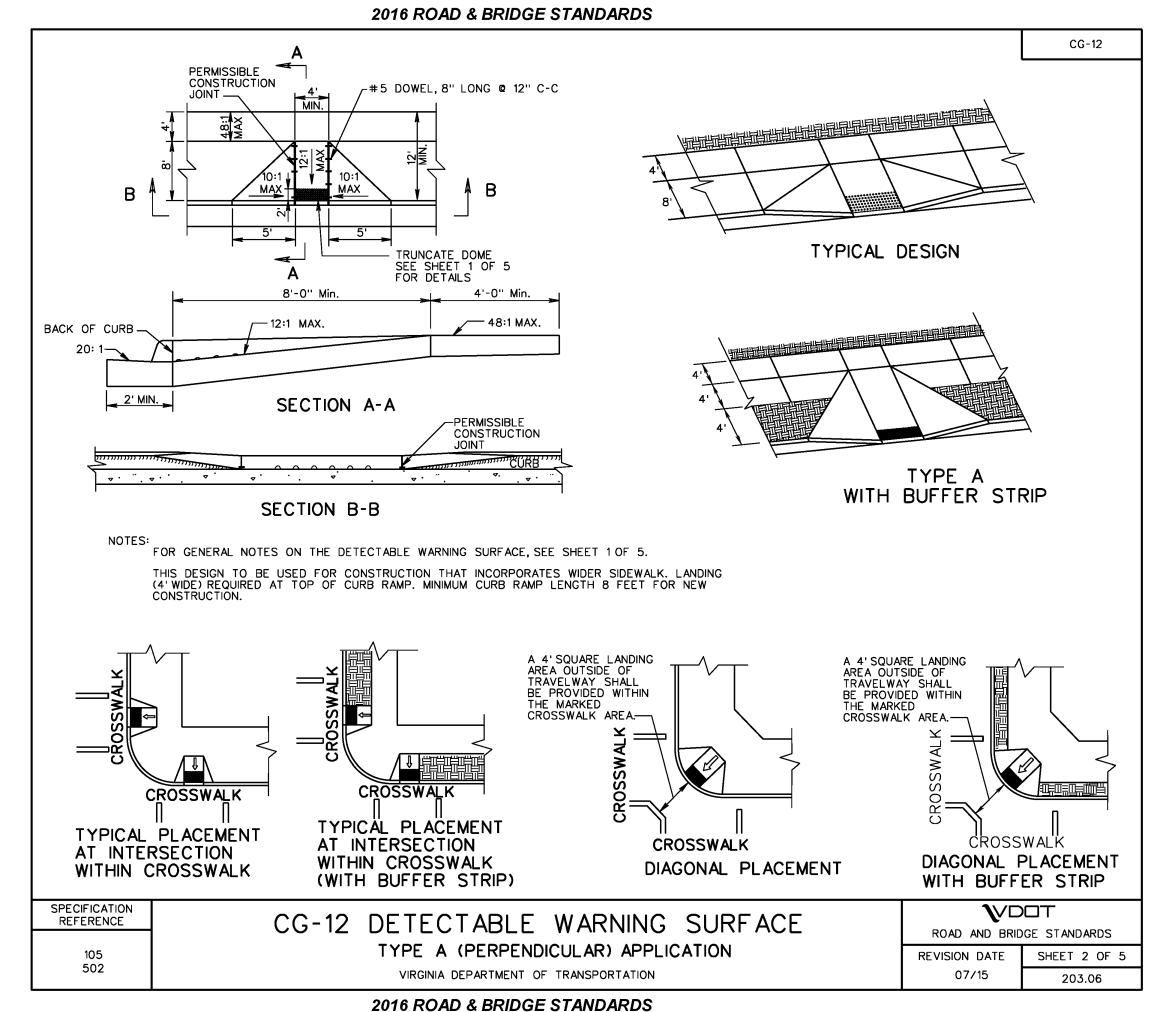


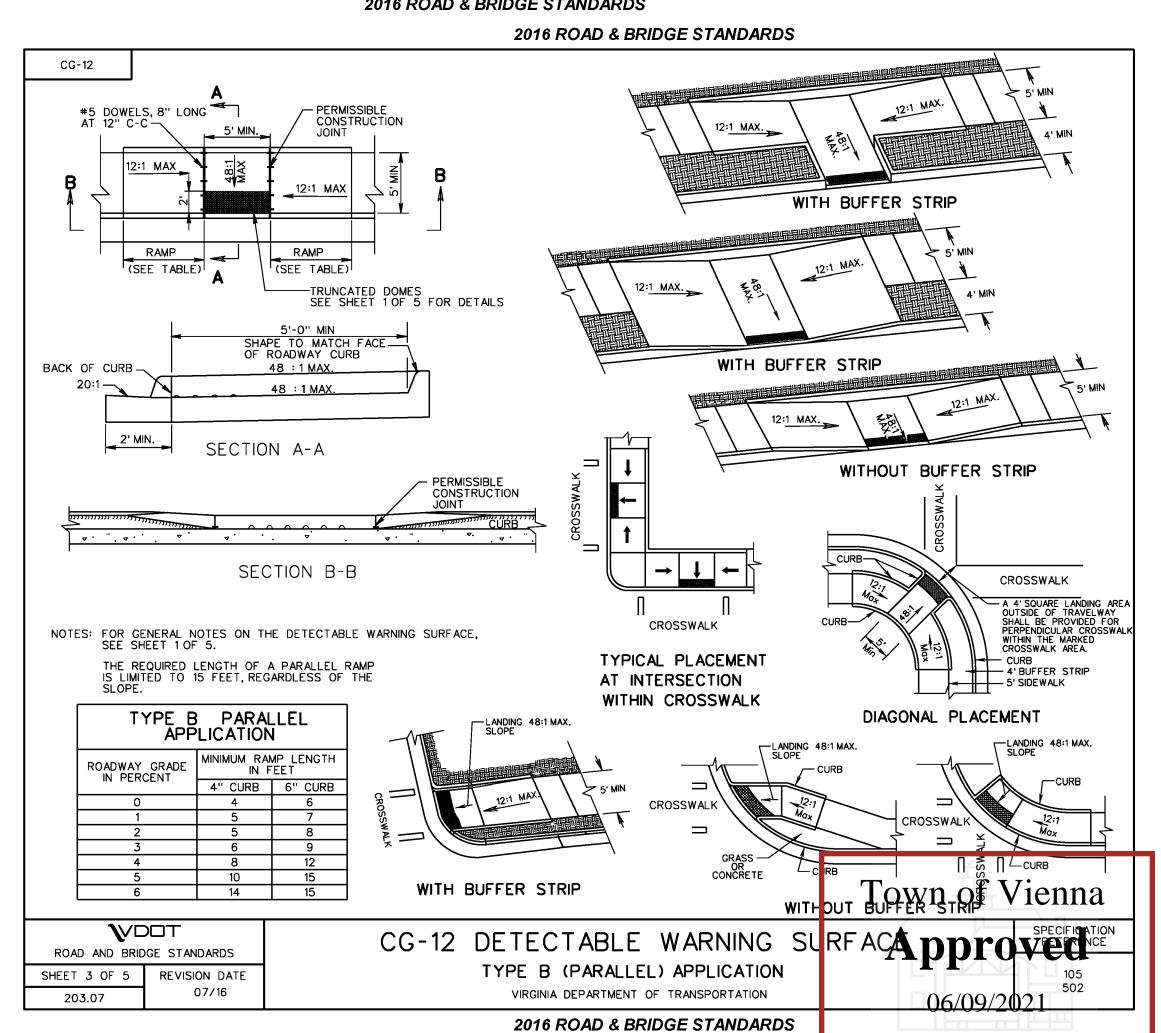






2016 ROAD & BRIDGE STANDARDS





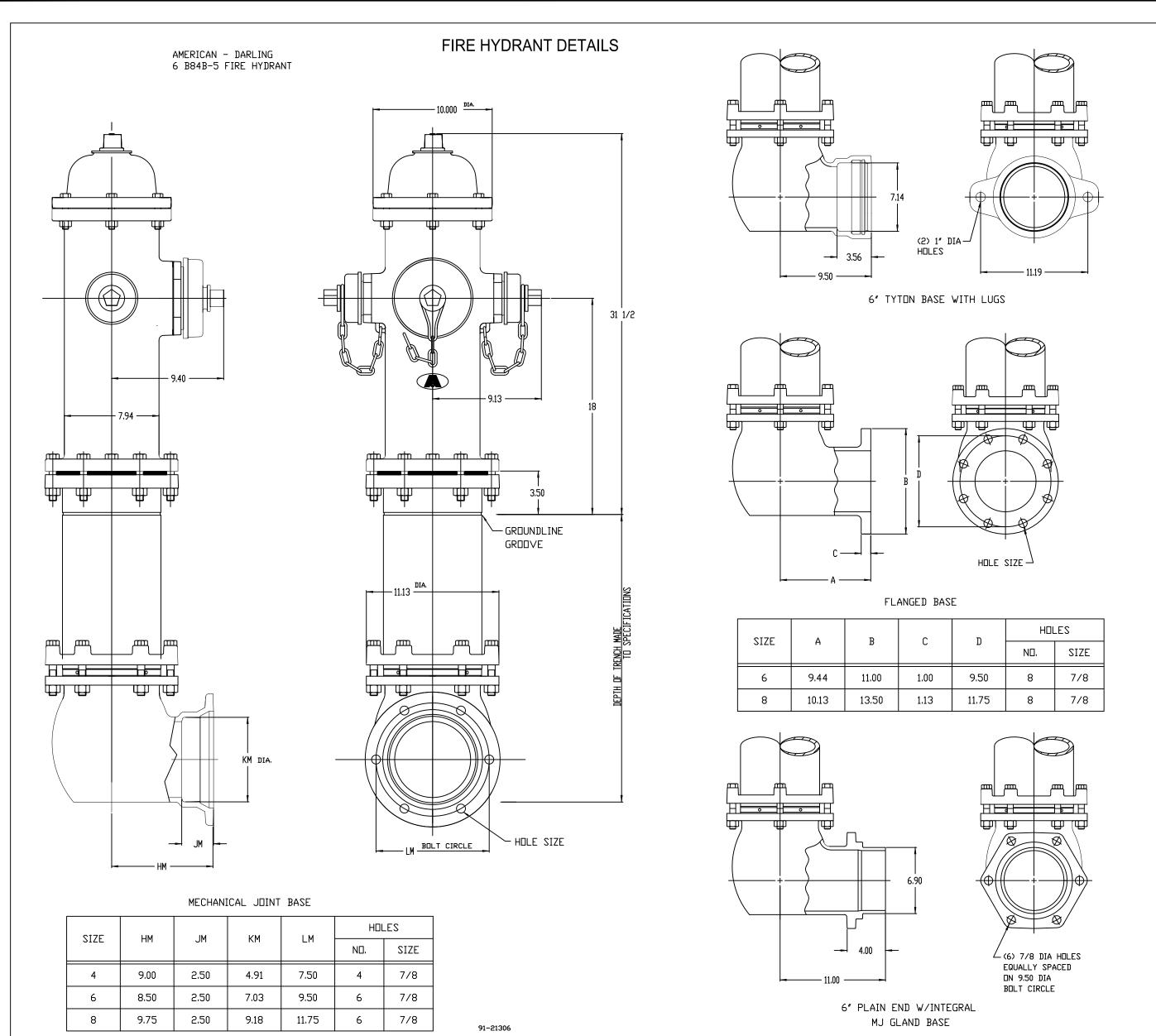
ALL DETAILS PROVIDED ON THIS SHEET ARE CURRENT AT TIME OF PLAN PREPARATION. CONTRACTOR IS RESPONSIBLE FOR USING CURRENT DETAILS AT TIME OF CONSTRUCTION.

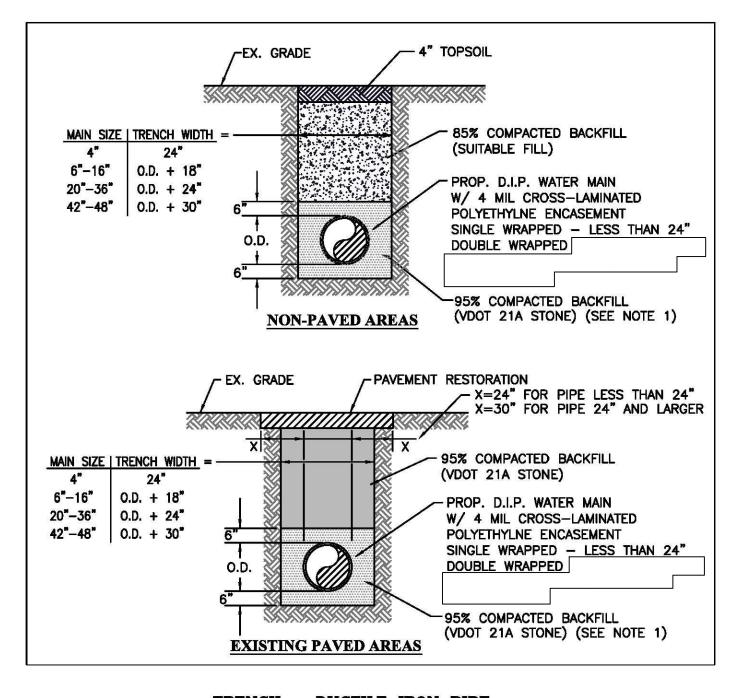
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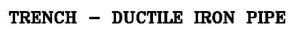
VIRGINIA

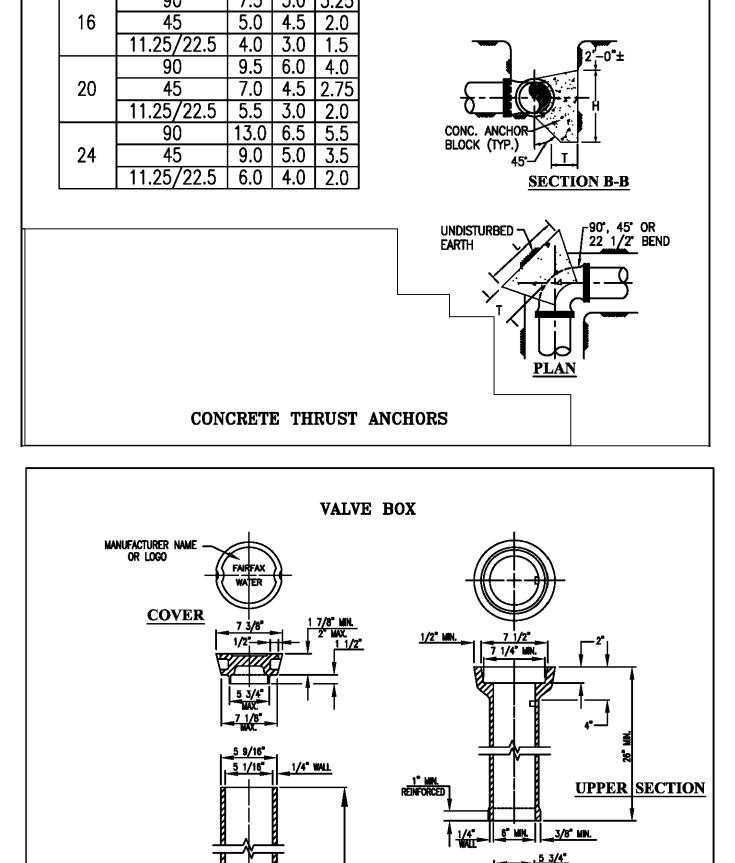
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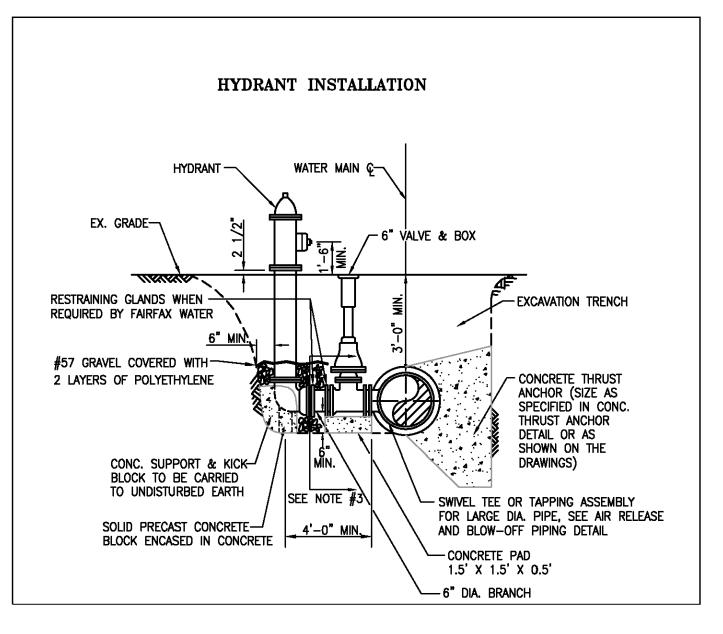
SIZE INCHES

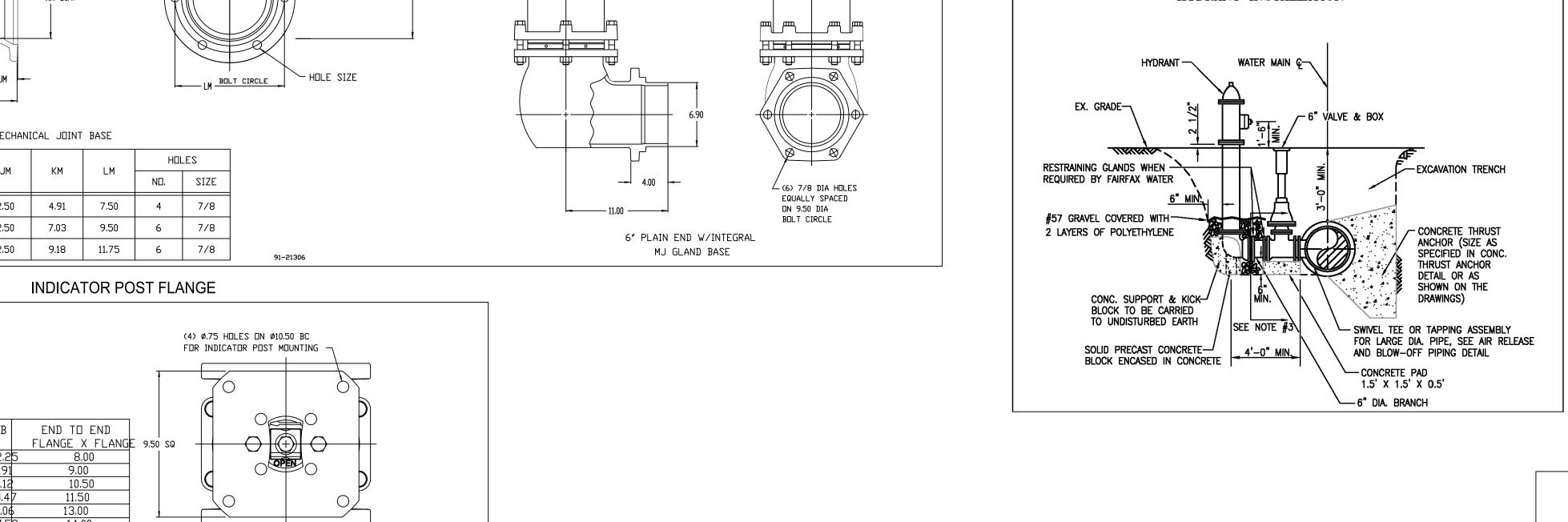
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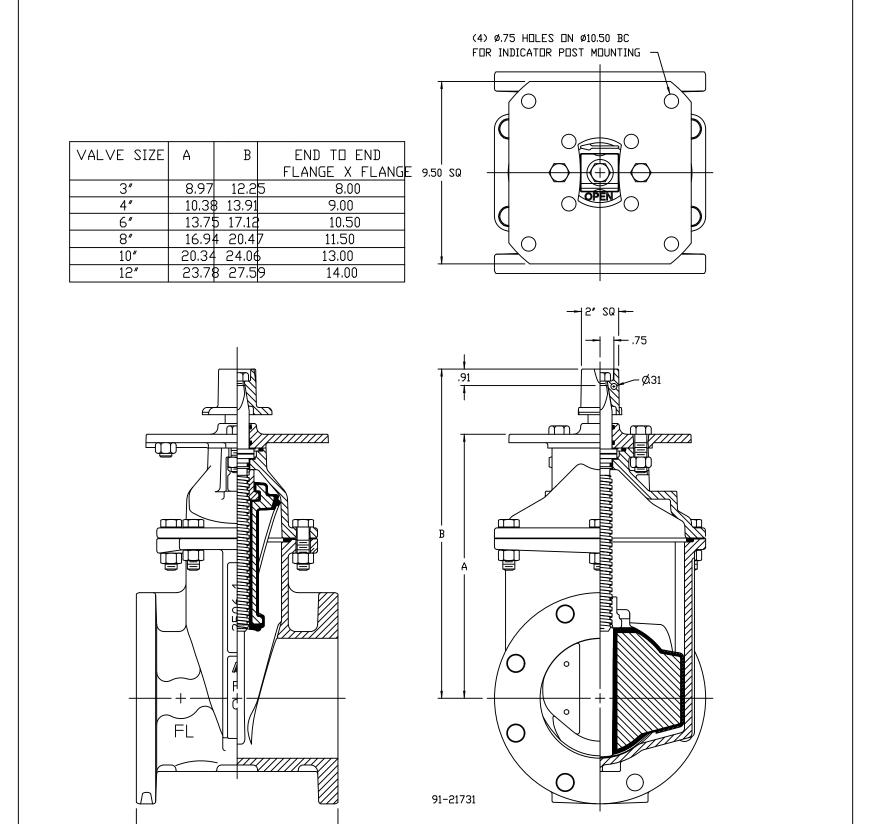
22.5

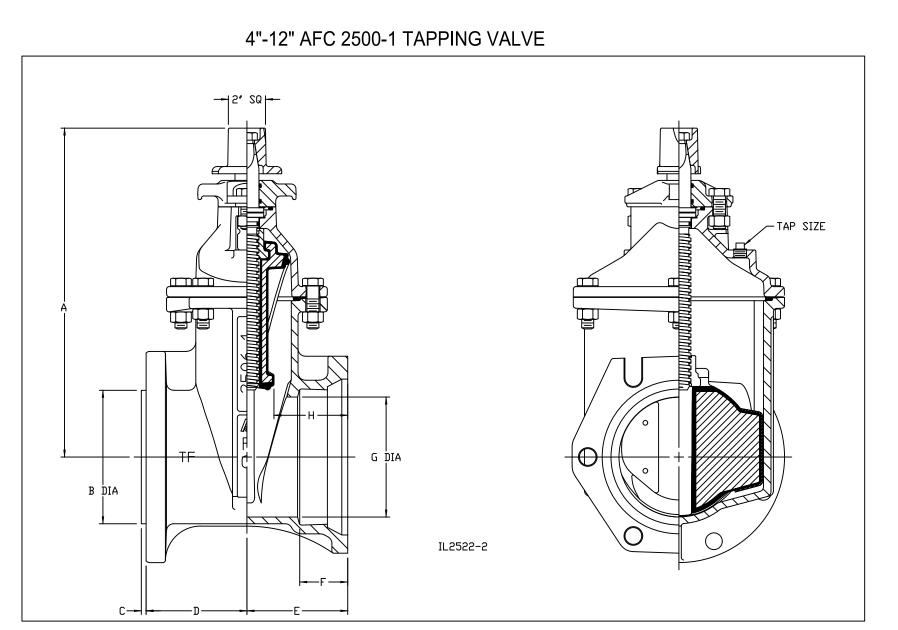
5.5 4.0 2.25

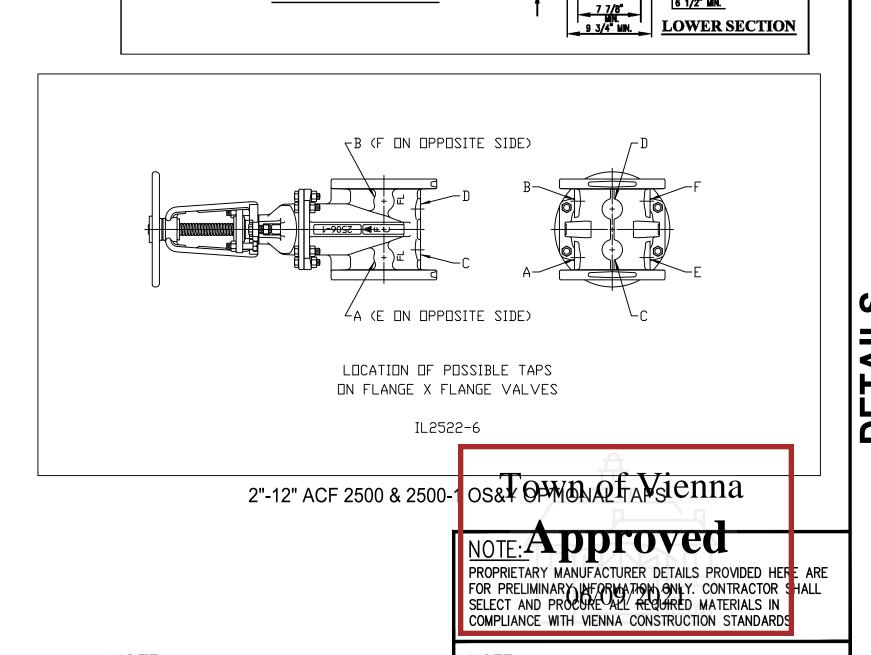
SECTION A-A











6 7/8"

ALL WATER SYSTEMS MUST BE BUILT TO THE LATEST FAIRFAX WATER CONSTRUCTION PRACTICE MANUAL

AND THE TOWN'S PUBLIC INFRASTRUCTURE MANUAL.

EXTENSION SECTION

ALL DETAILS PROVIDED ON THIS SHEET ARE CURRENT AT TIME OF PLAN PREPARATION. CONTRACTOR IS RESPONSIBLE FOR USING CURRENT DETAILS AT TIME OF CONSTRUCTION.

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WES

VIRGINIA

– END TO END ——

- EXISTING OR PROPOSED SIDEWALK. SEE NOTE 15

- BACK OF SIDEWALK

DI-2A, 2B, 2C

WEEP HOLE

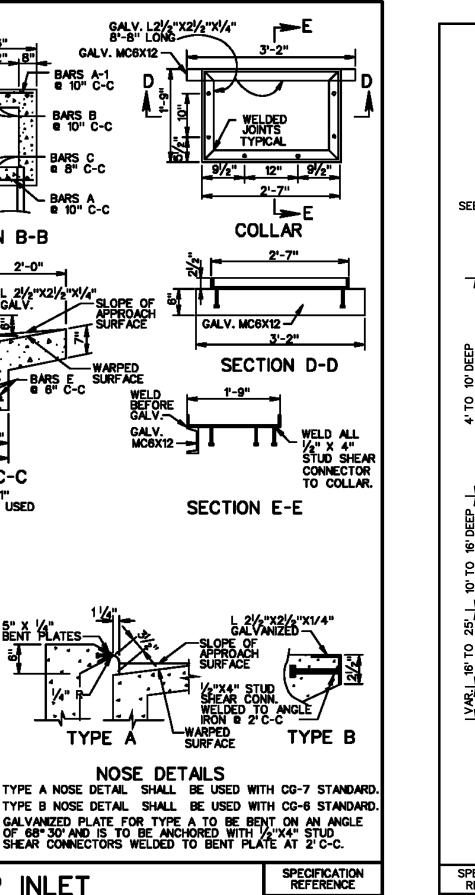
FRONT ELEVATION

FOR USE ON GRADES

FOR USE IN SAGS BOTH SIDES TO BE SYMMETRICAL

ROAD AND BRIDGE STANDARDS

SHEET 1 OF 2 REVISION DATE



233 302

Rev. 1-00, 11-05, 2011 Reprint, 2018

2016 ROAD & BRIDGE STANDARDS

STANDARD CURB DROP INLET

12" - 24" PIPE: MAXIMUM DEPTH (H) = 9'

VIRGINIA DEPARTMENT OF TRANSPORTATION

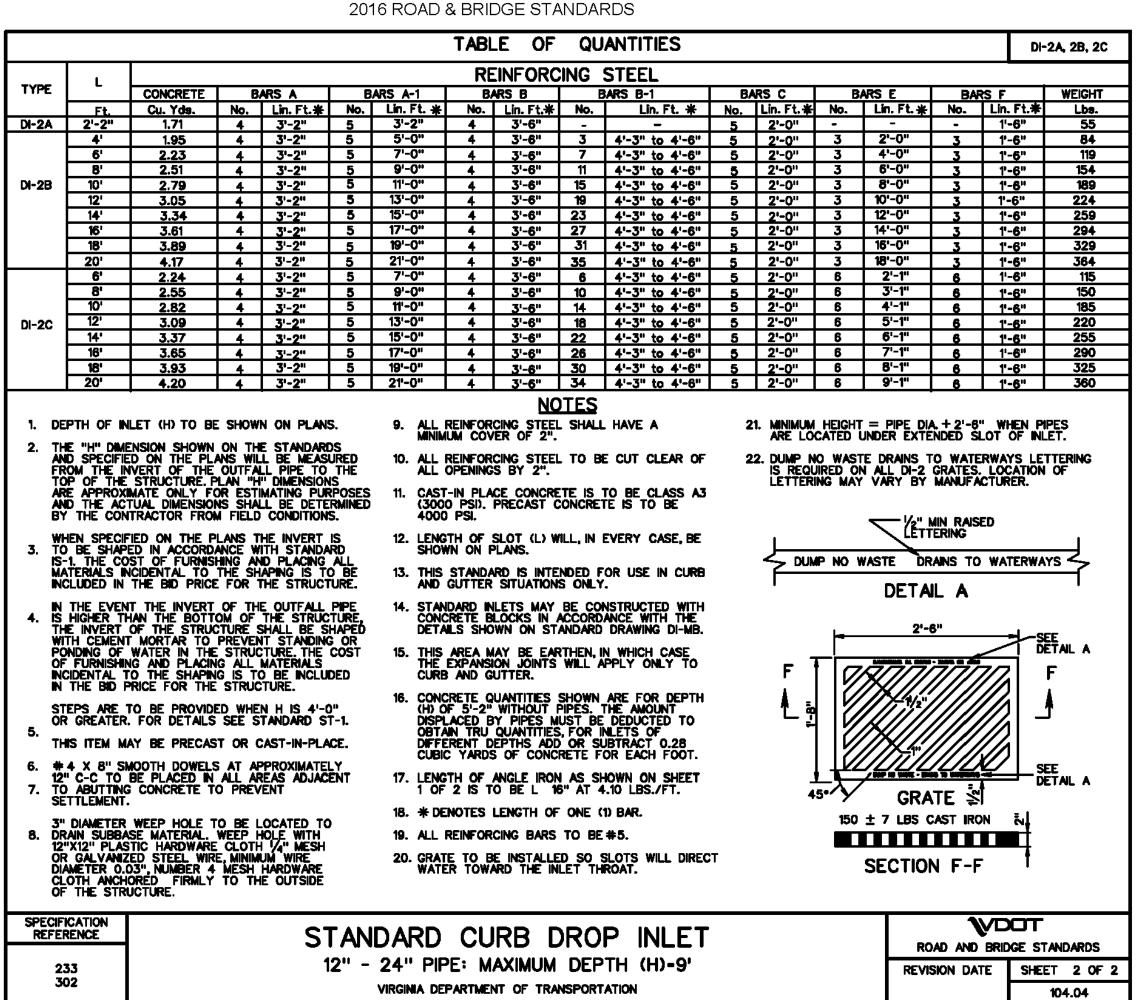
2'-0'

SECTION C-C

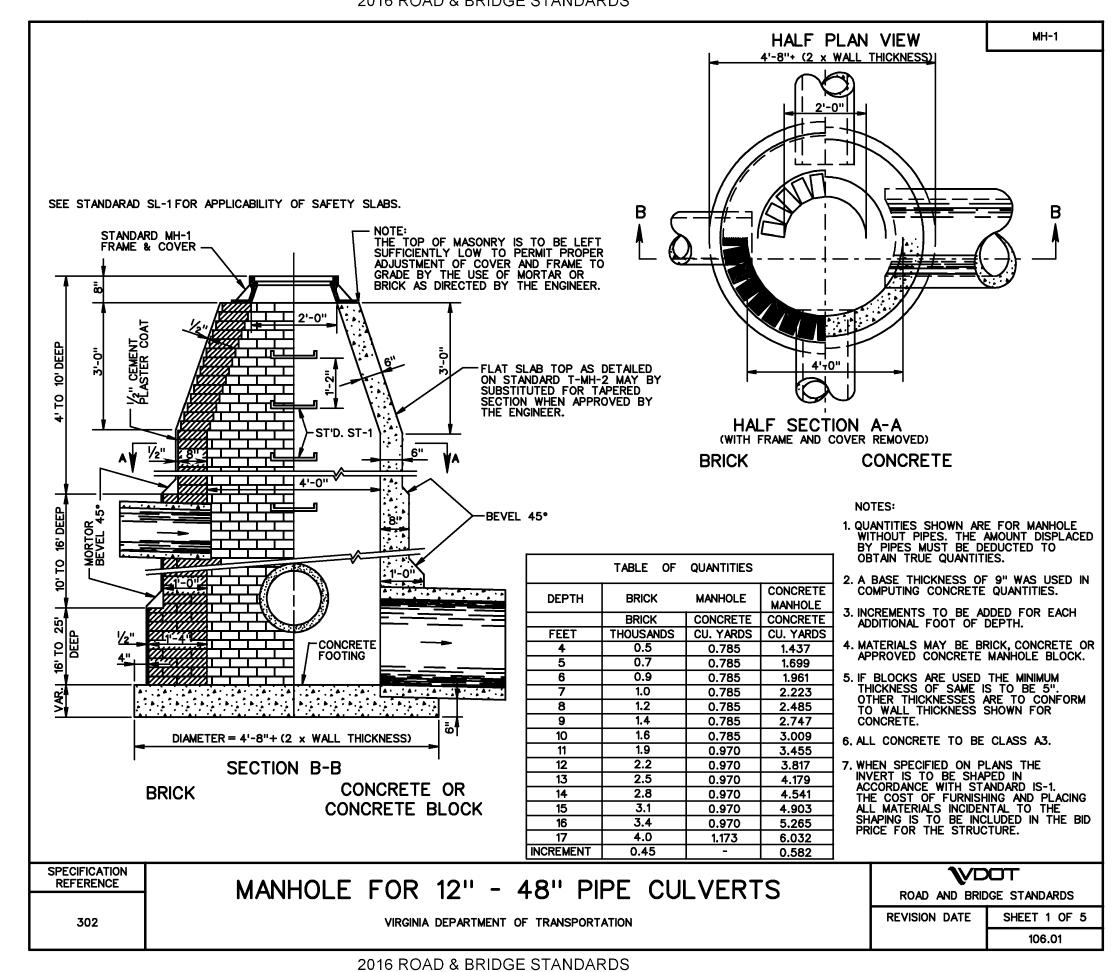
△DECREASE TO 1"
WHEN INLET IS USED

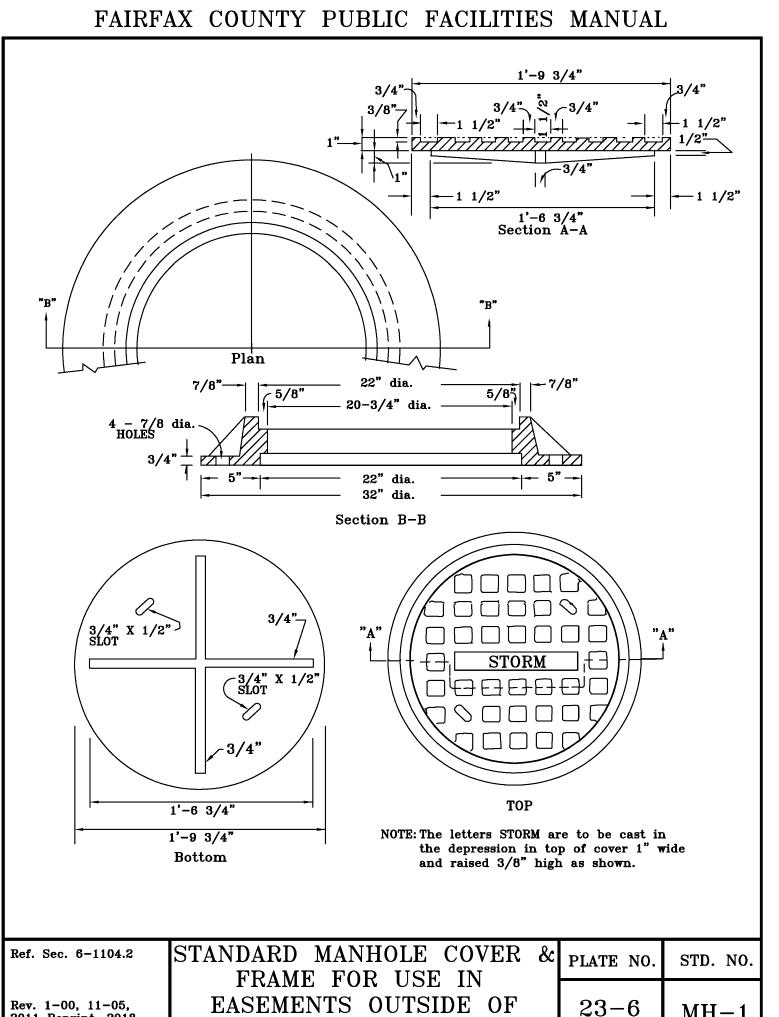
WITH MEDIANS.

SECTION A-A FOR STEP DETAILS SEE STD. ST-1.



2016 ROAD & BRIDGE STANDARDS





VDOT RIGHT-OF-WAY



ALL DETAILS PROVIDED ON THIS SHEET ARE CURRENT AT TIME OF PLAN PREPARATION. CONTRACTOR IS RESPONSIBLE FOR USING CURRENT DETAILS AT TIME OF CONSTRUCTION.

VIRGINI

0

TOWN

MH-1

23-6

4

寸

BARS C @ 12" C-C

SECTION B-B

JUNCTION BOX CHAMBER DETAILS FOR 48" - 72" PIPE CULVERTS MAX. DEPTH (H) - 20'

VIRGINIA DEPARTMENT OF TRANSPORTATION

2016 ROAD & BRIDGE STANDARDS

TWO BARS K TO BE USED IN EACH CORNER WITH CIRCULAR OPENING IN TOP.

BARS A 8 8" C-C N.F. BARS G 8" C-C F.F.

BARS J @ 6" C-C E.F.

- GALVANIZED S5X10 TO BE PLACED UNDER TOP SLAB AND EXTENDED INTO SIDE WALLS TO WITHIN 2" OF OUTSIDE FACE.

""W" DENOTES BOTH THE INTERNAL CHAMBER WIDTH ALONG THE NARROW AXIS AND THE DIMENSION OF THE OPENING IN THE TOP SLAB OF CHAMBER

←1 BAR F E.F.-E.S.

PLAN VIEW

(TOP SLAB REINFORCEMENT)

SECTION A-A

TYPE A, B, AND C TOWER. SEE SHEET 3 OF 4 FOR DETAILS.—

PIPE ...

VDOT

ROAD AND BRIDGE STANDARDS

SHEET 1 OF 4 REVISION DATE

106.10

1. ALL REINFORCING STEEL TO BE #6 BARS WITH A MINIMUM 11/2"

2. ANY REINFORCING BARS IN CONFLICT WITH PIPE SHELLS ARE TO BE CUT A MINIMUM OF $1\frac{1}{2}$ " FROM PIPE.

3. FOR DETAILS METHOD OF TURNING ANGLES AND APPROXIMATE QUANTITIES SEE SHEET 2 OF 4.

4. PRECAST CHAMBERS OF THE BOLT TOGETHER TYPE MAY BE SUBSTITUTED WHEN APPROVED ON AN INDIVIDUAL BASIS.

5. ALL CONCRETE TO BE CLASS A3 IF CAST IN PLACE, 4000 PSI IF PRECAST.

8. WHEN SPECIFIED ON PLANS INVERT IS TO BE SHAPED IN ACCORDANCE WITH STANDARD IS-1. THE MATERIALS INCIDENTAL TO THE SHAPING ARE NOT TO BE MEASURED OR PAID SEPARATELY BUT THE COST OF SAME IS TO BE INCLUDED IN THE PRICE BID PER CUBIC YARD OF JUNCTION BOX CONCRETE

_ D + 1'-1" _

SPECIFICATION REFERENCE

233 302

MINCREASE INDICATED DIMENSION BY 1'
FOR USE WITH 72" PIPE.

THE LENGTH OF ALL BARS AND THE NUMBER REQUIRED WILL VARY WITH THE SIZE OF CHAMBER.
FOR APPROXIMATE WEIGHT SEE SHEET 2 AND 4 OF THIS STANDARD.

BARS B, C, E, G, H, AND J ARE STRAIGHT.

8. THIS ITEM MAY BE PRECAST OR CAST IN PLACE.

15°

QUANTITIES FOR CHAMBER *

66" 6'-7" 6.46 2028 6'-10" 6.63 2069 7'-7" 7.12 2220 9'-4" 8.27 2510 72" 7'-2" 7.77 2248 7'-5" 7.95 2306 8'-3" 8.56 2490 10'-2" 9.97 2843

54" 5'-5" 4.61 1308 5'-7" 4.70 1373 6'-3" 5.07 1452 7'-8" 5.85 1619

60" 6'-0" 5.19 1487 6'-3" 5.33 1531 6'-11" 5.71 1651 8'-6" 6.63 1925

66" 6'-7" 5.79 1815 6'-10" 5.94 1934 7'-7" 6.40 2058 9'-4" 7.45 2359

72" 7'-2" 7.00 2020 7'-5" 7.17 2086 8'-3" 7.73 2264 10'-2" 9.03 2585

60" 6'-0" 4.54 1337 6'-3" 4.67 1381 6'-11" 5.02 1484 8'-6" 5.85 1710 66" 6'-7" 5.09 1592 6'-10" 5.22 1674 7'-7" 5.64 1789 9'-4" 6.59 2062 72" 7'-2" 6.19 1835 7'-5" 6.34 1885 8'-3" 6.86 2050 10'-2" 8.05 2355

_ | 10 |

CU. YDS. LBS. REINF. DIM. CU. YDS. LBS. REINF. DIM. CU. YDS. LBS. REINF. DIM. CU. YDS. LBS. REINF

STEEL D CONC. STEEL D CONC. STEEL D CONC. STEEL

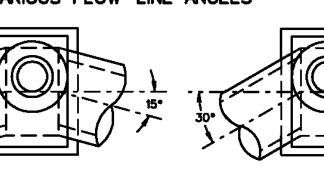
1293 | 5'-0" | 4.65 | 1317 | 5'-7" | 4.99 | 1412 | 6'-10" | 5.72 | 1602
 1466
 5'-7"
 5.26
 1558
 6'-3"
 5.10
 1617
 7'-8"
 6.53
 1798

1657 | 6'-3" | 5.96 | 1794 | 6'-11" | 6.38 | 1831 | 8'-6" | 7.38 | 2064

1149 | 5'-0" | 4.15 | 1173 | 5'-7" | 4.46 | 1274 | 6'-10" | 5.12 | 1440

1030 | 5'-0" | 3.61 | 1052 | 5'-7" | 3.88 | 1133 | 6'-10" | 4.47 | 1294 1177 | 5'-7" | 4.10 | 1236 | 6'-3" | 4.43 | 1305 | 7'-8" | 5.14 | 1454

ANGLE OF TURN



SEE SHEET 4 OF 4.

30°

61° - 90°

WHERE PIPES ENTER
THE CHAMBER ON A
SKEW, PIPE WALLS ARE
TO BE EXTENDED, IF
REQUIRED, INTO THE
CHAMBER A SUFFICIENT
DISTANCE TO MAINTAIN
A MINIMUM UNCUT
LENGTH OF 2' IN A 6' PIPE SECTION.

JB-1

QUANTITIES SHOWN ARE FOR CHAMBER WITHOUT PIPES.
PIPE DISPLACEMENT OF CONCRETE AND STEEL MUST BE
DEDUCTED TO OBTAIN TRUE QUANTITIES. CHAMBER AND TOWER QUANTITIES FOR CONCRETE AND REINFORCING STEEL MUST BE ADDED TO OBTAIN TOTALS. 40 Lbs.

"W" STRUCT. STEEL 2'-0" 30 Lbs. 3'-0"

4'-0" 50 Lbs.

ROAD AND BRIDGE STANDARDS REVISION DATE | SHEET 2 OF 4 106.11

2016 ROAD & BRIDGE STANDARDS

VIRGINIA DEPARTMENT OF TRANSPORTATION

JUNCTION BOX DETAILS FOR ANGULAR

CONNECTIONS OF 48" - 72" PIPE CULVERTS

31° - 60°

2016 ROAD & BRIDGE STANDARDS

LYETHYLENE CORRUGATED PIPE (PE)				
AMETER	AREA		MUM OF COVER ET	
NCHES	SQ. FT.	TYPE C	TYPE S	
12	0.8	23	20	
15	1.2	23	19	
18	1.8	19	19	
24	3.1	16	15	
30	4.9		13	
36	7.1	_	12	
42	9.6	_	10	
48	12.6	_	10	
54	15.9	_	10	

SPECIFICATION REFERENCE

105 233 302

PC-1

DLYVINYLCHLO	RIDE PROFILE V	VALL PIPE (PVC)
DIAMETER	AREA	MAXIMUM HEIGHT OF COVER
INCHES	SQ. FT.	FEET
18	1.7	41
21	2.3	40
24	3.0	37
30	4.7	34
36	6.9	34

POLYPROPYLENE PIPE (PP)								
DIAMETER	FEET							
INCHES	SQ. FT.	TYPE S TYPE D						
12	0.8	25	_					
15	1.2	24 —						
18	1.8	24	_					
24	3.1	20 —						
30	4.9	16 —						
36	7.1	14	16					
42	9.6	14	16					
48	12.6	13	16					
60	19.6	12	15					

NOTES:

- COVER HEIGHTS INDICATED IN TABLES ARE FOR FINISHED CONSTRUCTION, USING AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS. TO PROTECT PIPE DURING CONSTRUCTION, MINIMUM HEIGHT OF COVER TO BE IN ACCORDANCE WITH TABLE A PRIOR TO ALLOWING CONSTRUCTION TRAFFIC TO CROSS INSTALLATION. THE COVER SHALL EXTEND THE FULL LENGTH OF THE PIPE. THE APPROACH FILL IS TO EXTEND A MINIMUM OF 10(DIAMETER + $\frac{1}{2}$ DIAMETER) ON EACH SIDE OF THE PIPE OR TO THE INTERSECTION WITH A CUT.
- . STANDARD MINIMUM FINISHED HEIGHT OF COVER FOR ALL PIPES, EXCEPT THOSE UNDER ENTRANCES, SHALL BE 2.0' OR 1/2 DIAMETER WHICHEVER IS GREATER. FOR 12" THROUGH 48" DIAMETER PIPE INSTALLATIONS WHERE THE COVER HEIGHTS CANNOT BE ACHIEVED, AN ABSOLUTE MINIMUM FINISHED COVER HEIGHT OF 1.0' WILL BE ALLOWED ONLY IF ALL POSSIBLE MEANS TO OBTAIN THE STANDARD VALUE HAVE BEEN EXHAUSTED. THE MINIMUM FINISHED HEIGHT OF COVER FOR PIPES UNDER ENTRANCES IS 9" FOR PIPE DIAMETERS LESS THAN OR EQUAL TO 24", AND 12" FOR PIPE DIAMETERS GREATER THAN 24". WHERE THE SURFACE OVER THE TOP OF THE PIPE WILL BE ASPHALT, A MINIMUM OF 6" OF CLASS I BACKFILL MATERIAL IS TO BE PLACED BETWEEN THE TOP OF THE PIPE AND THE BOTTOM OF THE ASPHALT.
- SEE STANDARD PB-1 FOR PIPE BEDDING AND BACKFILL REQUIREMENTS. LARGE CULVERTS SHALL BE DESIGNED BY AN ENGINEER, REGISTERED IN THE COMMONWEALTH OF VIRGINIA, AND SHALL BE DESIGNED IN ACCORDANCE WITH THE REQUIREMENTS OF VOLUME V, PART 2 OF THE MANUAL OF THE STRUCTURE AND BRIDGE DIVISION. A LARGE CULVERT IS ANY CULVERT THAT WILL BECOME PART OF THE STRUCTURE AND BRIDGE INVENTORY. THE GEOMETRIC DEFINITION OF THESE STRUCTURES IS PROVIDED IN THE CURRENT VERSION OF VDOT'S IM-S&B-27.

TYPE C - SINGLE WALL PIPE (CORRUGATED WALL ONLY) TYPE S - DOUBLE WALL PIPE (CORRUGATED WALL WITH SMOOTH INNER WALL)

TYPE D - TRIPLE WALL PIPE (CORRUGATED WALL BETWEEN SMOOTH INNER AND OUTER WALL)

BLE A
MINIMUM COVER HEIGHT DURING CONSTRUCTION (SEE NOTE 2)
18"
√₂ DIAMETER

SPECIFICATION REFERENCE A COPY OF THE ORIGINAL SEALED AND SIGNED STANDARD DRAWING IS ON FILE IN THE CENTRAL OFFICE ****VDOT PLASTIC PIPE ROAD AND BRIDGE STANDARDS HEIGHT OF COVER TABLES FOR HL-93 LIVE LOAD 232 302 SHEET 15 OF 18 REVISION DATE 04/19 VIRGINIA DEPARTMENT OF TRANSPORTATION 107.19

2016 ROAD & BRIDGE STANDARDS

Town of Vienna 06/09/2021

ALL DETAILS PROVIDED ON THIS SHEET ARE CURRENT AT TIME OF PLAN PREPARATION. CONTRACTOR IS RESPONSIBLE FOR USING CURRENT DETAILS AT TIME OF CONSTRUCTION.

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N N O

File No. FM-18 Tax Map No. 038-3 Job No. 07-036 Cadd Dwg. File: Q: \sdskproj\07036\dwg\Engineering\site Plan\07036C-0201.dwg

Xref: site plan\07036B-0002

SHEET: **C-0206**

Introduction

for project specific designs.

The information in this document is designed to provide answers to general cover height questions; the data provided is not

intended to be used for project design. The design procedure described in the Structures section (Section 2) of the Drainage Handbook provides detailed information for analyzing most common installation conditions. This procedure should be utilized

The two common cover height concerns are minimum cover in areas exposed to vehicular traffic and maximum cover heights.

Pipe diameters from 12- through 48-inch (300-1200 mm) installed in traffic areas (AASHTO H-20, H-25, or HL-93 loads) must

have at least one foot (0.3m) of cover over the pipe crown, while 60-inch (1500 mm) pipes must have at least 24 inches (0.6m)

of cover. The backfill envelope must be constructed in accordance with the Installation section (Section 5) of the Drainage

Appendix A-5, Table A-5-2 of the Drainage Handbook. In Table 1 below, this condition is represented by a Class III material

material can provide similar strength at slightly lower levels of compaction. Structural backfill material should extend to the

crown of the pipe; the remaining cover should be appropriate for the installation and as specified by the design engineer. If

sub-base material can be considered in the minimum burial depth. While rigid pavements can be included in the minimum

settlement or rutting is a concern, it may be appropriate to extend the structural backfill to grade. Where pavement is involved,

Additional information that may affect the cover requirements is included in the Installation section (Section 5) of the Drainage

paving equipment and similar loads that are less than the design load, the potential of pipe flotation, and the type of surface

Handbook. Some examples of what may need to be considered are temporary heavy equipment, construction loading,

compacted to 95% standard Proctor density or a Class II material compacted to 90% standard proctor density, although other

Handbook and the requirements of ASTM D2321. The backfill envelope must be of the type and compaction listed in

Either may be considered "worst case" scenario from a loading perspective, depending on the project conditions.

Minimum Cover in Traffic Applications

cover, the thickness of flexible pavements should not be included in the minimum cover.

TN 2.04

March 2018

Pipe Requirements

ADS, Inc. Drainage Handbook

ADS HP Storm pipe shall have a smooth interior and annular exterior corrugations.

• 12- through 60-inch (300 to 1500 mm) pipe shall meet ASTM F2881 or AASHTO M330 • Manning's "n" value for use in design shall be 0.012

Pipe shall be joined using a bell & spigot joint meeting the requirements of ASTM F2881 or AASHTO M330. The joint shall be watertight according to the requirements of ASTM D3212. Gaskets shall meet the requirements of ASTM F477. Gasket shall be installed by the pipe manufacturer and covered with a removable, protective wrap to ensure the gasket is free from debris. A joint lubricant available from the manufacturer shall be used on the gasket and bell during assembly. 12- through 60-inch (300 to 1500 mm) diameters shall have an exterior bell wrap installed by the manufacturer.

Fittings shall conform to ASTM F2881 or AASHTO M330. Bell and spigot connections shall utilize a welded or integral bell and valley or inline gaskets meeting the watertight joint performance requirements of ASTM

Field Pipe and Joint Performance

To assure watertightness, field performance verification may be accomplished by testing in accordance with ASTM F1417 or ASTM F2487. Appropriate safety precautions must be used when field-testing any pipe material. Contact the manufacturer for recommended leakage rates.

Polypropylene compound for pipe and fitting production shall be impact modified copolymer meeting the material requirements of ASTM F2881, Section 5 and AASHTO M330, Section 6.1.

Installation shall be in accordance with ASTM D2321 and ADS recommended installation guidelines, with the exception that minimum cover in traffic areas for 12- through 48-inch (300 to 1200 mm) diameters shall be one foot (0.3 m) and for 60-inch (1500 mm) diameter the minimum cover shall be 2 ft. (0.6 m) in single run applications. Backfill for minimum cover situations shall consist of Class 1 (compacted), Class 2 (minimum 90% SPD), or Class 3 (minimum 95%) material. Maximum fill heights depend on embedment material and compaction level; please refer to Technical Note 2.04. Contact your local ADS representative or visit our website at www.ads-pipe.com for a copy of the latest installation guidelines.

P	ipe	Dimensions
- 1		Minustral Disc

Nominal Pipe I.D.	12	15	18	24	30	36	42	48	60
in (mm)	(300)	(375)	(450)	(600)	(750)	(900)	(1050)	(1200)	(1500)
Average Pipe I.D.	12.2	15.1	18.2	24.1	30.2	36.0	42.0	47.9	59.9
in (mm)	(310)	(384)	(462)	(612)	(767)	(914)	(1067)	(1217)	(1521)
Average Pipe O.D.	14.5	17.7	21.4	28.0	35.5	41.5	47.4	54.1	67.1
in (mm)	(368)	(450)	(544)	(711)	(902)	(1054)	(1204)	(1374)	(1704)
Minimum Pipe Stiffness *	75	60	56	50	46	40	35	35	30
@ 5% Deflection #/in./in. (kN/m²)	(517)	(414)	(386)	(345)	(317)	(276)	(241)	(241)	(207)

Minimum Cover Requirements for ADS HP Storm with AASHTO H-25, H-20, or HL-93 Load

Inside Dlameter, ID, in.(mm)	Minimum Cover ft. (m)	Inside Diameter, ID, in.(mm)	Minimum Cover ft. (m)
12 (300)	1 (0.3)	36 (900)	1 (0.3)
15 (375)	1 (0.3)	42 (1050)	1 (0.3)

- 1. Minimum covers presented here were calculated assuming Class III backfill material compacted to 95% standard Proctor density or Class II backfill material compacted to 90% standard Proctor density around the pipe, as recommended in Section 5 of the Drainage Handbook, with an additional layer of compacted traffic lane sub-base for a total cover as required. In shallow traffic installations, especially where pavement is involved, a good quality compacted material to
- grade is required to prevent surface settlement and rutting. The minimum covers specified do not include pavement thickness. A pavement section of 0.4' is typical. 3. Backfill materials and compaction levels not shown in the table may also be acceptable. Contact ADS for further detail.
- 4. Calculations assume no hydrostatic pressure and native soils that are as strong as the specified minimum backfill recommendations.

treatment which will be installed over the pipe zone.

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ATN204	©ADS 201

conservative AASHTO LRFD design factors represented in Structures section of the Drainage Handbook, use material properties consistent with the expected performance characteristics for HP Storm materials, as shown in Table 2, and assume the native (in-situ) soil is of adequate strength and suitable for installation. For applications requiring fill heights greater than

Maximum Cover

Wall thrust generally governs the maximum cover a pipe can withstand and conservative maximum cover heights will result when using the information presented in the Structures section (Section 2) of the Drainage Handbook. Table 2 below shows

the material properties consistent with the expected performance characteristics for HP Storm materials for a 100-year design

The maximum burial depth is highly influenced by the type of backfill and level of compaction around the pipe. General

maximum cover limits for ADS HP Storm use in storm drainage applications are shown in Tables 3 for a variety of backfill

Table 3 was developed assuming pipe is installed in accordance with ASTM D2321 and the Installation section (Section 5) of

the Drainage handbook. Additionally, the calculations assume no hydrostatic load around the pipe, incorporate the maximum

ADS HP Storm Mechanical Properties

Long Term

3.7

ADS HP Storm Pipe Trench Detail with Uniform Backfill

(Traffic and Non-Traffic Applications)

3,500

175,000

FLEXIBLE PAVEMENT. H

BACKFILL

Specification

ASTM F2881

Impact-modified

those shown in Table 3 or where hydrostatic pressure due to groundwater is expected, contact an ADS Engineer.

Specifications ♦ 1-15

Material Properties

Nominal Pipe I.D.	12	15	18	24	30	36	42	48	60
in (mm)	(300)	(375)	(450)	(600)	(750)	(900)	(1050)	(1200)	(1500)
Average Pipe I.D.	12.2	15.1	18.2	24.1	30.2	36.0	42.0	47.9	59.9
in (mm)	(310)	(384)	(462)	(612)	(767)	(914)	(1067)	(1217)	(1521)
Average Pipe O.D.	14.5	17.7	21.4	28.0	35.5	41.5	47.4	54.1	67.1
in (mm)	(368)	(450)	(544)	(711)	(902)	(1054)	(1204)	(1374)	(1704)
Minimum Pipe Stiffness *	75	60	56	50	46	40	35	35	30
@ 5% Deflection #/in./in. (kN/m²)	(517)	(414)	(386)	(345)	(317)	(276)	(241)	(241)	(207)

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4" FOR 12"-24" PIPE

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Maximum Cover for ADS HP Storm Pipe with Uniform Backfill, ft (m)

1. Results based on calculations shown in the Structures section of the ADS Drainage Handbook (v20.7). Calculations assume

3. For installations using lower quality backfill materials or lower compaction efforts, pipe deflection may exceed the 5% design

critical, pipe placement techniques or periodic deflection measurements may be required to ensure satisfactory pipe

. Backfill materials and compaction levels not shown in the table may also be acceptable. Contact ADS for further detail.

For projects where cover exceeds the maximum values listed above, contact ADS for specific design considerations.

limit; however controlled deflection may not be a structurally limiting factor for the pipe. For installation where deflection is

Material must be adequately "knifed" into haunch and in between corrugations. Compaction and backfill material is assumed

Installation assumed to be in accordance with ASTM D2321 and the Installation section of the Drainage Handbook.

no hydrostatic pressure and a density of 120 pcf (1926 kg/m³) for overburden material.

uniform throughout entire backfill zone.

Compaction levels shown are for standard Proctor density.

8. See ADS Standard Detail STD-101D for additional details.

90% 85% 95% 90% 90% 95% 95% 3

TECHNICAL NOTE HDPE and HP Storm Connections to Manholes and Structures for Storm Sewer Applications

Introduction

A full line of pipe jointing options is available to fit the requirements of nearly any storm drain or gravity flow project specifications. The joints available range from soil tight split couplers to gasketed soil-tight (ST) and watertight (WT) pipe. When connecting pipe to drainage structures it is important to make those connections with a joint performance at least equal to that of the piping system.

Connection Options

Soil-Tight Performance

When using soil-tight pipe in non-watertight applications, it may be acceptable to grout the void space between the pipe and drainage structure.

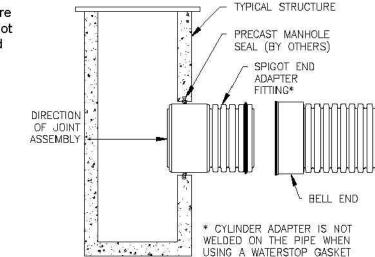
Watertight Performance

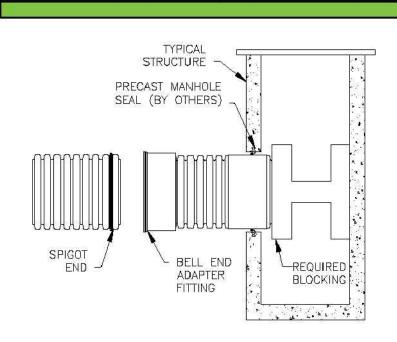
When using watertight pipe for testable systems, requiring some degree of watertight performance, it is necessary to provide additional measures to insure a watertight connection between the pipe and structure. ASTM F2510/F 2510M, "Standard Specification for Resilient Connectors Between Reinforced Concrete Manhole Structures and Corrugated High Density Polyethylene Drainage Pipes," is the governing standard for corrugated HDPE pipemanhole connections, but specific performance/installation requirements should be verified for each specific project. Along with a full line of adapter fittings available, including the Waterstop® Gasket, are flexible boot fittings provided by other manufactures. Fitting dimensions should be supplied to the manufacturer to insure the proper fitting size and manhole boot connector are supplied.

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Installation Recommendations

When installing a manhole adapter on the upstream end, the fitting may be over inserted into the structure temporarily while the adjoining pipe is laid. The spigot piece is then pushed back through the structure and connected to the bell end when pushing the joint together, as shown to the right.





Alternately, when using the adapter fitting in the downstream end of the structure, before pushing the bell and spigot together from inside the structure, it is necessary to provide blocking at the structure to prevent the fitting from moving in the structure, as shown to the left.

Summary

TN 5.04

The selection of which manhole connection is best suited for a project is based on the joint and connection requirements along with preferred manhole connection method for the region. It is imperative that prevailing regulations be consulted before selecting a manhole connection. Other options may be available for watertight manhole connections. Refer to 200 Series Standard Details for installation and connection-specific details. Contact your Regional Engineer or Application Engineering for further assistance.

HP STORM TRENCH INSTALLATION DETAIL MIN. COVER TO MIN. COVER TO RIGID PAVEMENT, H FLEXIBLE PAVEMENT, H BACKFILL BACKFILL 4" FOR 12"-24" PIPE 6" FOR 30"-60" PIPE MIN TRENCH WIDTH FOUNDATION (SEE TABLE)

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NOTES:

- ALL PIPE SYSTEMS SHALL BE INSTALLED IN ACCORDANCE WITH ASTM D2321, "STANDARD PRACTICE FOR UNDERGROUND INSTALLATION OF THERMOPLASTIC PIPE FOR SEWERS AND OTHER GRAVITY FLOW APPLICATIONS", LATEST ADDITION, WITH THE EXCEPTION THAT THE INITIAL BACKFILL MAY EXTEND TO THE CROWN OF THE PIPE. SOIL CLASSIFICATIONS ARE PER THE LATEST VERSION OF ASTM D2321. CLASS IVB MATERIALS (MH, CH) AS DEFINED IN PREVIOUS VERSIONS OF ASTM D2321 ARE NOT APPROPRIATE BACKFILL MATERIALS.
- MEASURES SHOULD BE TAKEN TO PREVENT MIGRATION OF NATIVE FINES INTO BACKFILL MATERIAL, WHEN REQUIRED.
- FOUNDATION: WHERE THE TRENCH BOTTOM IS UNSTABLE, THE CONTRACTOR SHALL EXCAVATE TO A DEPTH REQUIRED BY THE ENGINEER AND REPLACE WITH SUITABLE MATERIAL AS SPECIFIED BY THE ENGINEER. AS AN ALTERNATIVE AND AT THE DISCRETION OF THE DESIGN ENGINEER, THE TRENCH BOTTOM MAY BE STABILIZED USING A GEOTEXTILE MATERIAL.
- BEDDING: SUITABLE MATERIAL SHALL BE CLASS I, II, III, OR IV. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. COMPACTION SHALL BE SPECIFIED BY THE ENGINEER IN ACCORDANCE WITH TABLE 3 FOR THE APPLICABLE FILL HEIGHTS LISTED. UNLESS OTHERWISE NOTED BY THE ENGINEER, MINIMUM BEDDING THICKNESS SHALL BE 4" (100mm) FOR 12"-24" (300mm-600mm) DIAMETER PIPE; 6" (150mm) FOR 30"-60" (750mm-1500mm) DIAMETER PIPE. THE MIDDLE 1/3 BENEATH THE PIPE INVERT SHALL BE LOOSELY PLACED. PLEASE NOTE, CLASS IV MATERIAL HAS LIMITED APPLICATION AND CAN BE DIFFICULT TO PLACE AND COMPACT; USE ONLY WITH THE APPROVAL OF
- INITIAL BACKFILL: SUITABLE MATERIAL SHALL BE CLASS I, II, III, OR IV IN THE PIPE ZONE EXTENDING TO THE CROWN OF THE PIPE. THE CONTRACTOR SHALL PROVIDE DOCUMENTATION FOR MATERIAL SPECIFICATION TO ENGINEER. MATERIAL SHALL BE INSTALLED AS REQUIRED IN ASTM D2321, LATEST EDITION. COMPACTION SHALL BE SPECIFIED BY THE ENGINEER IN ACCORDANCE WITH TABLE 3 FOR THE APPLICABLE FILL HEIGHTS LISTED. PLEASE NOTE, CLASS IV MATERIAL HAS LIMITED APPLICATION AND CAN BE DIFFICULT TO PLACE AND COMPACT; USE ONLY WITH THE APPROVAL OF A SOIL EXPERT.
- MINIMUM COVER; MINIMUM COVER, H, IN NON-TRAFFIC APPLICATIONS (GRASS OR LANDSCAPE AREAS) IS 12" (300mm) FROM THE TOP OF PIPE TO GROUND SURFACE. ADDITIONAL COVER MAY BE REQUIRED TO PREVENT FLOTATION. FOR TRAFFIC APPLICATIONS: CLASS LOR II MATERIAL COMPACTED TO 90% SPD AND CLASS III COMPACTED TO 95% SPD IS REQUIRED. FOR TRAFFIC APPLICATIONS, MINIMUM COVER, H, IS 12" (300mm) UP TO 48" (1200mm) DIAMETER PIPE AND 24" (600mm) OF COVER FOR 60" (1500mm) DIÁMETER PIPE, MEASURED FROM TOP OF PIPE TO BOTTOM OF FLEXIBLE PAVEMENT OR TO TOP OF RIGID PAVEMENT.

FOR ADDITIONAL INFORMATION SEE TECHNICAL NOTE 2.04.

THE COMPONENTS AS REQUESTED. ADS HAS NOT PERFORMED ANY ENGINEERING OR DESIGN SERVICES FOR T JECT, NOR HAS ADS INDEPENDENTLY VERIFIED THE INFORMATION SUPPLIED. THE INSTALLATION DETAILS PROVIDED HEREIN ARE GENERAL OMMENDATIONS AND ARE NOT SPECIFIC FOR THIS PROJECT. THE DESIGN ENGINEER SHALL REVIEW THESE DETAILS PRIOR TO NSTRUCTION. IT IS THE DESIGN ENGINEERS RESPONSIBILITY TO ENSURE THE DETAILS PROVIDED HEREIN MEETS OR EXCEEDS THE PLICABLE NATIONAL, STATE, OR LOCAL REQUIREMENTS AND TO ENSURE THAT THE DETAILS PROVIDED HEREIN ARE ACCEPTABLE FOR THIS

PIPE DIAM. MIN. TRENCH TABLE 2, MINIMUM RECOMMENDED COVER BASED ON VEHICLE LOADING CONDITIONS SURFACE LIVE LOADING CONDITION HEAVY CONSTRUCTION PIPE DIAM. H-25 (75T AXLE LOAD) *

TABLE 1, RECOMMENDED MINIMUM TRENCH WIDTHS

TABLE 3, MAXIMUM COVER FOR ADS HP STORM PIPE, ft CLASSII CLASS III PIPE DIA COMPACTED 95% 90% 85% 95% 90% 95% (12.5m)(8.5m) (6.4m) (4.9m) (6.1m) (4.9m) (4.9m) (6.1m) (4.3m) (3.0m) (4.3m) (3.4m) (3.0m

FILL HEIGHT TABLE GENERATED USING AASHTO SECTION 12, LOAD RESISTANCE FACTOR DESIGN (LRFD) PROCEDURE WITH THE FOLLOWING NO HYDROSTATIC PRESSURE

Town of Vienna 06/09/2021

TRENCH INSTALLATION DETAIL (HP STORM)

CONTRACTOR TO USE POLYPROPYLENE PIPE (PP) MEETING VDOT SPEC. PC-1. USE ADS HP STORM OR EQUIVALENT.

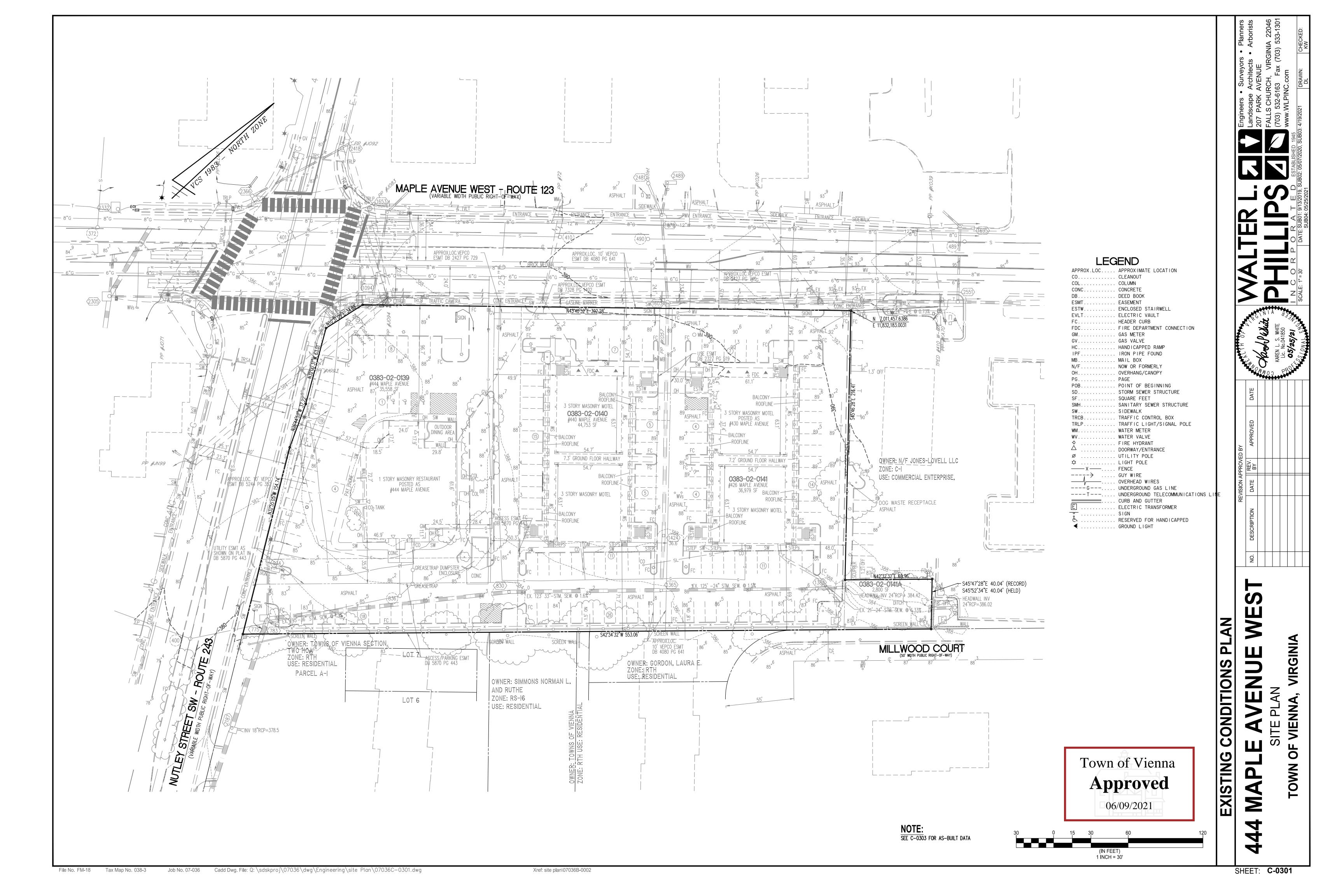
UNIT WEIGHT OF SOIL (Ys) = 120 PCF

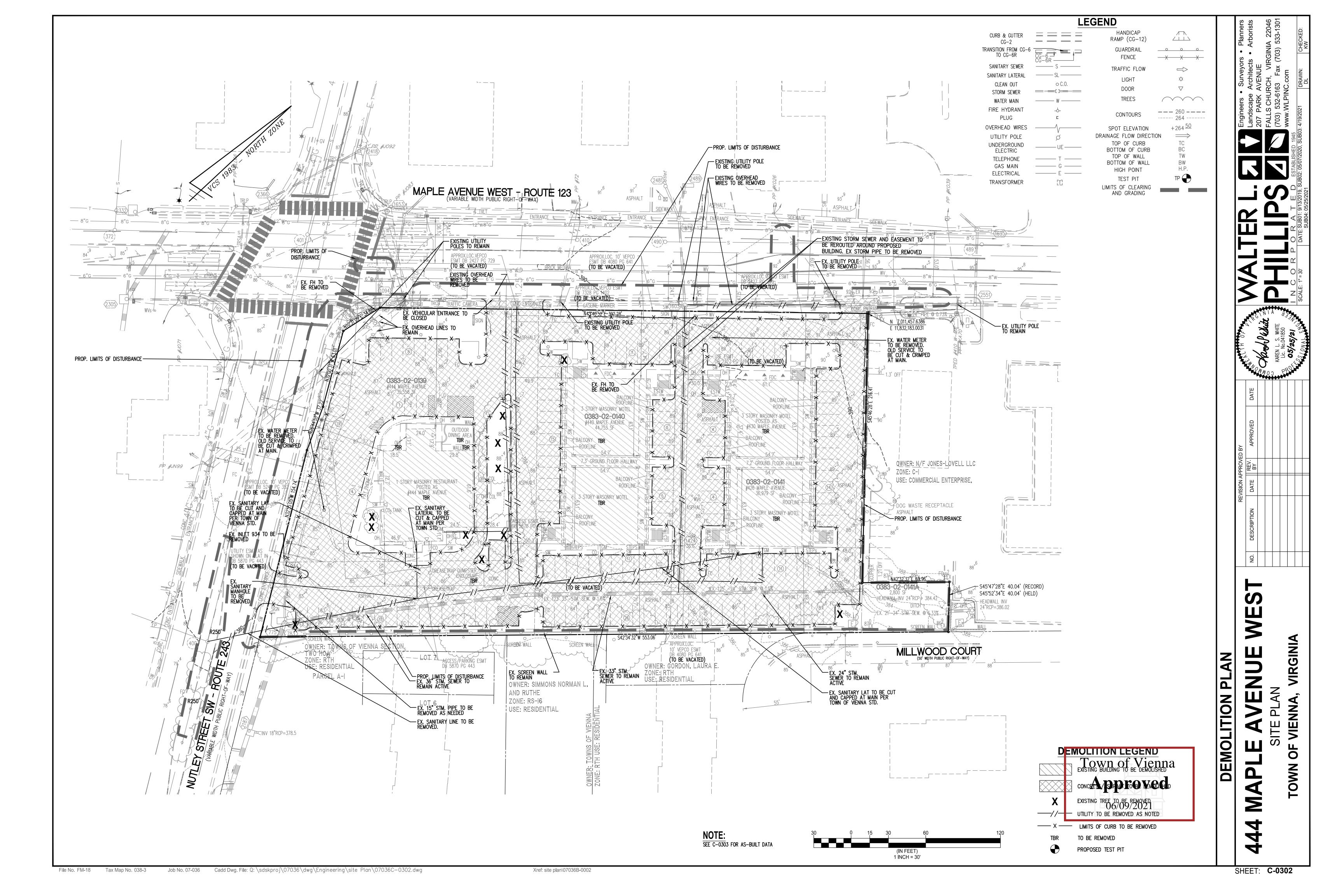
ALL DETAILS PROVIDED ON THIS SHEET ARE CURRENT AT TIME OF PLAN PREPARATION. CONTRACTOR IS RESPONSIBL FOR USING CURRENT DETAILS AT TIME OF CONSTRUCTION.

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NOTES

- I. THE PROPERTY IS DESIGNATED BY FAIRFAX COUNTY, VIRGINIA, AS HAVING TAX ASSESSMENT MAP NUMBERS 0383-02-0139, ZONED C-I, 0383-02-0140, 0383-02-0141, ZONED C-I AND RS-16, AND 0383-02-141A, ZONED RS-16
- 2. THE PROPERTY IS NOW AS FOLLOWS:
- A: PARCEL 0383-02-0139 IN THE NAME OF VIENNA DEVELOPMENT ASSOCIATES, LLC, JAMES C. MENG AND LUCY C. MENG, AS RECORDED IN DEED BOOK 19478 AT PAGE 1100.
- B: PARCELS 0383-02-0140, 0383-02-141 AND 0383-02-0141A IN THE NAME OF VIENNA DEVELOPMENT ASSOCIATES, LLC, JAMES C. MENG AND LUCY C. MENG, AS RECORDED IN DEED BOOK 19478 AT PAGE 1165.
 ALL OF THE FOREGOING AMONG THE LAND RECORDS OF FAIRFAX COUNTY, VIRGINIA.
- 3. THIS PLAT AND THE SURVEY ON WHICH IT IS BASED SHOWS ONLY THOSE IMPROVEMENTS WHICH ARE OBSERVABLE AND CAN BE MEASURED USING NORMAL SURVEYING METHODS. THE UNDERGROUND UTILITIES SHOWN HAVE BEEN LOCATED FROM FIELD SURVEY INFORMATION, MISS UTILITY MARKINGS, AND EXISTING RECORDS. THERE IS NO GUARANTEE, EITHER EXPRESS OR IMPLIED, THAT THE UNDERGROUND UTILITIES SHOWN COMPRISE ALL SUCH UTILITIES IN THE AREA, EITHER IN SERVICE OR ABANDONED, OR THAT THE UNDERGROUND UTILITIES SHOWN ARE IN THE EXACT LOCATION INDICATED.
- 4. THE FEDERAL EMERGENCY MANAGEMENT AGENCY FLOOD INSURANCE RATE MAP FOR FAIRFAX, VIRGINIA, MAP NUMBER 51059C0145E, EFFECTIVE DATE SEPTEMBER 17, 2010, DESIGNATES THE PROPERTY AS BEING IN ZONE X, AREAS DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN.
- 5. TOTAL COMPUTED AREA OF THE PROPERTY IS 120,091 SQUARE FEET OR 2.7569 ACRES. TOTAL RECORD AREA OF THE PROPERTY IS 120,470 SQUARE FEET OR 2.7656 ACRES.
- 6. THIS PLAT IS BASED ON A CURRENT FIELD SURVEY BY THIS FIRM.
- 7. EASEMENTS, COVENANTS, CONDITIONS AND RESTRICTIONS, SHOWN AND/OR NOTED HEREON, TAKEN FROM THE COMMITMENTS FOR TITLE INSURANCE PREPARED BY COMMONWEALTH LAND TITLE INSURANCE COMPANY, FILE NUMBER ARLO707009, DATED JUNE 12, 2007, AS TO PARCELS 0383-02-0140, 0383-02-0141 AND 0383-02-0141A, AND FILE NUMBER ARL0707010, DATED JUNE 14, 2007, AS TO PARCEL 0383-02-0139.
- 8. THE SITE SHOWN HEREON IS REFERENCED TO THE NORTH AMERICAN VERTICAL DATUM OF 1988 AND IS REFERENCED TO THE VIRGINIA COORDINATE SYSTEM OF 1983, [NAD 83(CORS96)(EPOCH:2002.0000)]. THE COMBINED FACTOR WHICH HAS BEEN APPLIED TO THE FIELD DISTANCES TO DERIVE THE REFERENCED COORDINATES IS 0.99994632. THE FOOT DEFINITION USED FOR CONVERSION OF THE MONUMENT COORDINATES AND IN THE PERFORMANCE OF THIS SURVEY IS THE U.S. SURVEY FOOT. CONTOUR INTERVAL IS TWO FEET.
- 9. THIS SURVEY WAS COMPLETED UNDER THE DIRECT AND RESPONSIBLE CHARGE OF, BRIAN G. BAILLARGEON, L.S. FROM AN ACTUAL X GROUND OR AIRBORNE SURVEY MADE UNDER MY SUPERVISION; THAT THE IMAGERY AND/OR ORIGINAL DATA WAS OBTAINED ON JUNE 8, 2007; AND THAT THIS PLAT, MAP, OR DIGITAL GEOSPATIAL DATA INCLUDING METADATA MEETS MINIMUM ACCURACY STANDARDS UNLESS OTHERWISE NOTED.

LEGAL DESCRIPTION

All of those lots or parcels of land, lying and being situate in the Town of Vienna, Virginia, being the property of Vienna Development Associates, LLC, James C. Meng and Lucy C. Meng, as recorded in Deed Book 19478 at page 1100, and in Deed Book 19478 at page 1165, all among the Land Records of Fairfax County, Virginia, being more particularly described by metes and bounds, as follows:

Beginning at a point in the westerly right-of-way line of Millwood Court, said point being the south corner of now-or-formerly Vienna Medical West Condominium Association; thence with the northerly right-of-way line of Millwood Court, the same course continued with the westerly line of Lot 10 and Parcel A1, Townes of Vienna, now-or-formerly Simmons, and Parcel A-1, Section Two, Townes of Vienna, S 42° 34' 32" W, 553.06 feet to a point in the northerly right-of-way line of Nutley Street, Route 243; thence with the northerly right-of-way line of Nutley Street, Route 243, N 32° 55' 05" W, 154.74 feet to a point; thence N 26° 44' 53" W, 37.22 feet to a point; thence N 32° 00' 31" W, 63.01 feet to a point; thence N 10° 31' 09" E, 34.88 feet to a point in the easterly right-of-way line of Maple Avenue, Route 123; thence with the easterly right-of-way line of Maple Avenue, Route 123, N 43° 40' 52" E, 392.28 feet to a point, said point being a corner of now-orformerly Jones-Lovell Enterprise, LLC; thence with the southerly line of nowor-formerly Jones-Lovell Enterprise, LLC, S 45° 46' 28" E, 216.41 feet to a point; thence N 42° 32' 32" E, 70.02 feet to a point in the southerly line of now-or-formerly Vienna Medical West Condominium Association; thence with the southerly line of now-or-formerly Vienna Medical West Condominium Association S 45° 47' 28" E, 40.04 feet to a point of beginning and containing an area of 120,091 square feet, or 2.7569 acres, more or less.

Being the same property as described in the title reports.

AS-BUILT DATA

SD 2418		SD 1388		SMH 410	
CURB INLET TOP =	387.48	DROP INLET, GRATE TOP =	387.57	MANHOLE TOP =	389.8
15"RCP OUT (SD 2366)=	384.90	24"RCP IN (HEADWALL)=	383.09	8"INV OUT (SMH 401)=	385.4
		24"RCP OUT (SD 1365)=	382.77		
SD 1653				SMH 401	
CURB INLET TOP =	387.91	SD 1365		MANHOLE TOP =	386.7
15"RCP IN (NORTH)=	385.41	DROP INLET, GRATE TOP =	385.62	8"INV IN (N.WEST)=	379.9
15"RCP OUT (SD 1094)=	385.36	30"RCP IN (SD 1424)=	380.96	8"INV IN (SMH 410)=	379.5
		24"RCP IN (SD 1388)=	380.87	8"INV OUT (SMH 372)=	379.3
SD 1094		33"RCP OUT (SD 830)=	380.12	8"INV OUT (SMH 400)=	379.2
CURB INLET TOP =	387.59				
15"RCP IN (SD 1653)=	383.74	SD 830		SMH 372	
15"RCP OUT (SD 2305)=	383.67	DROP INLET, GRATE TOP =	383.39	MANHOLE TOP =	384.3
		33"RCP IN (SD 1365)=	378.14	8"INV IN (N.WEST)=	379.2
SD 2305		36"RCP OUT (SD 775)=	377.49	8"INV IN (SMH 401)=	378.3
CURB INLET TOP =	384.27			8"INV OUT (S.WEST)=	378.3
15"RCP IN (SD 1094)=	377.45	SD 934			
42"x28"CMP IN (NORTH)=	377.44	CURB INLET TOP =	385.31	SMH 490	
18"RCP IN (WEST)=	377.57	15"RCP OUT (SD 775)=	378.36	MANHOLE TOP =	390.8
42"x28"CMP OUT (SOUTH)=	377.38			LAT INV IN (S.WEST)=	387.5
		SD 783		LAT INV IN (N.WEST)=	387.2
SD 1679		CURB INLET TOP =	382.06	8"INV OUT (SMH 489)=	387.0
CURB INLET TOP =	391.13	12"RCP OUT (SD 775)=	377.53		
18"RCP IN (WEST)=	386.41			SMH 489	
18"RCP IN (S.WEST)=	386.04	SD 775		MANHOLE TOP =	394.7
18"RCP OUT (SD 1603)=	385.93	STORM STRUCTURE TOP =	381.13	8"INV IN (SMH 490)=	386.0
		15"RCP IN (SD 934)=	377.03	8"INV OUT (N.EAST)=	385.8
SD 2551		12"RCP IN (SD 783)=	377.13		
MANHOLE TOP =	395.08	36"RCP IN (SD 830)=	375.44		
24"RCP IN (NORTH)=	386.33	42"RCP OUT (SOUTH)=	374.55		
18"RCP IN (SD 1695)=	387.46				
24"RCP OUT (SD 1603)=	385.93	SD 2187			
00. 4007		STORM STRUCTURE TOP =	381.98		
SD 1603	704.00	42"RCP IN (SD 775)=	373.86		
CURB INLET TOP = 18"RCP IN (SD 1679)=	391.08	18"RCP IN(END SECTION)= 42"RCP OUT (SOUTH)=	377.17		
18"RCP IN (SD 1679)= 24"RCP IN (SD 2551)=	384.29	42 RCP 001 (5001H)=	373.81		
$\frac{24 \text{ RCP IN}}{30^{\circ} \text{RCP OUT}} = \frac{(SD 2551)}{1424} = \frac{1}{2}$	384.23	CMIL 07C			
30 RCP 001 (30 1424)=	383.97	SMH 836	705.07		
CD 1404		MANHOLE TOP = DROP INV (GREASE TRAP)=	385.87		
SD 1424 DROP INLET, GRATE TOP =	707.50	6" INV DROP (BOTTOM)=	380.62		
$\frac{1}{30}$ RCP IN (SD 1603)=	387.52 382.71	6" INV IN (BUILDING)=	376.07		
$\frac{30 \text{ RCP IN}}{30 \text{ RCP OUT}} = \frac{30 \text{ RCP OUT}}{30 \text{ RCP OUT}} = \frac{30 \text{ RCP OUT}}{30 \text{ RCP OUT}} = \frac{30 \text{ RCP IN}}{30 \text{ RCP IN}} =$	382.71	6" INV IN (CLEANOUT)=	376.10 375.97		
	302.39	<u> </u>	+		
		6" INV OUT (SMH 732)=	375.94		

MANHOLE TOP =

8" INV IN

6" INV IN (SMH 401)= | 370.74

 $|8" \text{ INV IN} \qquad (SMH 836) = |370.72|$

 $|8" \text{ INV OUT} \qquad (\text{SOUTH}) = |370.05|$

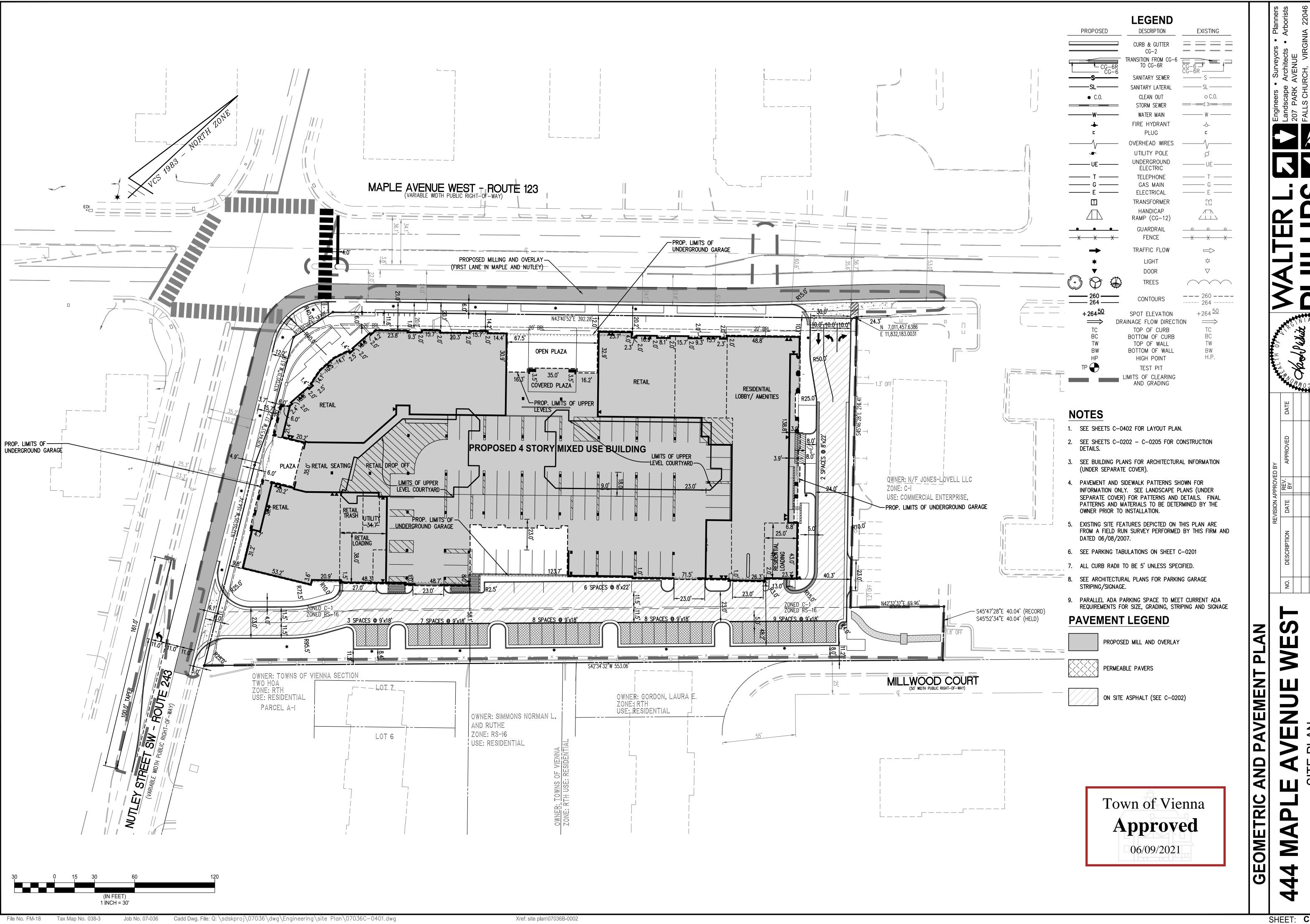
(WEST) = 370.38

380.60

Town of Vienna
Approved
06/09/2021

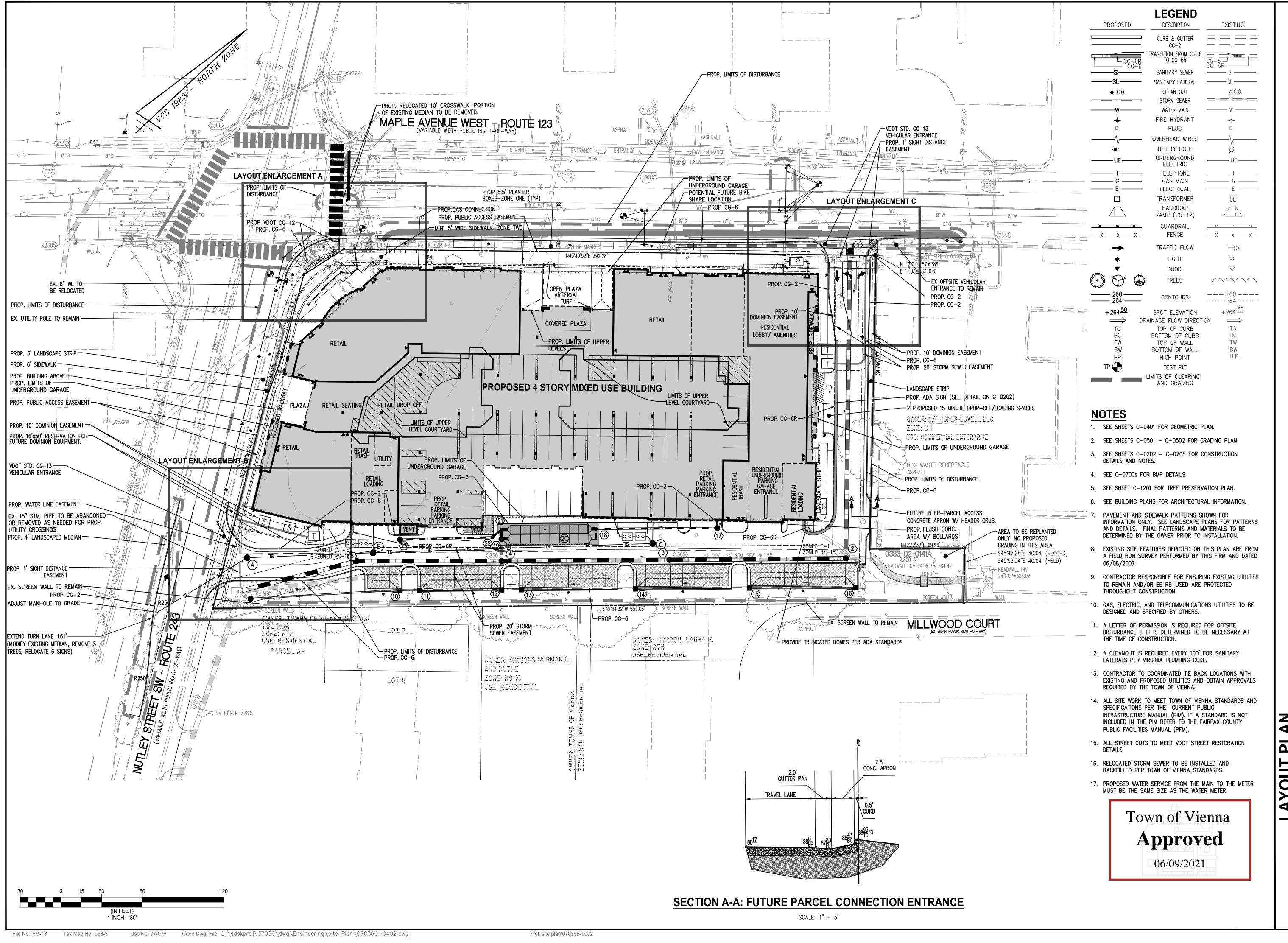
AS-BUILT D

43-BUILL DATA



SHEET: **C-0401**

VIRGINIA

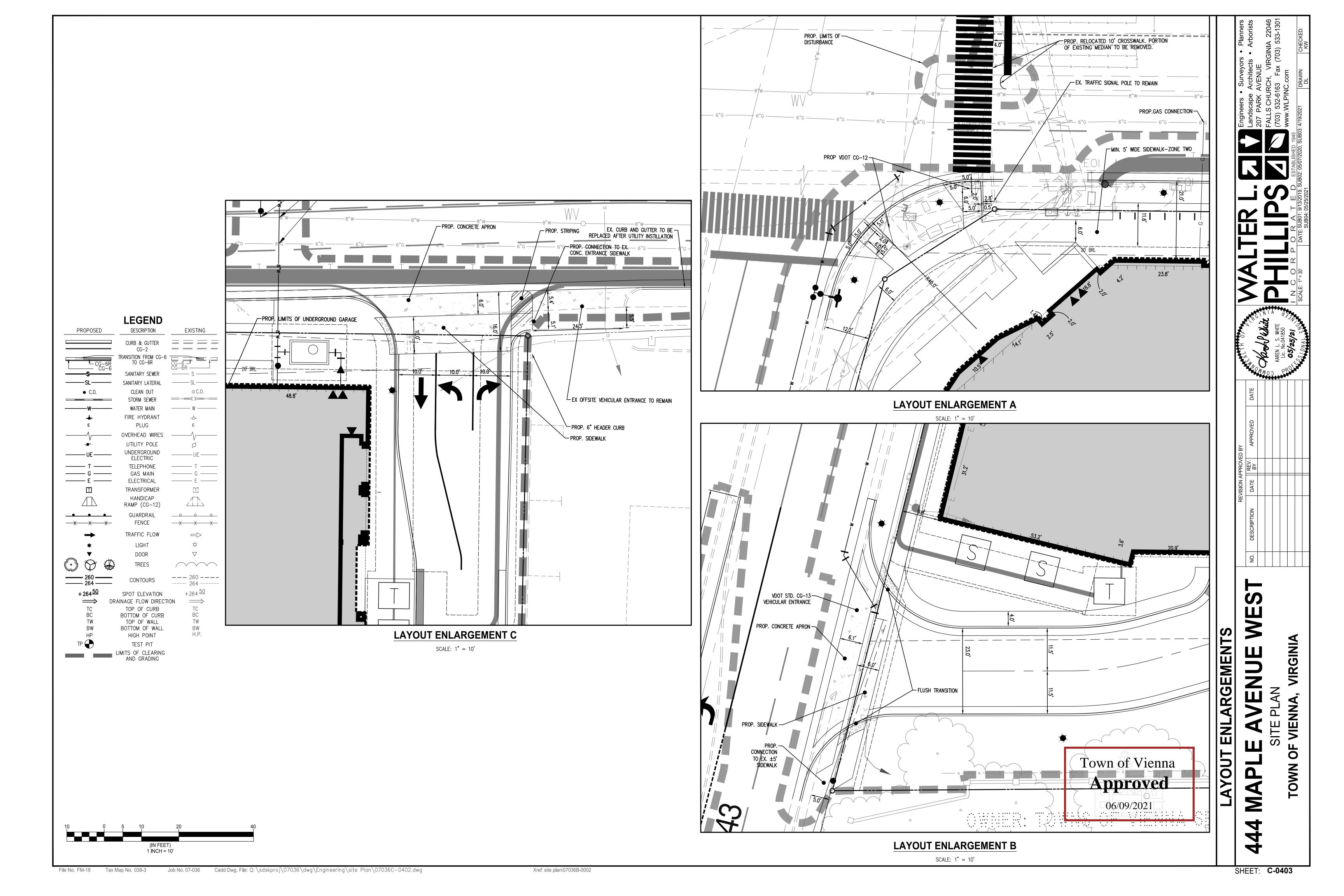


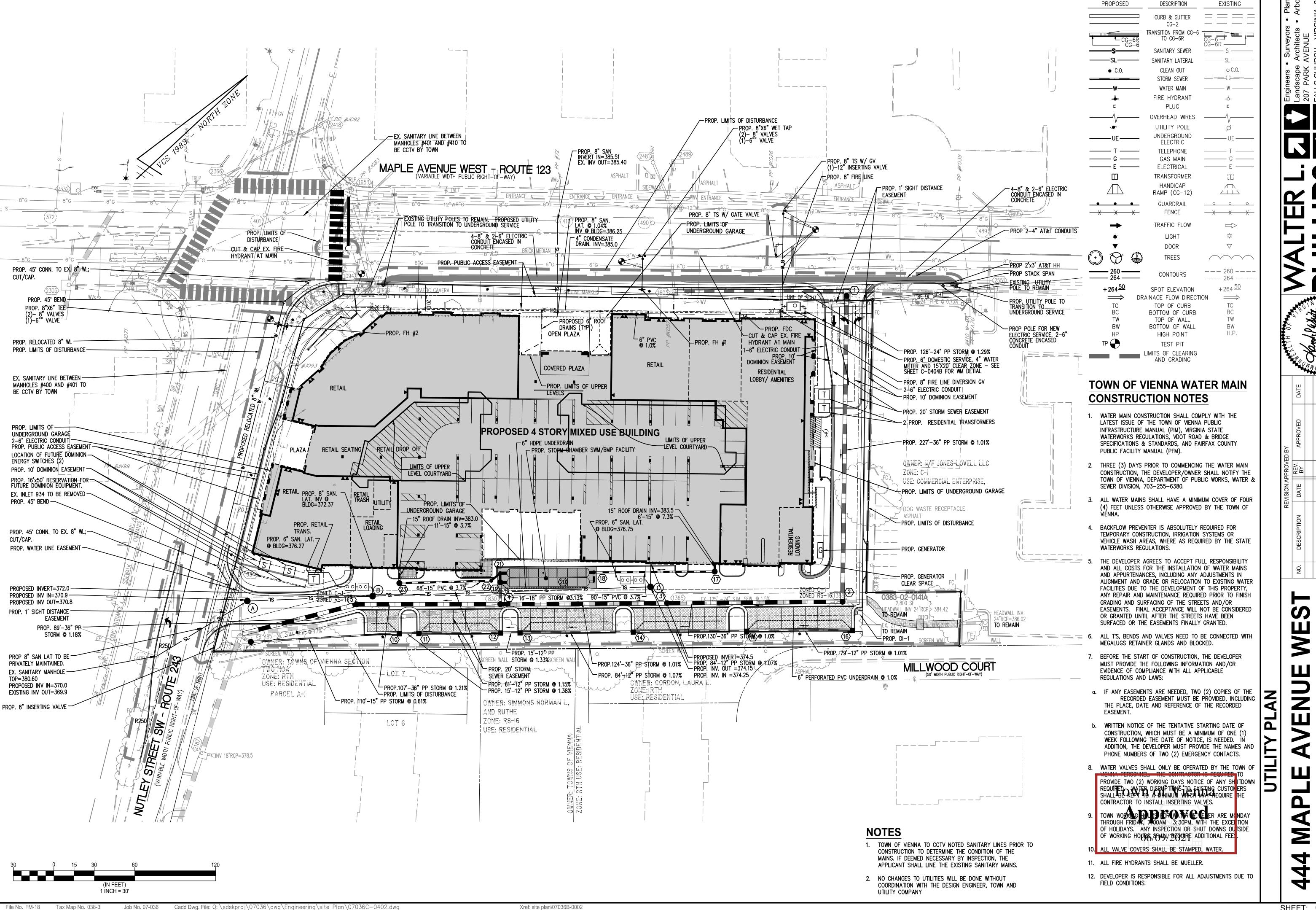
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VIRGINIA

SHEET: **C-0402**





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