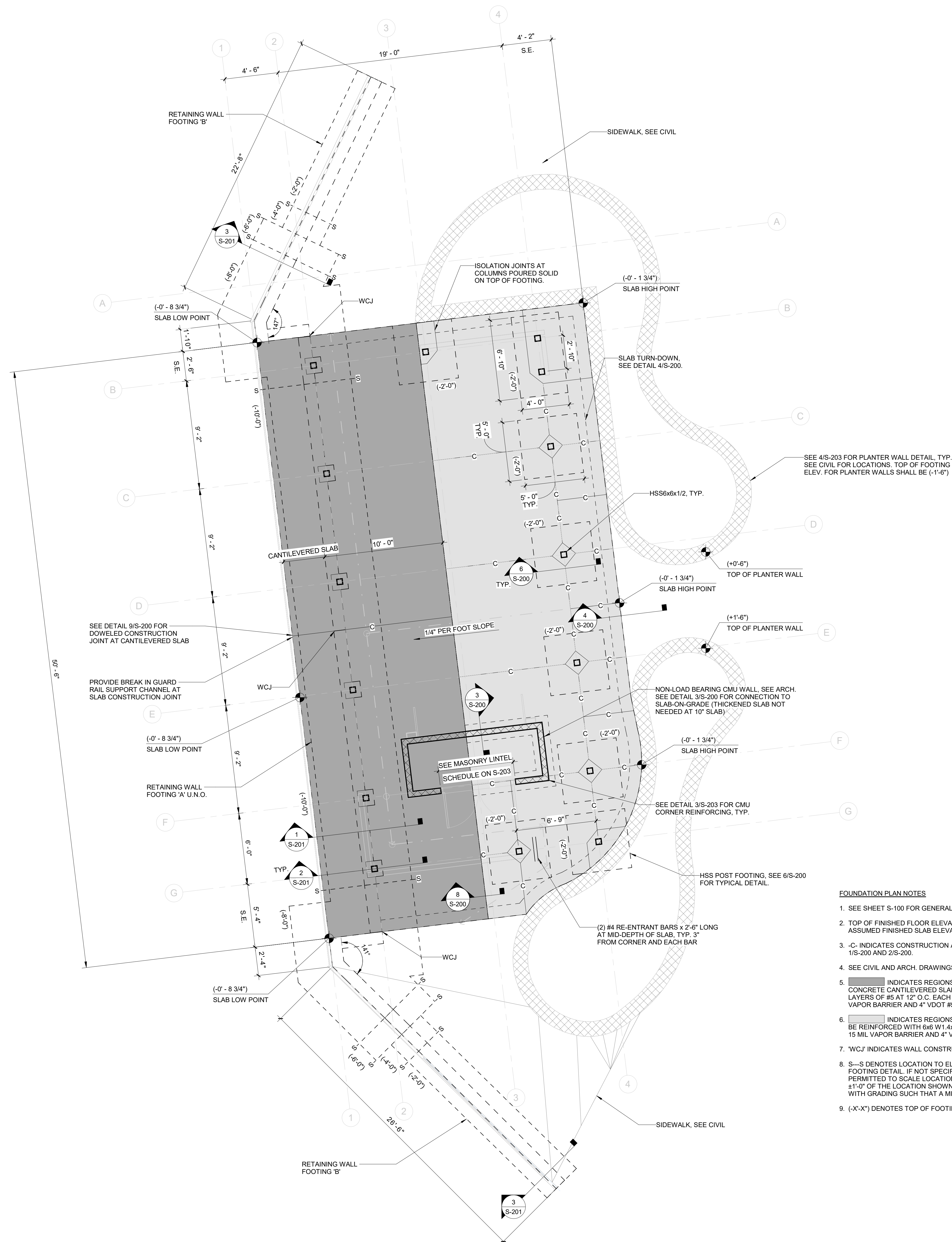


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NO.	DESCRIPTION	DATE
DRAWN BY		JLM
APPROVED BY		CCB
CHECKED BY		ACH
DATE		10/28/2025

Attachment 08



1 PAVILION FOUNDATION PLAN

Scale: 1/4" = 1'-0"

SEAL



KEY PLAN

SCALE

REVISIONS

NO.	DESCRIPTION	DATE

DRAWN BY JLM
APPROVED BY CCB
CHECKED BY ACH
DATE 10/28/2025

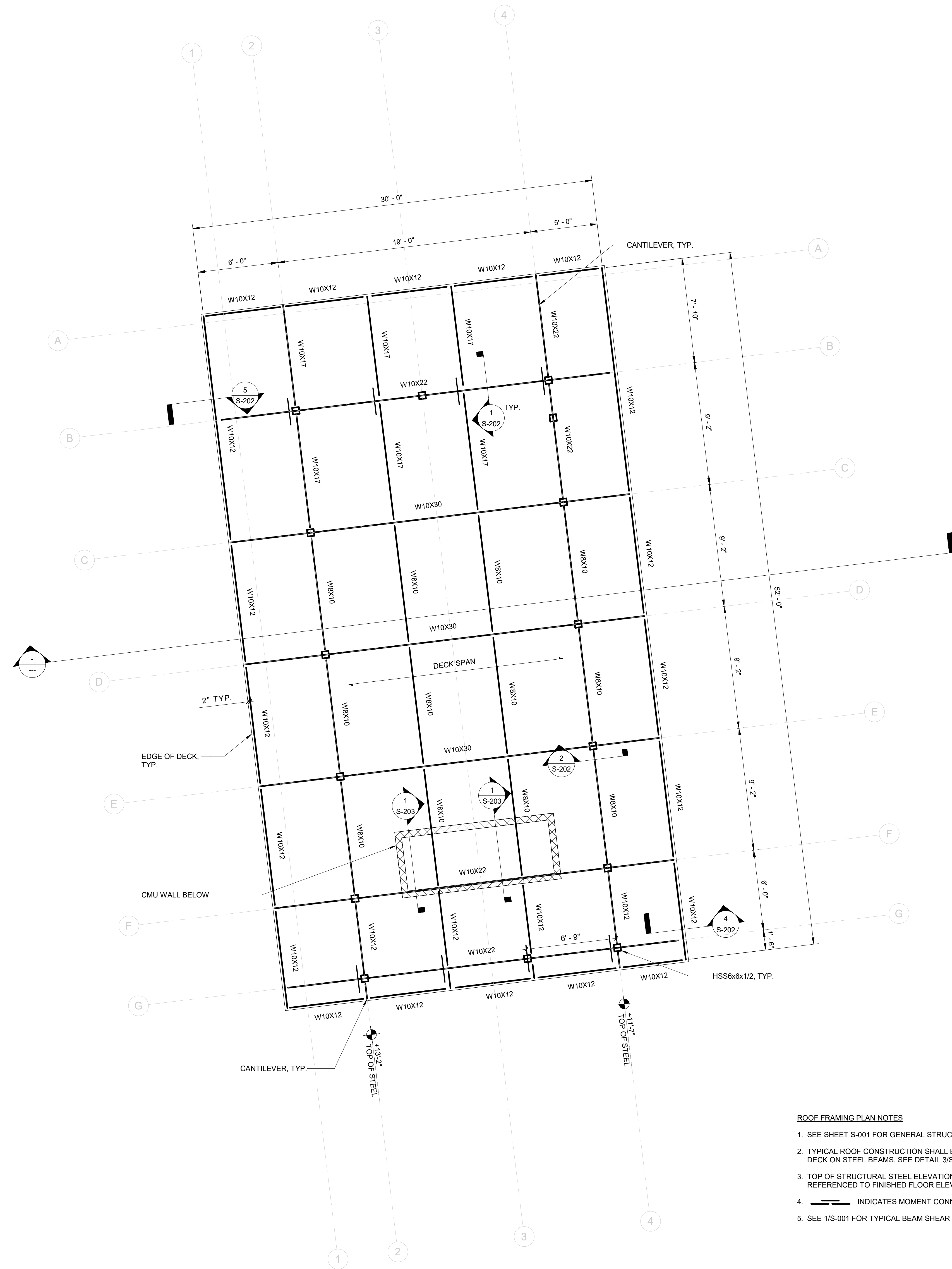
TITLE

ROOF FRAMING
PLAN

PROJECT NO. 50188638

S-101

SHEET NO.



1 PAVILLION ROOF FRAMING PLAN
Scale: 1/4" = 1'-0"

[illegible]

DRAWN BY	Author
APPROVED BY	Approver
CHECKED BY	Checker
DATE	OCTOBER 30 2025
TITLE	

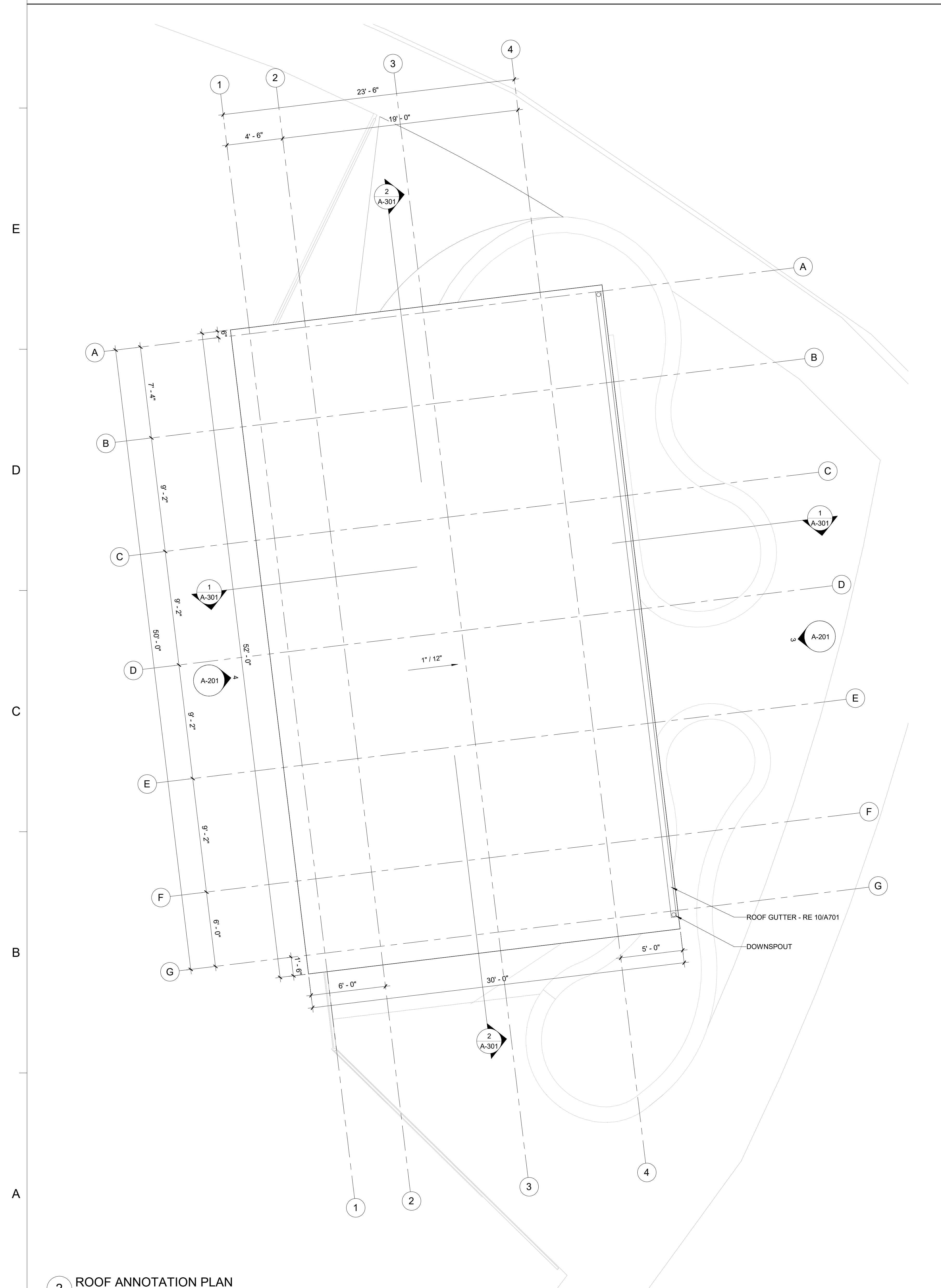
FIRST FLOOR &
ROOF
ANNOTATON
PLAN

PROJECT NO.	50188638
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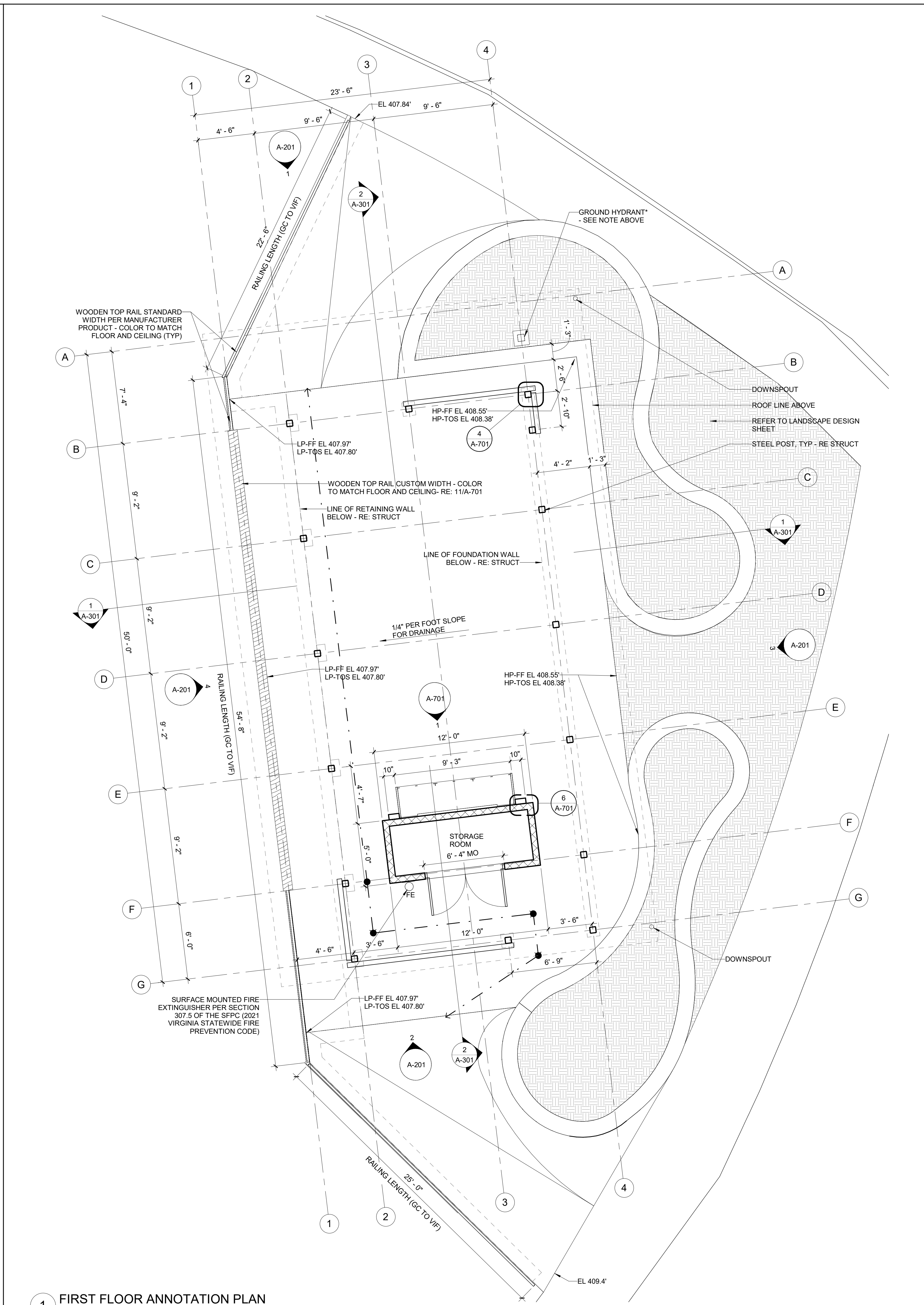
A-111

SHEET NO.

*GROSS HYDRANT NOTE: PROVIDE NEW GROUND HYDRANT OF THE NON-FREEZE TYPE. THE HYDRANT SEAT SHALL BE LOCATED 3'± BELOW FINISHED GRADE. THE HYDRANT SHALL BE LOOSE KEY OPERATED AND THE WASHER SHALL BE REPLACEABLE FROM GRADE. GROUND HYDRANT SHALL BE SURIN Z-1360, SMT#H 5801, WOODFORD V9S, OR APPROVED EQUIV FOR FLUSH WITH GRADE INSTALLATION. HYDRANT SHALL HAVE ALL BRONZE INTERIOR PARTS, REPLACEABLE BRONZE SEAT AND SEAT WASHER; BOX 3" THREADED HOSE CONNECTION, 1/4" DRAIN PORT, AND NON-TURNING OPERATING COUPLING WITH FREE-FLOATING BALL VALVE. CONCRETE PAD SHALL BE 12" MINIMUM THICKNESS. CONCRETE PAD EXTENDING A MINIMUM OF 4' BEYOND ALL SIDES OF THE HYDRANT BOX AND A MINIMUM OF 4" DEEP AND IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS INCLUDING SUITABLE GRAVEL BASE TO ALLOW FOR DRAINAGE FROM BOX. PROVIDE SUITABLE FITTING AT GROUND HYDRANT INLET TO ALLOW FOR CONNECTION TO NEW 3/4" TYPE K SOFT COPPER TUBING NON-POTABLE WATER SUPPLY PIPING. THE CONTRACTOR SHALL EXTEND NEW 3/4" TYPE K SOFT COPPER TUBING IN A TRENCH EXCAVATED FROM THE HYDRANT LOCATION TO THE OFFICE BUILDING FOLLOWING ROUGHLY THE SAME ROUTING AS INDICATED BY THE EXISTING NEW 3/4" TYPE K SOFT COPPER TUBING. THE TUBING SHALL BE INSTALLED AT LEAST 18" ABOVE FINISHED GRADE AND SHALL BE PROTECTED BY AN ABSOLUTE MINIMUM OF 12" OF COVER. THE TUBING SHALL BE APPROVED BY THE AHJ PLUMBING INSPECTOR. THE TUBING SHALL ENTER THE BUILDING WALL BELOW GRADE IN THE VICINITY WHERE THE ELECTRICAL SITE PLAN SHOWS THE NEW ELECTRICAL FEEDER PENETRATION OF THE BUILDING WALL. PROVIDE A THUNDERLINE UNKSEL OR SIMILAR PENETRATION SEALING DEVICE WHERE THE WATER TUBING ENTERS THE BUILDING VIA A NEW DRILLED HOLE. INSIDE THE BUILDING, THE TUBING SHALL BE NEW 3/4" TYPE K SOFT COPPER TUBING. PROVIDE A NEW 1/2" NPT FEMALE ADAPTER TO CONNECT THE TUBING TO THE EXISTING 1/2" NPT FEMALE ADAPTER FOR APPLICATION CONNECTION. PROVIDE AN ANSI 1013-COMPATIBLE REDUCED PRESSURE PRINCIPLE (RP2) BACK FLOW PREVENTION (BFP) DEVICE WITH ATMOSPHERIC VENT DRAIN LINE EXTENDED TO DISCHARGE ABOVE AN EXISTING DRY PIPE SPRINKLER VALVE OPEN SITE DRAIN. THE RP2 BFO SHALL BE INSTALLED AT 80" MAXIMUM ABOVE THE FLOOR PER CODE AND BE COORDINATED WITH OTHER WORK. NEW COLD WATER PIPING INSIDE THE BUILDING SHALL BE INSULATED WITH RIGID FOAM INSULATION WITH A MINIMUM OF 1" THICKNESS. PROVIDE A 100-DEGREE F MEAN TEMPERATURE AND WEIGHT LOSS TEST REPORT FOR THE INSULATION. THE JACKET SHALL BE IDENTIFIED BY LABELS ON EACH SIDE OF THE BUILDING STRUCTURE USING HANGERS ATTACHED TO EXISTING PIPING. PROVIDE 3" GALVANIZED STEEL TAGS TO PROVIDE A 1/2" CAPPED HOSE END DRAIN VALVE AT THE LOW POINT IN THE NEW COLD WATER PIPING INSIDE THE BUILDING TO ALLOW FOR DRAINING WATER FROM THE UNDERGROUND PIPING IF NEEDED.



2 ROOF ANNOTATION PLAN
Scale: 1/4" = 1'-0"



1 FIRST FLOOR ANNOTATION PLAN
Scale: 1/4" = 1'-0"

1

2

3

4

5

6

F

E

D

C

B

A



Dewberry Architects Inc.

8401 Arlington Boulevard
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703.849.0100

NAVY FEDERAL CREDIT UNION
NFCU POND PAVILION

820 Follin Ln SE,
Vienna, VA 22180

Permit Submission



SEAL

KEY PLAN

SCALE

REVISIONS

NO.	DESCRIPTION	DATE

DRAWN BY _____ Author
APPROVED BY _____ Approver
CHECKED BY _____ Checker
DATE _____ OCTOBER 30 2025

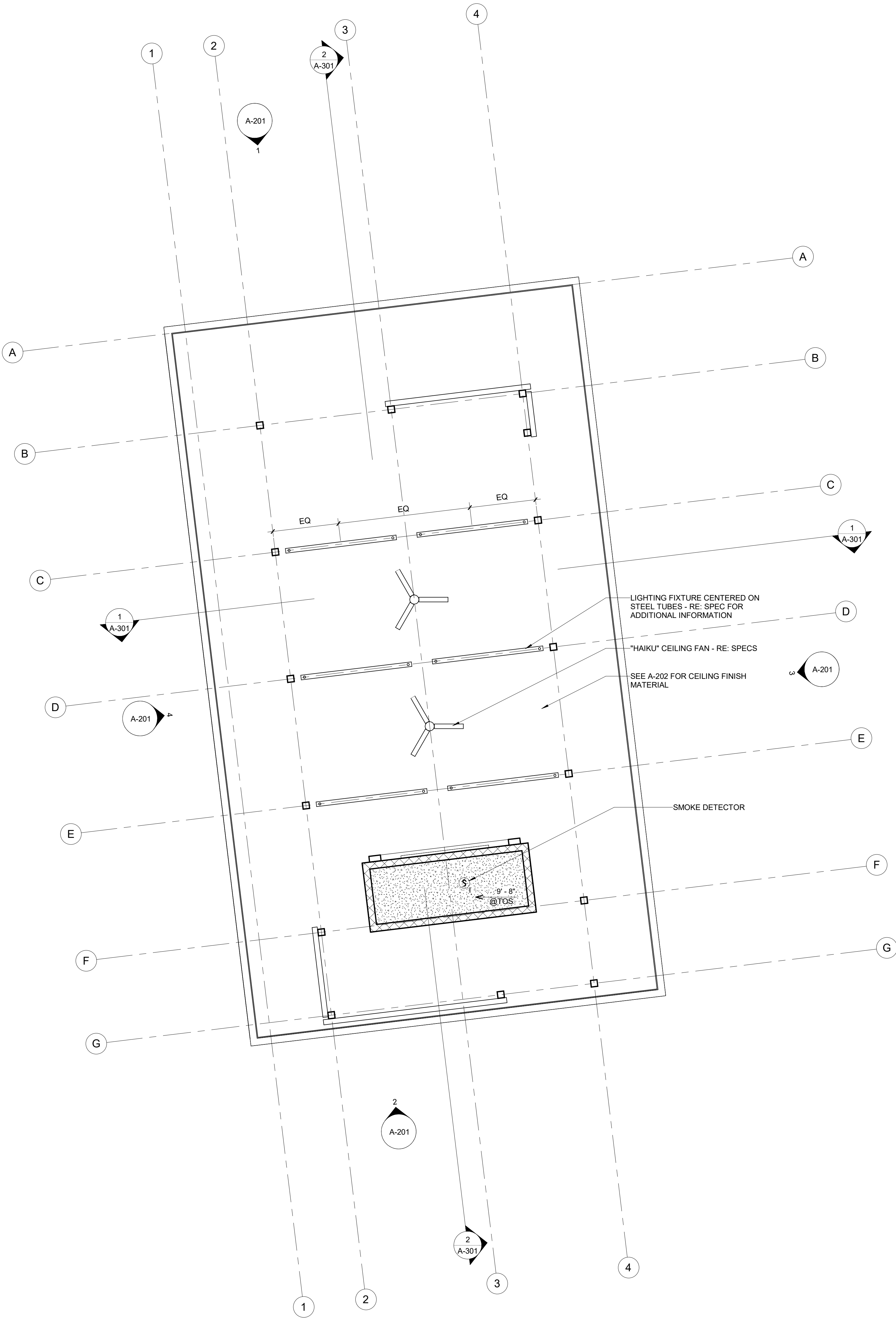
TITLE

FIRST FLOOR
REFLECTED
CEILING PLAN

PROJECT NO. 50188638

A-113

SHEET NO.



1 FIRST FLOOR REFLECTED CEILING PLAN
Scale: 1/4" = 1'-0"

1

2

3

4

5

6



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GENERAL STRUCTURAL NOTES

REBAR SPLICE AND HOOK SCHEDULE									
REBAR SPLICE AND HOOK DIMENSIONS IN REINFORCING CONCRETE F _c =3,000 PSI					REBAR SPLICE AND HOOK DIMENSIONS IN REINFORCING CONCRETE F _c =4,500 PSI				
ASTM BAR SIZE	LD	CLASS B LAP	TOP BAR LAP	LDH	ASTM BAR SIZE	LD	CLASS B LAP	TOP BAR LAP	LDH
#3	12"	13"	17"	9"	#3	12"	12"	14"	7"
#4	14"	18"	23"	11"	#4	12"	14"	19"	9"
#5	17"	22"	28"	14"	#5	14"	18"	23"	12"
#6	20"	26"	34"	17"	#6	17"	21"	28"	14"
#7	32"	41"	54"	20"	#7	26"	34"	44"	16"
#8	42"	54"	70"	22"	#8	34"	44"	57"	18"
#9	53"	68"	89"	25"	#9	43"	56"	73"	21"

LD - STANDARD DEVELOPMENT LENGTH OF BAR

LDH - STANDARD DEVELOPMENT LENGTH OF HOOK

F_c - SPECIFIED COMPRESSIVE STRENGTH OF CONCRETE

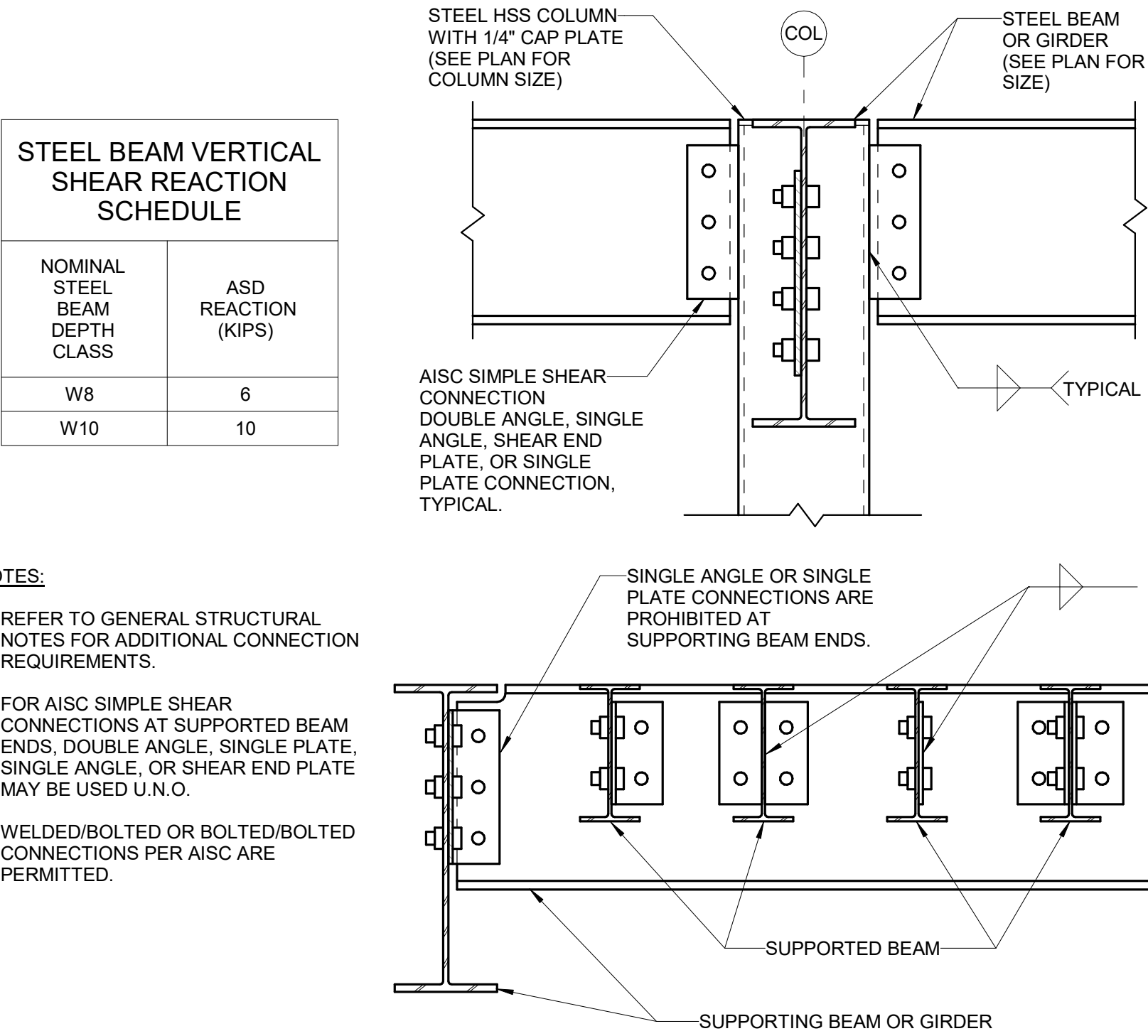
db - BAR DIAMETER PER ASTM

NOTES:

TOP BAR - DEFINED AS A BAR LOCATED SUCH THAT 12 IN. OR MORE OF FRESH CONCRETE IS CAST IN THE MEMBER BELOW THE SPLICE.

MINIMUM OUTSIDE RADIUS OF BEND, R, SHALL BE 4db.

STRUCTURAL ABBREVIATION LIST			
A.F.F.	ABOVE FINISHED FLOOR	KSI	KIPS PER SQUARE INCH
ARCH.	ARCHITECTURAL	LLH	LONG LEG HORIZONTAL
B.S.	BOTH SIDES	LLV	LONG LEG VERTICAL
BOT	BOTTOM	MAX	MAXIMUM
C=	CAMBER=	MECH.	MECHANICAL
CF	CUBIC FOOT	MIN.	MINIMUM
CLR	CLEAR	MPH	MILES PER HOUR
COL	COLUMN	N.T.S.	NOT TO SCALE
CONC.	CONCRETE	# / NO.	NUMBER
CONT.	CONTINUOUS	O.C.	ON CENTER
DET.	DETAIL	PCF	POUNDS PER CUBIC FOOT
DIA.	DIAMETER	PSF	POUNDS PER SQUARE INCH
DIM.	DIMENSION	PSI	POUNDS PER SQUARE FOOT
EA	EACH	R	RADIUS
ELEV.	ELEVATION	REIN.	REINFORCING
EQ.	EQUAL	REQ'D	REQUIRED
EQUIP.	EQUIPMENT	SIM	SIMILAR
EXIST.	EXISTING	S.E.	SLAB EDGE
EXP.	EXPANSION	S.S.	STAINLESS STEEL
EXT.	EXTERIOR	STD.	STANDARD
F.F.E.	FINISHED FLOOR ELEV.	STRU.	STRUCTURAL
FIN.	FINISH	TS	TUBE STEEL
FTG.	FOOTING	T.O.S.	TOP OF STEEL
GALV.	GALVANIZED	TYP.	TYPICAL
GA	GAGE	U.N.O.	UNLESS NOTED OTHERWISE
HSS	HOLLOW STRUCTURAL SECTION	V.I.F.	VERIFY IN FIELD
HORIZ.	HORIZONTAL	VERT.	VERTICAL
INT.	INTERIOR	W.W.R.	WELDED WIRE REINFORCING
		W.P.	WORK POINT



- NOTES:
- REFER TO GENERAL STRUCTURAL NOTES FOR ADDITIONAL CONNECTION REQUIREMENTS.
 - FOR AISC SIMPLE SHEAR CONNECTIONS AT SUPPORTED BEAM ENDS, DOUBLE ANGLE, SINGLE PLATE, SINGLE ANGLE, OR SHEAR END PLATE MAY BE USED U.N.O.
 - WELDED/BOLTED OR BOLTED/BOLTED CONNECTIONS PER AISC ARE PERMITTED.

1 BEAM SHEAR CONNECTION DETAIL

Scale: 1" = 1'-0"

DESIGN:

- THE GENERAL STRUCTURAL NOTES ARE INTENDED TO AUGMENT THE DRAWINGS AND SPECIFICATIONS. SHOULD CONFLICTS EXIST BETWEEN THE DRAWINGS, SPECIFICATIONS, AND GENERAL STRUCTURAL NOTES, THE STRICTEST PROVISION SHALL GOVERN.
- STRUCTURAL DESIGN CONFORMS TO THE REQUIREMENTS OF THE FOLLOWING CODES:
2021 VIRGINIA BUILDING CODE (2021 INTERNATIONAL BUILDING CODE WITH AMENDMENTS)
BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE (ACI 318-19)
SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (AISC 360-16)
MINIMUM DESIGN LOADS FOR BUILDINGS AND OTHER STRUCTURES (ASCE 7-22)
- DESIGN LOADS AND DESIGN DATA ARE AS FOLLOWS:

A. FLOOR LIVE LOADS:	
SLAB-ON-GRADE / CANTILEVERED SLAB	150 PSF
B. ROOF LIVE LOAD:	20 PSF
C. ROOF SNOW LOAD:	
GROUND SNOW LOAD (P _g)	68 PSF
SNOW EXPOSURE FACTOR (C _e)	0.9
THERMAL FACTOR	1.1
FLAT ROOF SNOW LOAD (P _f)	47.2 PSF
D. WIND LOAD:	
RISK CATEGORY	II
ULTIMATE WIND SPEED (3 SECOND GUST)	112 MPH
WIND EXPOSURE CATEGORY	C
INTERNAL PRESSURE COEFFICIENT	±0.0
E. SEISMIC DESIGN DATA:	
RISK CATEGORY	II
SEISMIC IMPORTANCE FACTOR (I _e)	1.0
SPECTRAL RESPONSE ACCELERATION, S _s	0.15
SPECTRAL RESPONSE ACCELERATION, S ₁	0.042
SPECTRAL RESPONSE COEFF. S _{0.5}	0.13
SPECTRAL RESPONSE COEFF. S _{D1}	0.059
SITE CLASS	D (ASSUMED)
SEISMIC DESIGN CATEGORY	A
- UNLESS SPECIFICALLY NOTED, THERE ARE NO PROVISIONS MADE FOR FUTURE FLOORS, ROOFS, OR OTHER LOADS.

COORDINATION:

- STRUCTURAL DRAWINGS SHALL BE USED IN CONJUNCTION WITH, AND COORDINATED WITH CIVIL, MECHANICAL, ELECTRICAL, ARCHITECTURAL, AND OTHER CONTRACT DOCUMENTS. INCLUDING SPECIFICATIONS. A CONFLICT BETWEEN THE DRAWINGS AND SPECIFICATIONS OCCUR, THE MORE STRINGENT REQUIREMENT SHALL APPLY.
- THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. AND, EXCEPT WHERE SPECIFICALLY SHOWN, DO NOT INDICATE THE METHOD OR MEANS OF CONSTRUCTION. THE CONTRACTOR SHALL SUPERVISE AND DIRECT THE WORK AND SHALL BE SOLELY RESPONSIBLE FOR ALL CONSTRUCTION MEANS, METHODS, PROCEDURES, TECHNIQUES, AND SEQUENCE. CONTRACTOR SHALL FOLLOW ALL APPLICABLE SAFETY REGULATIONS.
- THE CONTRACTOR SHALL CHECK AND VERIFY ALL DIMENSIONS AND GRADE CONDITIONS (BOTH NEW AND EXISTING), REPORTING ANY DISCREPANCIES TO THE ARCHITECT PRIOR TO ORDERING MATERIALS OR PROCEEDING WITH ANY PHASE OF THE WORK.
- DO NOT SCALE DIMENSIONS FROM DRAWINGS. THE CONTRACTOR SHALL REQUEST, FROM THE ARCHITECT, NECESSARY DIMENSIONS NOT SHOWN ON THE DRAWINGS.
- COORDINATE THE EXACT SIZE AND LOCATION OF ALL SLEEVES AND OPENINGS THROUGH CONCRETE SLABS AND CONCRETE WALLS WITH CIVIL, MECHANICAL, PLUMBING, AND ELECTRICAL DRAWINGS. SLEEVES SHALL BE ASTM A 53 SCHEDULE 40 STEEL WITH A DIAMETER NOT GREATER THAN 12 INCHES AND SHALL BE GALVANIZED AFTER CUTTING.
- ALL PRODUCTS AND MATERIALS REQUIRED FOR THE WORK SHALL BE INSTALLED IN ACCORDANCE WITH MANUFACTURER'S WRITTEN INSTRUCTIONS AND RECOMMENDATIONS FOR INSTALLATION IN APPLICATIONS INDICATED, INCLUDING ALL ACCESSORIES, ATTACHMENTS, AND THE LIKE.

FOUNDATIONS:

- THE STRUCTURAL ENGINEER OF RECORD IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD CONTRARY TO THOSE ASSUMED FOR DESIGN.
- ASSUMED FOUNDATION BEARING PRESSURE USED IN DESIGN (POUNDS PER SQUARE FOOT):

SLAB-ON-GRADE, FOOTINGS	2,000 PSF
-------------------------	-----------

BEARING PRESSURE SHALL BE VERIFIED IN THE FIELD BY GEOTECHNICAL ENGINEER PRIOR TO CONSTRUCTION.
- ALL COMPACTED FILL, EXCAVATIONS, AND SUBGRADES SHALL BE OBSERVED AND TESTED BY A GEOTECHNICAL ENGINEER REGISTERED IN THE COMMONWEALTH OF VIRGINIA (OR A QUALIFIED GEOTECHNICAL TECHNICIAN WORKING UNDER THE DIRECT SUPERVISION OF A REGISTERED ENGINEER) TO VERIFY SPECIFIED GEOTECHNICAL CONFORMANCE REQUIREMENTS. CONTRACTOR SHALL COORDINATE TESTING WITH OWNER AS DECLARED IN THE CONTRACT DOCUMENTS.
- AFTER STRIPPING TOPSOIL, AREAS INTENDED TO SUPPORT NEW FILL, FOUNDATIONS, AND FLOOR SLABS SHALL BE EVALUATED BY THE PROJECT GEOTECHNICAL ENGINEER. PROJECT GEOTECHNICAL ENGINEER MAY REQUIRE PROOFROLLING OF THE EXPOSED SUBGRADE WITH AN APPROVED PIECE OF EQUIPMENT, SUCH AS LOADED DUMP TRUCK, HAVING A SINGLE-AXLE WEIGHT OF AT LEAST 10 TONS. PROOFROLLING SHALL BE PERFORMED UNDER THE DIRECT INSPECTION OF THE GEOTECHNICAL ENGINEER DURING A TIME OF GOOD WEATHER AND NOT WHILE THE PROJECT SITE IS WET, FROZEN, OR SEVERELY DESICCATED. SUBGRADES WHICH EXHIBIT EXCESSIVE PUMPING OR RUTTING SHALL BE UNDERCUT AND REPLACED WITH COMPACTED ENGINEERED FILL AS DIRECTED BY THE PROJECT GEOTECHNICAL ENGINEER.
- COMPACTED STRUCTURAL FILL SHALL BE AS FOLLOWS:

A. INSPECTED AND APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER.
B. NON-ORGANIC ON-SITE OR OFF-SITE SOILS APPROVED BY THE PROJECT GEOTECHNICAL ENGINEER.
C. PLACED IN LOOSE LIFTS NOT EXCEEDING 8 INCHES IN THICKNESS.
D. STANDARD PROCTOR (ASTM D 698) MAXIMUM DRY DENSITY EQUAL TO OR EXCEEDING 105 POUNDS PER CUBIC FOOT.
E. COMPACTED TO AT LEAST 95 PERCENT OF STANDARD PROCTOR (ASTM D 698) MAXIMUM DRY DENSITY.
F. MOISTURE CONTENT WITHIN 3 PERCENTAGE POINTS OF THE OPTIMUM MOISTURE CONTENT. PLASTICITY INDEX AS RECOMMENDED BY GEOTECHNICAL ENGINEER.
- FREE OF BOULDERS, ORGANICS, TRASH, PARTICLES OF 3 INCHES OR MORE IN DIAMETER, AND OTHER DELETERIOUS MATERIALS.
- FOR EXTERIOR BUILDING WALL FOOTINGS, PLACE BACKFILL EVENLY ON BOTH SIDES OF WALL SUCH THAT UNBALANCED BACKFILL DOES NOT EXCEED 12 INCHES.
- DURING FILLING AND BACKFILLING, DENSITY TESTING SHALL BE MADE IN ACCORDANCE WITH ASTM D-6938 (OR EQUIVALENT) TO MONITOR COMPACTION LEVELS AND MOISTURE CONTENTS. FREQUENCY OF DENSITY TESTING SHALL BE A MINIMUM OF ONCE PER 2,500 SQUARE FEET OF AREA PER LIFT AND AS DIRECTED BY GEOTECHNICAL ENGINEER TO VERIFY SPECIFIED COMPACTION AND MOISTURE CONTENT REQUIREMENTS.
- CARE SHALL BE EXERCISED DURING EXCAVATION FOR FOUNDATIONS SO THAT AS LITTLE DISTURBANCE AS POSSIBLE OCCURS AT THE FOUNDATION LEVEL. LOOSE OR SOFT SOILS SHALL BE CAREFULLY CLEANED FROM THE BOTTOM OF THE EXCAVATIONS BEFORE PLACING CONCRETE. ACTUAL FOUNDATION SUBGRADES SHALL BE OBSERVED DURING CONSTRUCTION BY THE GEOTECHNICAL ENGINEER TO EVALUATE THE SUITABILITY OF SUBGRADE SOILS.
- FOUNDATION SUBGRADES REQUIRING UNDERCUT SHALL BE FILLED FROM THE ELEVATION OF UNDERCUT TO THE ORIGINAL DESIGN SUBGRADE ELEVATION WITH LEAN CONCRETE, MINIMUM 2,000 PSI FLOWABLE FILL UNLESS BACKFILL MATERIAL IS APPROVED BY THE GEOTECHNICAL ENGINEER.
- WHENEVER POSSIBLE, FOUNDATION CONCRETE SHALL BE PLACED IMMEDIATELY AFTER EXCAVATION SO THAT ACCUMULATION OF WATER IN THE EXCAVATION OR DRYING OF FOUNDATION SOILS CAN BE AVOIDED.
- CONTRACTOR SHALL CONTROL SITE GROUNDWATER AND/OR SURFACE WATER BY ALL MEANS NECESSARY TO MAINTAIN A WATER LEVEL ONE FOOT BELOW SLAB SUBGRADE SO AS TO NOT DAMAGE FOUNDATION EXCAVATIONS.
- ANY SUBGRADE SOILS WHICH HAVE BEEN WEAKENED DUE TO SATURATION OR DISTURBANCE SHALL BE RECOMPACTED OR REMOVED AND REPLACED WITH STRUCTURAL FILL AS DIRECTED BY THE PROJECT GEOTECHNICAL ENGINEER. CONCRETE STRUCTURES SHALL BE CONSTRUCTED IN AN EXPEDIENT MANNER ONCE EXCAVATIONS ARE MADE TO AVOID WEATHER DAMAGE.
- DO NOT UNDERCUT SOILS BELOW EXISTING FOUNDATIONS.
- UTILITY LINES SHALL NOT BE PLACED THROUGH OR BELOW FOUNDATIONS WITHOUT APPROVAL OF THE STRUCTURAL ENGINEER. CONTRACTOR SHALL SUBMIT DETAILED DRAWINGS OF ALL SUCH CONDITIONS PRIOR TO CONSTRUCTION.

REINFORCED CONCRETE:

- UNLESS NOTED OTHERWISE, ALL CONCRETE WORK, DETAILING, FABRICATION, AND PLACING OF REINFORCING AND CONCRETE SHALL BE GOVERNED BY THE LATEST REVISIONS OF:

A. ACI 301, ACI 315, AND ACI 318.
B. CRSI RECOMMENDED PRACTICE OF PLACING REINFORCING BARS.
C. ACI 308 AND ACI 308 FOR COLD AND HOT WEATHER CONCRETING, RESPECTIVELY.
D. ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE.
- ALL CONCRETE SHALL BE NORMAL WEIGHT WITH A MAXIMUM UNIT WEIGHT OF 150 POUNDS PER CUBIC FOOT AND SHALL HAVE A MINIMUM 28 DAY COMPRESSIVE STRENGTH, AS SPECIFIED BELOW, FOR THE RESPECTIVE AREAS:

SLAB-ON-GRADE, CANTILEVERED SLAB, WALLS, AND PIER	4,500 PSI
FOOTINGS	3,000 PSI
- PLACE 1/2" EXPANSION JOINT MATERIAL BETWEEN EDGES OF SLABS AND VERTICAL SURFACES UNLESS NOTED OTHERWISE.
- REINFORCING STEEL SHALL CONFORM TO ASTM A 615, AND SHALL BE GRADE 60 U.N.O.
- WELDED WIRE REINFORCING SHALL BE NEW BILLET STEEL, COLD DRAWN CONFORMING TO ASTM SPECIFICATIONS A 1064 AND SHALL BE PROVIDED IN FLAT SHEETS.
- REINFORCING BAR LAP SPLICES AND HOOK DIMENSIONS SHALL BE AS REQUIRED PER THE SCHEDULE ON SHEET S-001 UNLESS NOTED OTHERWISE.
- WALL REINFORCING SHALL BE CONTINUOUS THROUGH ADJACENT FOOTINGS.
- THE CONTRACTOR SHALL NOTIFY THE ARCHITECT/ENGINEER FAR ENOUGH IN ADVANCE (48 HOURS MIN.) OF EACH CONCRETE POUR TO ALLOW AMPLE TIME TO CHECK THE LAYOUT OF THE STEEL BEFORE THE BEGINNING OF THE ACTUAL POUR, BUT NOT PRIOR TO 90% OF THE STEEL HAVING BEEN PLACED.

STRUCTURAL STEEL:

- UNLESS NOTED OTHERWISE, DESIGN, DETAILING, FABRICATION, AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH AISC 303-22 AND 360-16.
- ALL STEEL WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A-992 (F_y=50 KSI MIN.). ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A-36, U.N.O.
- TUBE STEEL SHALL CONFORM TO ASTM A500, GRADE C (F_y=50 KSI).
- STEEL PIPE SHALL CONFORM TO ASTM A53, GRADE B.
- ALL WELDING SHALL BE DONE BY WELDERS CURRENTLY CERTIFIED BY THE AMERICAN WELDING SOCIETY (AWS) AS HAVING PASSED AWS QUALIFICATION TESTS FOR THE TYPE OF WELDING THEY ARE TO PERFORM. ALL WELDERS SHALL USE E70XX ELECTRODES AND SHALL CONFORM TO AWS STANDARDS.
- ALL COPES, BLOCKS, CUTS, CUT-OUTS, AND OTHER CUTTING OF STRUCTURAL MEMBERS SHALL HAVE ALL RE-ENTRANT CORNERS SHAPED, NOTCH-FREE, TO A RADIUS OF AT LEAST 1/2" INCH.
- SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED WITHOUT PRIOR APPROVAL OF THE ENGINEER AS TO LOCATION AND TYPE OF SPLICE TO BE MADE. ANY MEMBER HAVING A SPLICE NOT SHOWN AND DETAILED ON SHOP DRAWINGS WILL BE REJECTED.

STRUCTURAL STEEL CONNECTIONS:

- STEEL FRAMING CONNECTION DESIGN NOT SPECIFICALLY NOTED ON PLANS IS A DELEGATED DESIGN. ALL CONNECTIONS SHALL BE DESIGNED AND DETAILED BY A PROFESSIONAL ENGINEER HIRED BY THE STEEL FABRICATOR. CONNECTION CALCULATIONS, SIGNED AND SEALED BY THE CONNECTION ENGINEER, SHALL BE SUBMITTED TO THE EOR FOR REVIEW.
- STRUCTURAL STEEL CONNECTIONS SHALL BE DESIGNED BY A LICENSED PROFESSIONAL ENGINEER IN ACCORDANCE WITH OPTION (3) OF SECTION 3.1.2 OF AISC 303-10. CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES. ALL CONNECTION DESIGN AND DETAILING SHALL CONFORM TO AISC 303-10 LOAD AND RESISTANCE FACTOR DESIGN (LRFD), UNLESS NOTED OTHERWISE.
- ALL STEEL DETAILS AND CONNECTIONS SHALL BE IN ACCORDANCE WITH THE REQUIREMENTS OF AISC 360-10.
- UNLESS NOTED OTHERWISE, ALL STRUCTURAL STEEL CONNECTIONS SHALL BE BEARING TYPE "N" USING 3/4" DIAMETER ASTM A 325 HIGH STRENGTH BOLTS (MINIMUM OF 2 PER CONNECTION).
- UNLESS NOTED OTHERWISE, DETAILS INDICATED ON THE DRAWINGS DEPICT GENERAL CRITERIA FOR CONNECTION DESIGN AND DETAILING. THESE DETAILS ARE NOT INTENDED TO DESCRIBE ANY SPECIFIC ELEMENTS OF THE CONNECTIONS INCLUDING, BUT NOT LIMITED TO, CONNECTION GEOMETRY, ELEMENT THICKNESS, WELD SIZE OR LENGTH, AND BOLT QUANTITY. CONNECTION WORK POINT SHALL BE THE INTERSECTION OF MEMBER CENTERLINES UNLESS NOTED OTHERWISE.
- ERECTION AIDS ARE NOT DEPICTED ON THE DRAWINGS. PROVIDE ERECTION AIDS AS REQUIRED AND REMOVE THEM AFTER WORK IS COMPLETE.
- ALL SHEAR CONNECTIONS SHALL BE BOLTED FRAMED BEAM CONNECTIONS DESIGNED FOR THE LOADS INDICATED ON THE DRAWINGS OR A MINIMUM 14 KIPS (ULTIMATE LOAD). DEVELOP THE LARGER OF THE BEAM SHEAR REACTIONS SCHEDULED, SHOWN ON THE PLANS, OR SHOWN ON THE DETAILS.
- ALL BEAM REACTIONS, AXIAL FORCES, AND MOMENTS ACT CONCURRENTLY UNLESS NOTED OTHERWISE. BEAM REACTIONS ACT IN THE GRAVITY DIRECTION. AXIAL FORCES AND MOMENTS SHALL BE CONSIDERED REVERSIBLE.
- ALL BOLTED MOMENT AND AXIAL CONNECTIONS SHALL EMPLOY PRETENSIONED BOLTS IN STANDARD HOLES.
- THE USE OF OVERSIZED OR SLOTTED HOLES FOR ANY CONNECTIONS IS PROHIBITED UNLESS SPECIFICALLY NOTED ON THE DRAWINGS.

STEEL DECK:

- TYPICAL ROOF DECK SHALL BE 1-1/2" DEEP, WIDE RIB, GALVANIZED STEEL DECK AND SHALL HAVE THE FOLLOWING MINIMUM SECTION PROPERTIES:

A. 22 GAUGE; Sp=0.169 IN/FT, Sn=0.172 IN/FT, AND I=0.155 IN ⁴ /FT.

- CHALKLINES OR OTHER METHODS SHALL BE USED TO ENSURE THAT DECK WELDS ARE ALIGNED WITH AND WILL OCCUR OVER THE TOP OF STEEL JOISTS OR TRUSSES. EXCESSIVE WELD BLOWTHROUGH IN THE DECK DUE TO MISALIGNMENT OF WELDS OR EXCESSIVE WELD HEAT WILL NOT BE PERMITTED. IF, IN THE OPINION OF THE ARCHITECT (OR HIS APPROVED REPRESENTATIVE), EXCESSIVE BLOWTHROUGH HAS OCCURRED, THEN THE CONTRACTOR SHALL REPLACE THE DAMAGED DECK AT HIS OWN EXPENSE.

MASONRY:

- ALL MASONRY CONSTRUCTION SHALL CONFORM TO THE REQUIREMENTS OF "BUILDING CODE REQUIREMENTS FOR MASONRY STRUCTURES" (ACI 530-13/ASCE 5-13/TMS 402-13) AND "SPECIFICATION FOR MASONRY STRUCTURES" (ACI 530.1-13/ASCE 6-13/TMS 602-13).
- CONCRETE MASONRY UNITS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 90.
- MINIMUM REQUIRED COMPRESSIVE STRENGTH OF MASONRY ASSEMBLAGE, F_m, AT 28 DAYS SHALL BE 2000 PSI.
- MORTAR SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 270 FOR JOB-MIXED MORTAR AND ASTM C 1142 FOR READY MIXED MORTAR AND SHALL BE TYPE S.
- GROUT FOR HOLLOW MASONRY UNITS SHALL CONFORM TO THE REQUIREMENTS OF ASTM C 476 AND SHALL HAVE A 28 DAY COMPRESSIVE STRENGTH OF 3,000 PSI. JOB SITE MIXING OF GROUT IS NOT ALLOWED.

POST-INSTALLED ANCHORS:

UNLESS OTHERWISE INDICATED ON PLANS, POST-INSTALLED ANCHORS SHALL CONSIST OF THE FOLLOWING ANCHOR TYPES, OR APPROVED EQUAL:

	ADHESIVE ANCHOR	MECHANICAL ANCHOR
SOLID CONCRETE	HILTI HY 200 SAFE SET SYSTEM	HILTI KWIK HUS EZ

- SUBSTITUTION REQUESTS FOR ALTERNATE PRODUCTS MUST BE APPROVED IN WRITING BY THE STRUCTURAL ENGINEER OF RECORD PRIOR TO USE. CONTRACTOR SHALL PROVIDE CALCULATIONS DEMONSTRATING THAT THE SUBSTITUTED PRODUCT IS CAPABLE OF ACHIEVING THE PERFORMANCE VALUES OF THE SPECIFIED PRODUCT. SUBSTITUTIONS WILL BE EVALUATED BY THEIR HAVING AN ICC ESR SHOWING COMPLIANCE WITH THE RELEVANT BUILDING CODE.
- INSTALL ANCHORS PER THE MANUFACTURER INSTRUCTIONS, AS INCLUDED IN THE ANCHOR PACKAGING.
- THE CONTRACTOR SHALL ARRANGE AN ANCHOR MANUFACTURER'S REPRESENTATIVE TO PROVIDE ONSITE INSTALLATION TRAINING FOR ALL OF THEIR ANCHORING PRODUCTS SPECIFIED. THE STRUCTURAL ENGINEER OF RECORD MUST RECEIVE DOCUMENTED CONFIRMATION THAT ALL OF THE CONTRACTOR'S PERSONNEL WHO INSTALL ANCHORS ARE TRAINED PRIOR TO THE COMMENCEMENT OF INSTALLING ANCHORS.
- ANCHOR CAPACITY IS DEPENDANT UPON SPACING BETWEEN ADJACENT ANCHORS AND PROXIMITY OF ANCHORS TO EDGE OF CONCRETE. INSTALL ANCHORS IN ACCORDANCE WITH SPACING AND EDGE CLEARANCES INDICATED ON THE DRAWINGS.
- EXISTING REINFORCING BARS IN THE CONCRETE STRUCTURE MAY CONFLICT WITH SPECIFIC ANCHOR LOCATIONS. UNLESS NOTED ON THE DRAWINGS THAT THE BARS CAN BE CUT, THE CONTRACTOR SHALL REVIEW THE EXISTING STRUCTURAL DRAWINGS AND SHALL UNDERTAKE TO LOCATE THE POSITION OF THE REINFORCING BARS AT THE LOCATIONS OF THE CONCRETE ANCHORS.
- FOR METAL DECK ATTACHMENT, STEEL ROOF/DECK SHALL BE ATTACHED TO SUPPORTING STEEL MEMBERS USING HILTI MECHANICAL FASTENERS. USE HILTI MECHANICAL DECK FASTENER X-HS24 WHERE THE STEEL BASE MATERIAL THICKNESS IS 1/8" ≤ t ≤ 3/8" OR X-ENP-19 L15 WHERE THE STEEL BASE MATERIAL THICKNESS IS t > 1/4". THE ACTUAL BASE MATERIAL THICKNESS SHOULD BE VERIFIED ON-SITE PRIOR TO INSTALLING THE FASTENER.

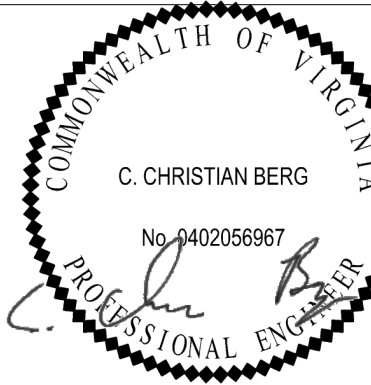
NAVY FEDERAL CREDIT UNION

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PERMIT SUBMISSION

SEAL



KEY PLAN

SCALE

REVISIONS

NO.	DESCRIPTION	DATE

DRAWN BY JLM
APPROVED BY CCB
CHECKED BY ACH
DATE 10/28/2025

TITLE

GENERAL
STRUCTURAL
NOTES

PROJECT NO. 50188638

S-001

SHEET NO.

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STRUCTURAL STEEL FRAMING SPECIFICATION

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

- A. DRAWINGS AND GENERAL PROVISIONS OF THE CONTRACT, INCLUDING GENERAL AND SUPPLEMENTARY CONDITIONS AND DIVISION 01 SPECIFICATION SECTIONS, APPLY TO THIS SECTION.

1.2 SUMMARY

- A. SECTION INCLUDES:
1. STRUCTURAL STEEL.
 2. GROUT.

1.3 DEFINITIONS

- A. STRUCTURAL STEEL: ELEMENTS OF STRUCTURAL-STEEL FRAME, AS CLASSIFIED BY AISC 303, "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."

1.4 PERFORMANCE REQUIREMENTS

- A. CONNECTIONS: PROVIDE DETAILS OF SIMPLE SHEAR CONNECTIONS REQUIRED BY THE CONTRACT DOCUMENTS TO BE SELECTED OR COMPLETED BY STRUCTURAL-STEEL FABRICATOR TO WITHSTAND LOADS INDICATED AND COMPLY WITH OTHER INFORMATION AND RESTRICTIONS INDICATED.
1. SELECT AND COMPLETE CONNECTIONS USING SCHEMATIC DETAILS INDICATED AND AISC 360.
 2. USE ASD; DATA ARE GIVEN AT SERVICE-LOAD LEVEL.

1.5 ACTION SUBMITTALS

- A. PRODUCT DATA: FOR EACH TYPE OF PRODUCT INDICATED.
- B. SHOP DRAWINGS: SHOW FABRICATION OF STRUCTURAL-STEEL COMPONENTS.
1. INCLUDE DETAILS OF CUTS, CONNECTIONS, SPLICES, CAMBER, HOLES, AND OTHER PERTINENT DATA.
 2. INCLUDE EMBEDMENT DRAWINGS.
 3. INDICATE WELDS BY STANDARD AWS SYMBOLS, DISTINGUISHING BETWEEN SHOP AND FIELD WELDS, AND SHOW SIZE, LENGTH, AND TYPE OF EACH WELD. SHOW BACKING BARS THAT ARE TO BE REMOVED AND SUPPLEMENTAL FILLET WELDS WHERE BACKING BARS ARE TO REMAIN.
 4. INDICATE TYPE, SIZE, AND LENGTH OF BOLTS, DISTINGUISHING BETWEEN SHOP AND FIELD BOLTS. IDENTIFY PRETENSIONED AND SLIP-CRITICAL HIGH-STRENGTH BOLTED CONNECTIONS.

1.6 INFORMATIONAL SUBMITTALS

- A. QUALIFICATION DATA: FOR QUALIFIED FABRICATOR AND PROFESSIONAL ENGINEER.
- B. WELDING CERTIFICATES.
- C. MILL TEST REPORTS FOR STRUCTURAL STEEL, INCLUDING CHEMICAL AND PHYSICAL PROPERTIES.
- D. PRODUCT TEST REPORTS: FOR THE FOLLOWING:
1. BOLTS, NUTS, AND WASHERS INCLUDING MECHANICAL PROPERTIES AND CHEMICAL ANALYSIS.
 2. TENSION-CONTROL, HIGH-STRENGTH BOLT-NUT-WASHER ASSEMBLIES.
 3. SHEAR STUD CONNECTORS.
 4. NONSHRINK GROUT.
- E. SOURCE QUALITY-CONTROL REPORTS.

1.7 QUALITY ASSURANCE

- A. INSTALLER QUALIFICATIONS: ENGAGE AN EXPERIENCED INSTALLER WHO HAS COMPLETED STRUCTURAL STEEL WORK SIMILAR IN MATERIAL, DESIGN, AND EXTENT TO THAT INDICATED FOR THIS PROJECT AND WITH A RECORD OF SUCCESSFUL IN-SERVICE PERFORMANCE.
- B. FABRICATOR QUALIFICATIONS: ENGAGE A FIRM EXPERIENCED IN FABRICATING STRUCTURAL STEEL SIMILAR TO THAT INDICATED FOR THIS PROJECT AND WITH A RECORD OF SUCCESSFUL IN-SERVICE PERFORMANCE, AS WELL AS SUFFICIENT PRODUCTION CAPACITY TO FABRICATE STRUCTURAL STEEL WITHOUT DELAYING THE WORK.
- C. WELDING QUALIFICATIONS: QUALIFY PROCEDURES AND PERSONNEL ACCORDING TO AWS D1.1, "STRUCTURAL WELDING CODE - STEEL."
1. WELDERS AND WELDING OPERATORS PERFORMING WORK ON BOTTOM-FLANGE, DEMAND-CRITICAL WELDS SHALL PASS THE SUPPLEMENTAL WELDER QUALIFICATION TESTING, AS REQUIRED BY AWS D1.8. FCAW-S AND FCAW-G SHALL BE CONSIDERED SEPARATE PROCESSES FOR WELDING PERSONNEL QUALIFICATION.
- D. COMPLY WITH APPLICABLE PROVISIONS OF THE FOLLOWING SPECIFICATIONS AND DOCUMENTS:
1. AISC 303.
 2. AISC 360.
 3. RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS."

1.8 DELIVERY, STORAGE, AND HANDLING

- A. STORE MATERIALS TO PERMIT EASY ACCESS FOR INSPECTION AND IDENTIFICATION. KEEP STEEL MEMBERS OFF GROUND AND SPACED BY USING PALLETS, DUNNAGE, OR OTHER SUPPORTS AND SPACERS. PROTECT STEEL MEMBERS AND PACKAGED MATERIALS FROM CORROSION AND DETEIORATION.
1. DO NOT STORE MATERIALS ON STRUCTURE IN A MANNER THAT MIGHT CAUSE DISTORTION, DAMAGE, OR OVERLOAD TO MEMBERS OR SUPPORTING STRUCTURES. REPAIR OR REPLACE DAMAGED MATERIALS OR STRUCTURES AS DIRECTED.
- B. STORE FASTENERS IN A PROTECTED PLACE IN SEALED CONTAINERS WITH MANUFACTURER'S LABELS INTACT.
1. FASTENERS MAY BE REPACKAGED PROVIDED DESIGN-BUILDER'S TESTING AND INSPECTING AGENCY OBSERVES REPACKAGING AND SEALS CONTAINERS.
 2. CLEAN AND RELUBRICATE BOLTS AND NUTS THAT BECOME DRY OR RUSTY BEFORE USE.
 3. COMPLY WITH MANUFACTURERS' WRITTEN RECOMMENDATIONS FOR CLEANING AND LUBRICATING ASTM F 1852 FASTENERS AND FOR RETESTING FASTENERS AFTER LUBRICATION.

1.9 COORDINATION

- A. COORDINATE INSTALLATION OF ANCHORAGE ITEMS TO BE EMBEDDED IN OR ATTACHED TO OTHER CONSTRUCTION WITHOUT DELAYING THE WORK. PROVIDE SETTING DIAGRAMS, SHEET METAL TEMPLATES, INSTRUCTIONS, AND DIRECTIONS FOR INSTALLATION.

PART 2 - PRODUCTS

2.1 STRUCTURAL STEEL MATERIALS

- A. W-SHAPES: ASTM A 992, GRADE 50.
- B. CHANNELS, ANGLES: ASTM A 36.
- C. PLATE AND BAR: ASTM A 36.
- D. COLD-FORMED HOLLOW STRUCTURAL SECTIONS: ASTM A 500, GRADE C (FY=46 KSI MINIMUM), STRUCTURAL TUBING.
- E. STEEL PIPE: ASTM A 53, TYPE E OR S, GRADE B.
- F. WELDING ELECTRODES: COMPLY WITH AWS REQUIREMENTS.

2.2 BOLTS, CONNECTORS, AND ANCHORS

- A. HIGH-STRENGTH BOLTS, NUTS, AND WASHERS: ASTM A 325, TYPE 1, HEAVY-HEX STEEL STRUCTURAL BOLTS; ASTM A 563, GRADE C, HEAVY-HEX CARBON-STEEL NUTS; AND ASTM F 436, TYPE 1, HARDENED CARBON-STEEL WASHERS, ALL WITH PLAIN FINISH.
- B. TENSION-CONTROL, HIGH-STRENGTH BOLT-NUT-WASHER ASSEMBLIES: ASTM F 1852, TYPE 1, HEAVY-HEX HEAD ASSEMBLIES CONSISTING OF STEEL STRUCTURAL BOLTS WITH SPLINED ENDS, HEAVY-HEX CARBON-STEEL NUTS, AND HARDENED CARBON-STEEL WASHERS.
1. FINISH: PLAIN
- C. SHEAR CONNECTORS: ASTM A 108, GRADES 1015 THROUGH 1020, HEADED-STUD TYPE, COLD-FINISHED CARBON STEEL; AWS D1.1, TYPE B.
- D. UNHEADED ANCHOR RODS: ASTM F 1554, GRADE 36.
1. CONFIGURATION: STRAIGHT.
 2. NUTS: ASTM A 563 HEAVY-HEX CARBON STEEL.
 3. PLATE WASHERS: ASTM A 36 CARBON STEEL.
 4. WASHERS: ASTM F 436, TYPE 1, HARDENED CARBON STEEL.
 5. FINISH: PLAIN.
- E. HEADED ANCHOR RODS: ASTM F 1554, GRADE 36, STRAIGHT.
1. NUTS: ASTM A 563 HEAVY-HEX CARBON STEEL.
 2. PLATE WASHERS: ASTM A 36 CARBON STEEL.
 3. WASHERS: ASTM F 436, TYPE 1, HARDENED CARBON STEEL.
 4. FINISH: PLAIN.
- F. THREADED RODS: ASTM A 36.
1. NUTS: ASTM A 563 HEAVY-HEX CARBON STEEL.
 2. WASHERS: ASTM F 436, TYPE 1, HARDENED CARBON STEEL.
 3. FINISH: PLAIN.

2.3 PRIMER

- A. PRIMER: SSPC-PAINT 25 BCS, TYPE, ZINC OXIDE, ALKYD, LINSEED OIL PRIMER, COLOR: GREY.
- B. GALVANIZING REPAIR PAINT: ASTM A 780.

2.4 GROUT

- A. NONMETALLIC, SHRINKAGE-RESISTANT GROUT: ASTM C 1107, FACTORY-PACKAGED, NONMETALLIC AGGREGATE GROUT, NONCORROSIVE AND NONSTAINING, MIXED WITH WATER TO CONSISTENCY SUITABLE FOR APPLICATION AND A 30-MINUTE WORKING TIME.

2.5 FABRICATION

- A. STRUCTURAL STEEL: FABRICATE AND ASSEMBLE IN SHOP TO GREATEST EXTENT POSSIBLE. FABRICATE ACCORDING TO AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES" AND AISC 360.
1. CAMBER STRUCTURAL-STEEL MEMBERS WHERE INDICATED.
 2. FABRICATE BEAMS WITH ROLLING CAMBER UP.
 3. IDENTIFY HIGH-STRENGTH STRUCTURAL STEEL ACCORDING TO ASTM A 6 AND MAINTAIN MARKINGS UNTIL STRUCTURAL STEEL HAS BEEN ERECTED.
 4. MARK AND MATCH-MARK MATERIALS FOR FIELD ASSEMBLY.
 5. COMPLETE STRUCTURAL-STEEL ASSEMBLIES, INCLUDING WELDING OF UNITS, BEFORE STARTING SHOP-PRIMING OPERATIONS.
- B. THERMAL CUTTING: PERFORM THERMAL CUTTING BY MACHINE TO GREATEST EXTENT POSSIBLE.
1. PLANE THERMALLY CUT EDGES TO BE WELDED TO COMPLY WITH REQUIREMENTS IN AWS D1.1.
- C. BOLT HOLES: CUT, DRILL, OR PUNCH STANDARD BOLT HOLES PERPENDICULAR TO METAL SURFACES.
- D. FINISHING: ACCURATELY FINISH ENDS OF COLUMNS AND OTHER MEMBERS TRANSMITTING BEARING LOADS.
- E. CLEANING: CLEAN AND PREPARE STEEL SURFACES THAT ARE TO REMAIN UNPAINTED ACCORDING TO SSPC-SP 1, "SOLVENT CLEANING OR SSPC-SP 2, "HAND TOOL CLEANING."
- F. SHEAR CONNECTORS: PREPARE STEEL SURFACES AS RECOMMENDED BY MANUFACTURER OF SHEAR CONNECTORS. USE AUTOMATIC END WELDING OF HEADED-STUD SHEAR CONNECTORS ACCORDING TO AWS D1.1 AND MANUFACTURERS' WRITTEN INSTRUCTIONS.
- G. HOLES: PROVIDE HOLES REQUIRED FOR SECURING OTHER WORK TO STRUCTURAL STEEL AND FOR OTHER WORK TO PASS THROUGH STEEL FRAMING MEMBERS.
1. CUT, DRILL, OR PUNCH HOLES PERPENDICULAR TO STEEL SURFACES. DO NOT THERMALLY CUT BOLT HOLES OR ENLARGE HOLES BY BURNING.
 2. BASEPLATE HOLES: CUT, DRILL, MECHANICALLY THERMAL CUT, OR PUNCH HOLES PERPENDICULAR TO STEEL SURFACES.
 3. WELD THREADED NUTS TO FRAMING AND OTHER SPECIALTY ITEMS INDICATED TO RECEIVE OTHER WORK.

2.6 SHOP CONNECTIONS

- A. HIGH-STRENGTH BOLTS. SHOP INSTALL HIGH-STRENGTH BOLTS ACCORDING TO RCSC'S "SPECIFICATION FOR STRUCTURAL JOINTS USING ASTM A 325 OR A 490 BOLTS" FOR TYPE OF BOLT AND TYPE OF JOINT SPECIFIED.
1. JOINT TYPE: SNUG TIGHTENED UNLESS INDICATED AS PRETENSIONED OR SLIP CRITICAL ON DRAWINGS.
- B. WELD CONNECTIONS: COMPLY WITH AWS D1.1 FOR TOLERANCES, APPEARANCES, WELDING PROCEDURE SPECIFICATIONS, WELD QUALITY, AND METHODS USED IN CORRECTING WELDING WORK.
1. ASSEMBLE AND WELD BUILT-UP SECTIONS BY METHODS THAT WILL MAINTAIN TRUE ALIGNMENT OF AXES WITHOUT EXCEEDING TOLERANCES IN AISC 303 FOR MILL MATERIAL.

2.7 SHOP PRIMING

- A. SHOP PRIME STEEL SURFACES EXCEPT THE FOLLOWING:
1. SURFACES EMBEDDED IN CONCRETE OR MORTAR. EXTEND PRIMING OF PARTIALLY EMBEDDED MEMBERS TO A DEPTH OF 2 INCHES.
 2. SURFACES TO BE FIELD WELDED.
 3. SURFACES OF HIGH-STRENGTH BOLTED, SLIP-CRITICAL CONNECTIONS.
 4. SURFACES TO RECEIVE SPRAYED FIRE-RESISTIVE MATERIALS (APPLIED FIREPROOFING).
 5. GALVANIZED SURFACES.
 6. SURFACES ENCLOSED IN INTERIOR CONSTRUCTION.
- B. SURFACE PREPARATION: CLEAN SURFACES TO BE PAINTED. REMOVE LOOSE RUST AND MILL SCALE AND SPATTER, SLAG, OR FLUX DEPOSITS. PREPARE SURFACES ACCORDING TO THE FOLLOWING SPECIFICATIONS AND STANDARDS:
1. SSPC-SP 3, "POWER TOOL CLEANING."
- C. PRIMING: IMMEDIATELY AFTER SURFACE PREPARATION, APPLY PRIMER ACCORDING TO MANUFACTURER'S WRITTEN INSTRUCTIONS AND AT RATE RECOMMENDED BY SSPC TO PROVIDE A MINIMUM DRY FILM THICKNESS OF 1.5 MILS. USE PRIMING METHODS THAT RESULT IN FULL COVERAGE OF JOINTS, CORNERS, EDGES, AND EXPOSED SURFACES.
1. STRIPE PAINT CORNERS, CREVICES, BOLTS, WELDS, AND SHARP EDGES.
 2. APPLY TWO COATS OF SHOP PAINT TO SURFACES THAT ARE INACCESSIBLE AFTER ASSEMBLY OR ERECTION. CHANGE COLOR OF SECOND COAT TO DISTINGUISH IT FROM FIRST.

2.8 GALVANIZING

- A. HOT-DIP GALVANIZED FINISH: APPLY ZINC COATING BY THE HOT-DIP PROCESS TO STRUCTURAL STEEL ACCORDING TO ASTM A 123.
1. FILL VENT AND DRAIN HOLES THAT WILL BE EXPOSED IN THE FINISHED WORK UNLESS THEY WILL FUNCTION AS WEEP HOLES. BY PLUGGING WITH ZINC SOLDER AND FILING OFF SMOOTH.
 2. GALVANIZE LINTELS AND SHELF ANGLES IN EXTERIOR WALLS AND OTHER STRUCTURAL STEEL AS INDICATED ON DRAWINGS.

PART 3 - EXECUTION

3.1 EXAMINATION

- A. VERIFY, WITH STEEL ERECTOR PRESENT, ELEVATIONS OF CONCRETE- AND MASONRY-BEARING SURFACES AND LOCATIONS OF ANCHOR RODS, BEARING PLATES, AND OTHER EMBEDMENTS FOR COMPLIANCE WITH REQUIREMENTS.
1. PREPARE A CERTIFIED SURVEY OF BEARING SURFACES, ANCHOR RODS, BEARING PLATES, AND OTHER EMBEDMENTS SHOWING DIMENSIONS, LOCATIONS, ANGLES, AND ELEVATIONS.
- B. PROCEED WITH INSTALLATION ONLY AFTER UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED.

3.2 PREPARATION

- A. PROVIDE TEMPORARY SHORES, GUYS, BRACES, AND OTHER SUPPORTS DURING ERECTION TO KEEP STRUCTURAL STEEL SECURE, PLUMB, AND IN ALIGNMENT AGAINST TEMPORARY CONSTRUCTION LOADS AND LOADS EQUAL IN INTENSITY TO DESIGN LOADS. REMOVE TEMPORARY SUPPORTS WHEN PERMANENT STRUCTURAL STEEL, CONNECTIONS, AND BRACING ARE IN PLACE UNLESS OTHERWISE INDICATED.

3.3 ERECTION

- A. SET STRUCTURAL STEEL ACCURATELY IN LOCATIONS AND TO ELEVATIONS INDICATED AND ACCORDING TO AISC 303 AND AISC 360.
- B. BASEPLATES AND BEAM BEARING PLATES: CLEAN CONCRETE- AND MASONRY-BEARING SURFACES OF BOND-REDUCING MATERIALS, AND ROUGHEN SURFACES PRIOR TO SETTING PLATES. CLEAN BOTTOM SURFACE OF PLATES.
1. SET PLATES FOR STRUCTURAL MEMBERS ON WEDGES, SHIMS, OR SETTING NUTS AS REQUIRED.
 2. WELD PLATE WASHERS TO TOP OF BASEPLATE.
 3. SNUG-TIGHTEN ANCHOR RODS AFTER SUPPORTED MEMBERS HAVE BEEN POSITIONED AND PLUMBED. DO NOT REMOVE WEDGES OR SHIMS BUT, IF PROTRUDING, CUT OFF FLUSH WITH EDGE OF PLATE BEFORE PACKING WITH GROUT.
 4. PROMPTLY PACK GROUT SOLIDLY BETWEEN BEARING SURFACES AND PLATES SO NO VOIDS REMAIN. NEATLY FINISH EXPOSED SURFACES; PROTECT GROUT AND ALLOW TO CURE. COMPLY WITH MANUFACTURER'S WRITTEN INSTALLATION INSTRUCTIONS FOR SHRINKAGE-RESISTANT GROUTS.
- C. MAINTAIN ERECTION TOLERANCES OF STRUCTURAL STEEL WITHIN AISC'S "CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES."
- D. ALIGN AND ADJUST VARIOUS MEMBERS THAT FORM PART OF COMPLETE FRAME OR STRUCTURE BEFORE PERMANENTLY FASTENING. BEFORE ASSEMBLY, CLEAN BEARING SURFACES AND OTHER SURFACES THAT WILL BE IN PERMANENT CONTACT WITH MEMBERS. PERFORM NECESSARY ADJUSTMENTS TO COMPENSATE FOR DISCREPANCIES IN ELEVATIONS AND ALIGNMENT.
1. LEVEL AND PLUMB INDIVIDUAL MEMBERS OF STRUCTURE.
 2. MAKE ALLOWANCES FOR DIFFERENCE BETWEEN TEMPERATURE AT TIME OF ERECTION AND MEAN TEMPERATURE WHEN STRUCTURE IS COMPLETED AND IN SERVICE.
- E. SPLICE MEMBERS ONLY WHERE INDICATED.
- F. DO NOT USE THERMAL CUTTING DURING ERECTION UNLESS APPROVED BY ENGINEER. FINISH THERMALLY CUT SECTIONS WITHIN SMOOTHNESS LIMITS IN AWS D1.1.
- G. DO NOT ENLARGE UNFAIR HOLES IN MEMBERS BY BURNING OR USING DRIFT PINS. REAM HOLES THAT MUST BE ENLARGED TO ADMIT BOLTS.
- H. SHEAR CONNECTORS: PREPARE STEEL SURFACES AS RECOMMENDED BY MANUFACTURER OF SHEAR CONNECTORS. USE AUTOMATIC END WELDING OF HEADED-STUD SHEAR CONNECTORS ACCORDING TO AWS D1.1 AND MANUFACTURERS' WRITTEN INSTRUCTIONS.



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KEY PLAN

SCALE

REVISIONS

NO.	DESCRIPTION	DATE

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APPROVED BY _____ CCB
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DATE _____ 10/28/2025
TITLE

STRUCTURAL
STEEL FRAMING
SPECIFICATION

PROJECT NO. 50188638

S-003

SHEET NO.

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STEEL DECKING SPECIFICATION

PART 1 - GENERAL

1.1 RELATED DOCUMENTS

A. DRAWINGS AND GENERAL PROVISIONS OF THE CONTRACT, INCLUDING GENERAL AND SUPPLEMENTARY CONDITIONS AND DIVISION 01 SPECIFICATION SECTIONS, APPLY TO THIS SECTION.

1.2 SUMMARY

A. SECTION INCLUDES:
1. ROOF DECK.

1.3 ACTION SUBMITTALS

A. PRODUCT DATA: FOR EACH TYPE OF DECK, ACCESSORY, AND PRODUCT INDICATED.

B. SHOP DRAWINGS:
1. INCLUDE LAYOUT AND TYPES OF DECK PANELS, ANCHORAGE DETAILS, REINFORCING CHANNELS, PANS, CUT DECK OPENINGS, SPECIAL JOINTING, ACCESSORIES, AND ATTACHMENTS TO OTHER CONSTRUCTION.

1.4 INFORMATIONAL SUBMITTALS

A. WELDING CERTIFICATES.

B. PRODUCT CERTIFICATES: FOR EACH TYPE OF STEEL DECK.

C. PRODUCT TEST REPORTS: BASED ON EVALUATION OF COMPREHENSIVE TESTS PERFORMED BY A QUALIFIED TESTING AGENCY, INDICATING THAT EACH OF THE FOLLOWING COMPLIES WITH REQUIREMENTS:
1. POWER-ACTUATED MECHANICAL FASTENERS.

1.5 QUALITY ASSURANCE

A. WELDING QUALIFICATIONS: QUALIFY PROCEDURES AND PERSONNEL ACCORDING TO AWS D1.3, "STRUCTURAL WELDING CODE - SHEET STEEL."

1.6 DELIVERY, STORAGE, AND HANDLING

A. PROTECT STEEL DECK FROM CORROSION, DEFORMATION, AND OTHER DAMAGE DURING DELIVERY, STORAGE, AND HANDLING.

B. STACK STEEL DECK ON PLATFORMS OR PALLETS AND SLOPE TO PROVIDE DRAINAGE. PROTECT WITH A WATERPROOF COVERING AND VENTILATE TO AVOID CONDENSATION.

PART 2 - PRODUCTS

2.1 PERFORMANCE REQUIREMENTS

A. AISI SPECIFICATIONS: COMPLY WITH CALCULATED STRUCTURAL CHARACTERISTICS OF STEEL DECK ACCORDING TO AISI'S "NORTH AMERICAN SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS."
B. FIRE-RESISTANCE RATINGS: COMPLY WITH ASTM E 119; TESTING BY A QUALIFIED TESTING AGENCY. IDENTIFY PRODUCTS WITH APPROPRIATE MARKINGS OF APPLICABLE TESTING AGENCY.
1. INDICATE DESIGN DESIGNATIONS FROM UL'S "FIRE RESISTANCE DIRECTORY" OR FROM THE LISTINGS OF ANOTHER QUALIFIED TESTING AGENCY.

2.2 ROOF DECK

A. MANUFACTURERS: SUBJECT TO COMPLIANCE WITH REQUIREMENTS, AVAILABLE MANUFACTURERS OFFERING PRODUCTS THAT MAY BE INCORPORATED INTO THE WORK INCLUDE, BUT ARE NOT LIMITED TO, THE FOLLOWING:
1. ASC PROFILES, INC.; A BLUE SCOPE STEEL COMPANY.
2. CANAM UNITED STATES; CANAM GROUP INC.
3. CMC JOIST & DECK.
4. CONSOLIDATED SYSTEMS, INC.; METAL DEK GROUP.
5. EPIC METALS CORPORATION.
6. MARLYN STEEL DECKS, INC.
7. NEW MILLENNIUM BUILDING SYSTEMS, LLC.
8. Nucor Corp.; VULCRAFT GROUP.
9. ROOF DECK, INC.
10. VEROO MANUFACTURING CO.
11. WHEELING CORRUGATING COMPANY; DIV. OF WHEELING-PITTSBURGH STEEL CORPORATION.
B. TYPICAL ROOF DECK: FABRICATE PANELS, WITHOUT TOP-FLANGE STIFFENING GROOVES, TO COMPLY WITH "SDI SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK," IN SDI PUBLICATION NO. 31, AND WITH THE FOLLOWING:
1. GALVANIZED-STEEL SHEET, ASTM A 653, STRUCTURAL STEEL (SS), GRADE 33, G60 ZINC COATING.
2. DECK PROFILE: TYPE WR, WIDE RIB.
3. PROFILE DEPTH: 1-1/2 INCHES.
4. DESIGN UNCOATED-STEEL THICKNESS: 0.0295 INCH (22 GAUGE).
5. SPAN CONDITION: AS INDICATED.
6. SIDE LAPS: OVERLAPPED OR INTERLOCKING SEAM AT CONTRACTOR'S OPTION.

2.3 ACCESSORIES

A. GENERAL: PROVIDE MANUFACTURER'S STANDARD ACCESSORY MATERIALS FOR DECK THAT COMPLY WITH REQUIREMENTS INDICATED.
B. MECHANICAL FASTENERS: CORROSION-RESISTANT, LOW-VELOCITY, POWER-ACTUATED OR PNEUMATICALLY DRIVEN CARBON-STEEL FASTENERS; OR SELF-DRILLING, SELF-THREADING SCREWS.
C. SIDE-LAP FASTENERS: CORROSION-RESISTANT, HEXAGONAL WASHER HEAD, SELF-DRILLING, CARBON-STEEL SCREWS, NO. 10 MINIMUM DIAMETER.
D. MISCELLANEOUS SHEET METAL DECK ACCESSORIES: STEEL SHEET, MINIMUM YIELD STRENGTH OF 33,000 PSI, NOT LESS THAN 0.0359-INCH DESIGN UNCOATED THICKNESS, OF SAME MATERIAL AND FINISH AS DECK, OF PROFILE INDICATED OR REQUIRED FOR APPLICATION.
E. GALVANIZING REPAIR PAINT: ASTM A 780.

PART 3 - EXECUTION

3.1 EXAMINATION

A. EXAMINE SUPPORTING FRAME AND FIELD CONDITIONS FOR COMPLIANCE WITH REQUIREMENTS FOR INSTALLATION TOLERANCES AND OTHER CONDITIONS AFFECTING PERFORMANCE.

B. PROCEED WITH INSTALLATION ONLY AFTER UNSATISFACTORY CONDITIONS HAVE BEEN CORRECTED.

3.2 INSTALLATION, GENERAL

A. INSTALL DECK PANELS AND ACCESSORIES ACCORDING TO APPLICABLE SPECIFICATIONS AND COMMENTARY IN SDI PUBLICATION NO. 31, MANUFACTURER'S WRITTEN INSTRUCTIONS, AND REQUIREMENTS IN THIS SECTION.

B. INSTALL TEMPORARY SHORING BEFORE PLACING DECK PANELS IF REQUIRED TO MEET DEFLECTION LIMITATIONS.

C. LOCATE DECK BUNDLES TO PREVENT OVERLOADING OF SUPPORTING MEMBERS.

D. PLACE DECK PANELS ON SUPPORTING FRAME AND ADJUST TO FINAL POSITION WITH ENDS ACCURATELY ALIGNED AND BEARING ON SUPPORTING FRAME BEFORE BEING PERMANENTLY FASTENED. DO NOT STRETCH OR CONTRACT SIDE-LAP INTERLOCKS.

E. PLACE DECK PANELS FLAT AND SQUARE AND FASTEN TO SUPPORTING FRAME WITHOUT WARP OR DEFLECTION.

F. CUT AND NEATLY FIT DECK PANELS AND ACCESSORIES AROUND OPENINGS AND OTHER WORK PROJECTING THROUGH OR ADJACENT TO DECK.

G. PROVIDE ADDITIONAL REINFORCEMENT AND CLOSURE PIECES AT OPENINGS AS REQUIRED FOR STRENGTH, CONTINUITY OF DECK, AND SUPPORT OF OTHER WORK.

H. COMPLY WITH AWS REQUIREMENTS AND PROCEDURES FOR MANUAL SHIELDED METAL ARC WELDING, APPEARANCE AND QUALITY OF WELDS, AND METHODS USED FOR CORRECTING WELDING WORK.

I. MECHANICAL FASTENERS MAY BE USED IN LIEU OF WELDING TO FASTEN DECK UPON WRITTEN APPROVAL FROM ARCHITECT. SIZE AND SPACE MECHANICAL FASTENERS TO PROVIDE SAME STRENGTH AS INDICATED WELD PATTERN AND INSTALL ACCORDING TO DECK MANUFACTURER'S WRITTEN INSTRUCTIONS.

3.3 ROOF-DECK INSTALLATION

A. FASTEN ROOF-DECK PANELS TO STEEL SUPPORTING MEMBERS BY ARC SPOT (PUDDLE) WELDS OF THE SURFACE DIAMETER INDICATED OR ARC SEAM WELDS WITH AN EQUAL PERIMETER THAT IS NOT LESS THAN 1-1/2 INCHES LONG, AND AS FOLLOWS:
1. WELD DIAMETER: AS INDICATED ON DRAWINGS.
2. WELD SPACING: WELD EDGE AND INTERIOR RIBS OF DECK UNITS WITH A MINIMUM OF TWO WELDS PER DECK UNIT AT EACH SUPPORT. SPACE WELDS AS INDICATED ON DRAWINGS.

B. SIDE-LAP AND PERIMETER EDGE FASTENING: FASTEN SIDE LAPS AND PERIMETER EDGES OF PANELS BETWEEN SUPPORTS, AT INTERVALS NOT EXCEEDING THE LESSER OF 1/2 OF THE SPAN OR 36 INCHES, AND AS FOLLOWS:
1. MECHANICALLY FASTEN WITH SELF-DRILLING, NO. 10 DIAMETER OR LARGER, CARBON-STEEL SCREWS.

C. END BEARING: INSTALL DECK ENDS OVER SUPPORTING FRAME WITH A MINIMUM END BEARING OF 1-1/2 INCHES, WITH END JOINTS AS FOLLOWS:
1. END JOINTS: LAPPED 2 INCHES MINIMUM OR BUTTED AT CONTRACTOR'S OPTION.

3.4 FIELD QUALITY CONTROL

A. TESTING AGENCY: OWNER WILL ENGAGE A SPECIAL INSPECTOR AND QUALIFIED INDEPENDENT TESTING AND INSPECTING AGENCY TO INSPECT FIELD WELDS AND MECHANICAL FASTENER CONNECTIONS AND PREPARE TEST REPORTS.
B. INSPECTIONS: SEE STATEMENT OF SPECIAL INSPECTIONS.
C. TESTING AGENCY WILL REPORT INSPECTION RESULTS PROMPTLY AND IN WRITING TO CONTRACTOR AND ARCHITECT.
D. REMOVE AND REPLACE WORK THAT DOES NOT COMPLY WITH SPECIFIED REQUIREMENTS.
E. ADDITIONAL INSPECTING, AT CONTRACTOR'S EXPENSE, WILL BE PERFORMED TO DETERMINE COMPLIANCE OF CORRECTED WORK WITH SPECIFIED REQUIREMENTS.

3.5 PROTECTION

A. GALVANIZING REPAIRS: PREPARE AND REPAIR DAMAGED GALVANIZED COATINGS ON BOTH SURFACES OF DECK WITH GALVANIZED REPAIR PAINT ACCORDING TO ASTM A 780 AND MANUFACTURER'S WRITTEN INSTRUCTIONS.
B. PROVIDE FINAL PROTECTION AND MAINTAIN CONDITIONS TO ENSURE THAT STEEL DECK IS WITHOUT DAMAGE OR DETERIORATION AT TIME OF SUBSTANTIAL COMPLETION.



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PERMIT SUBMISSION

SEAL



KEY PLAN

SCALE

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NO.	DESCRIPTION	DATE

DRAWN BY JLM
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DATE 10/28/2025

TITLE

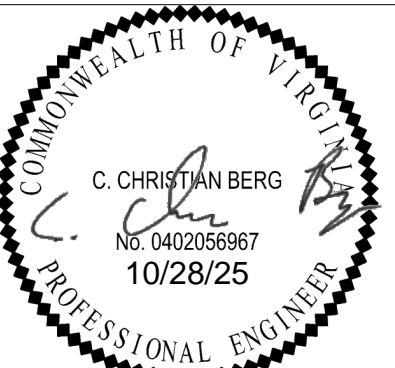
STEEL DECK
SPECIFICATION

PROJECT NO. 50188638

S-004

SHEET NO.

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KEY PLAN

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TITLE

STRUCTURAL FOUNDATION DETAILS

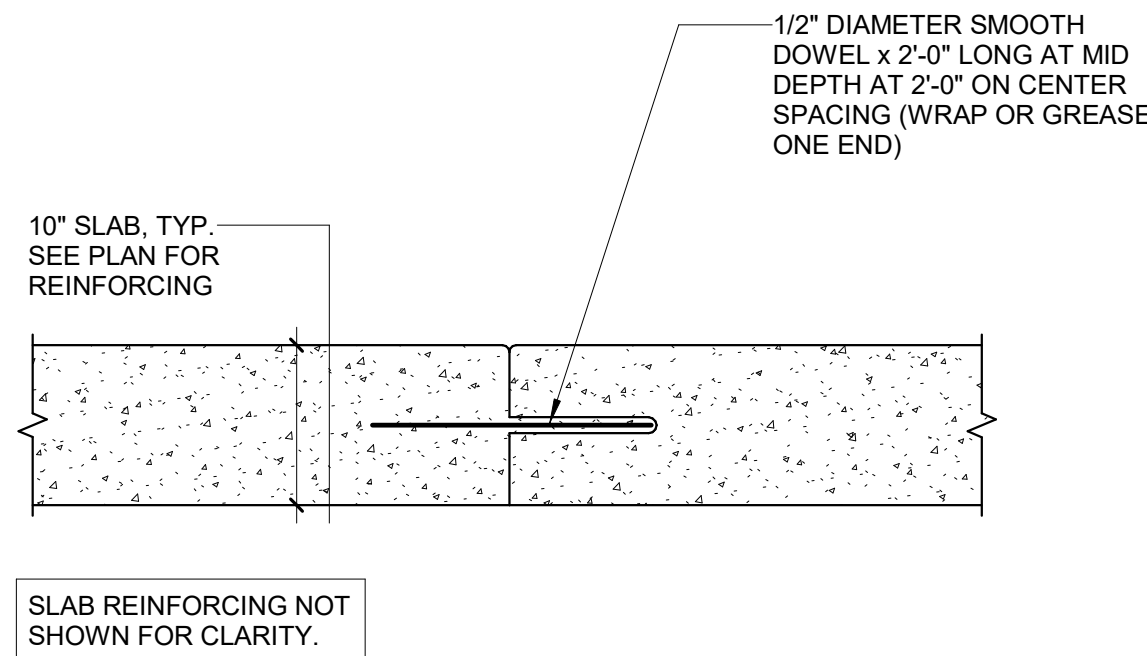
PROJECT NO. 50188638

S-200

SHEET NO.

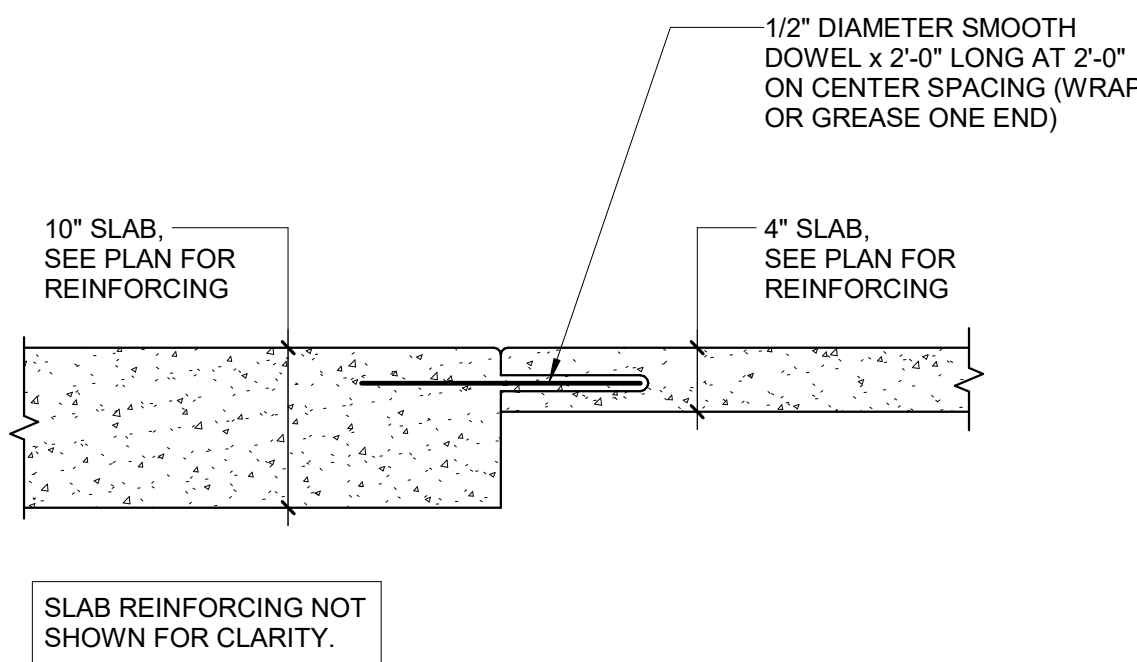
9 DOWELED CONTROL JOINT DETAIL

Scale: 1" = 1'-0"



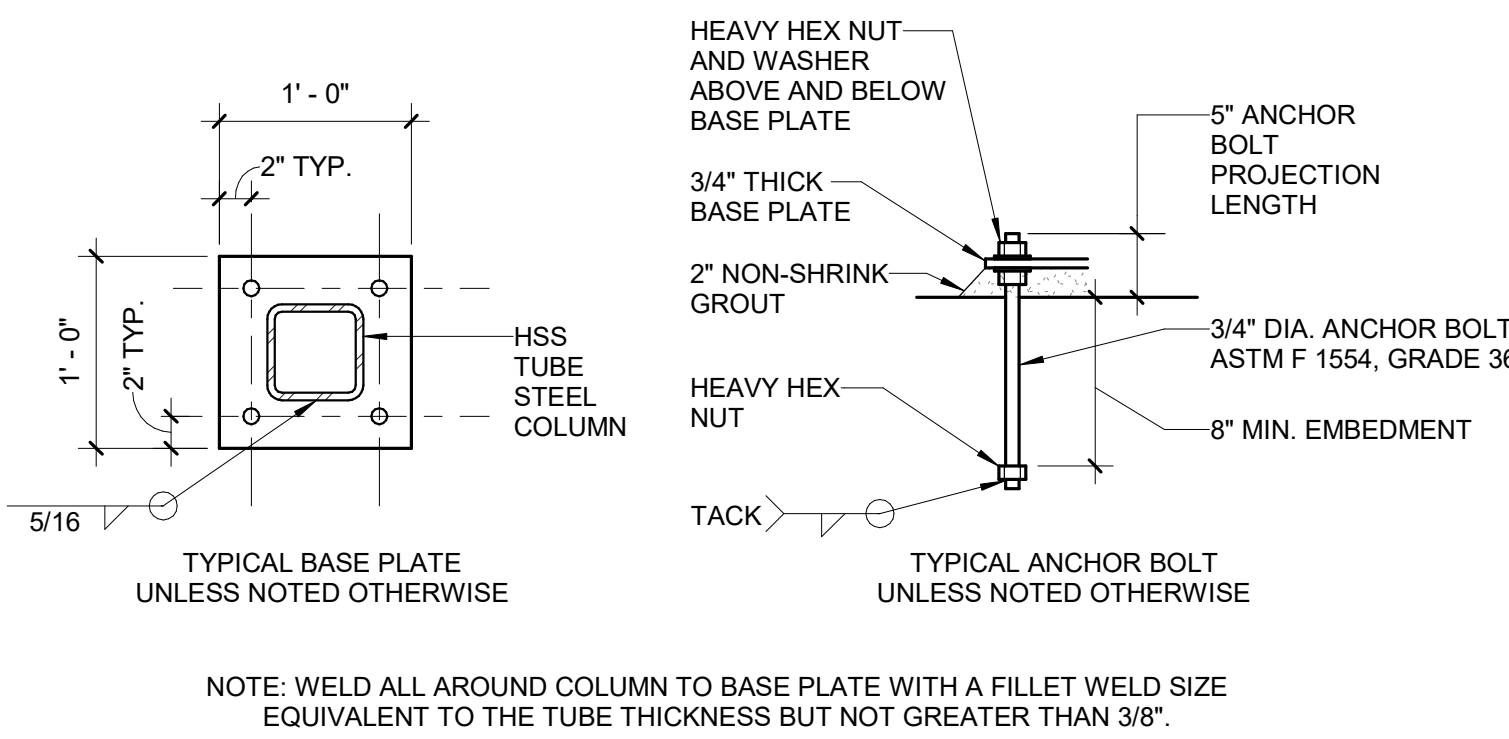
8 SLAB TRANSITION DETAIL

Scale: 1" = 1'-0"



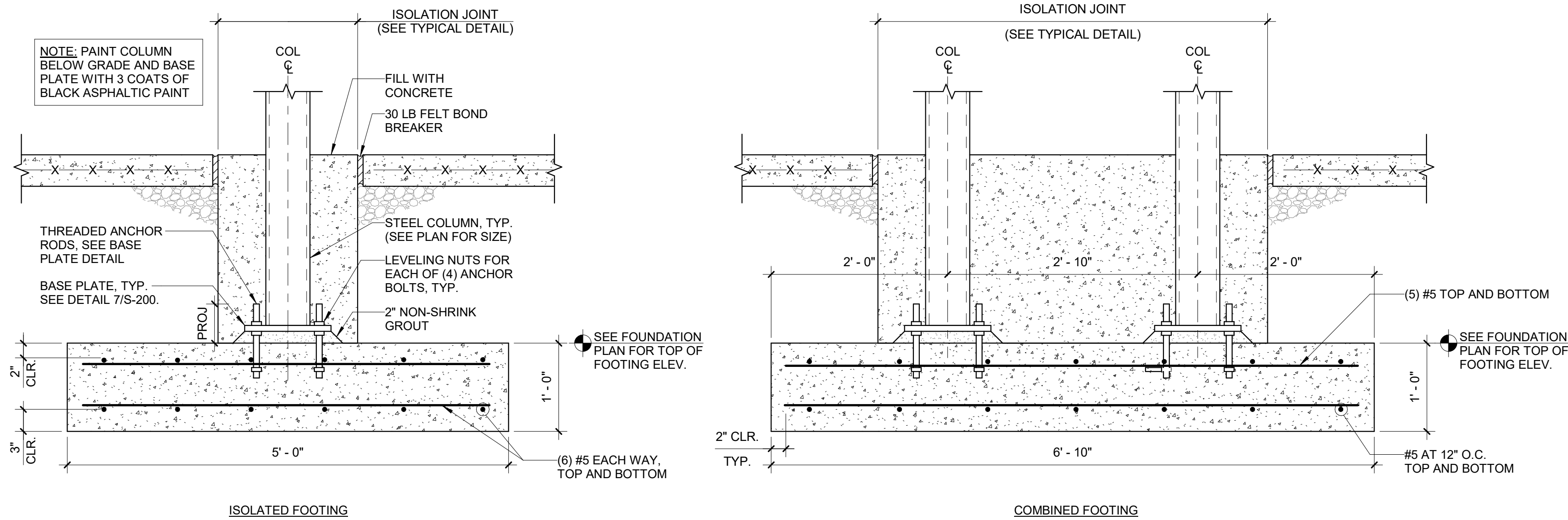
7 BASE PLATE DETAIL

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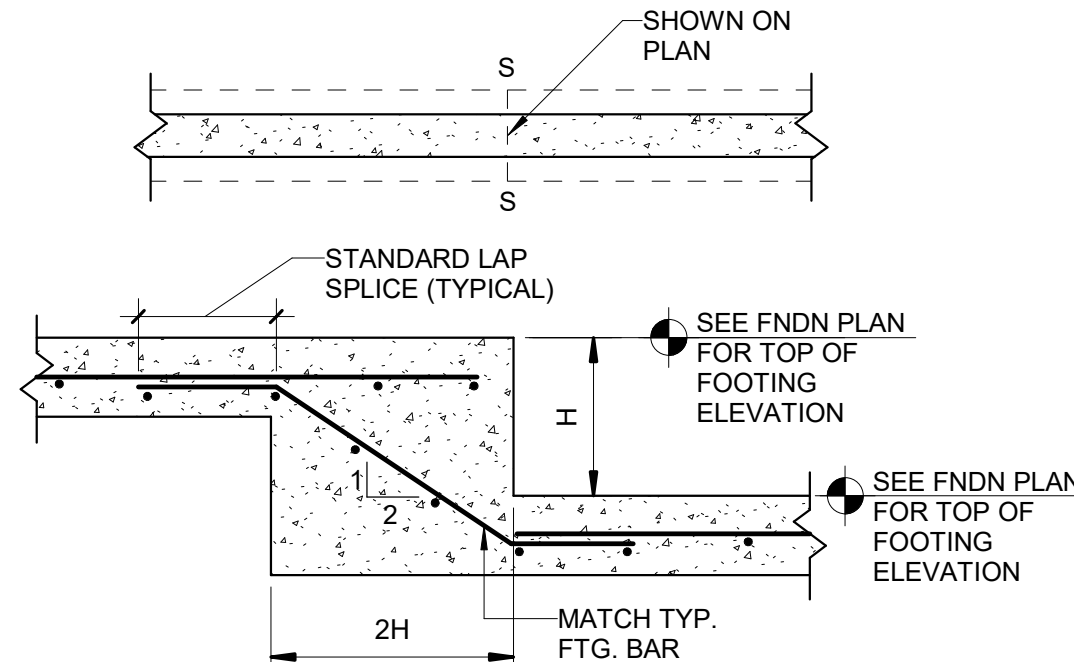
6 HSS FOOTING DETAIL

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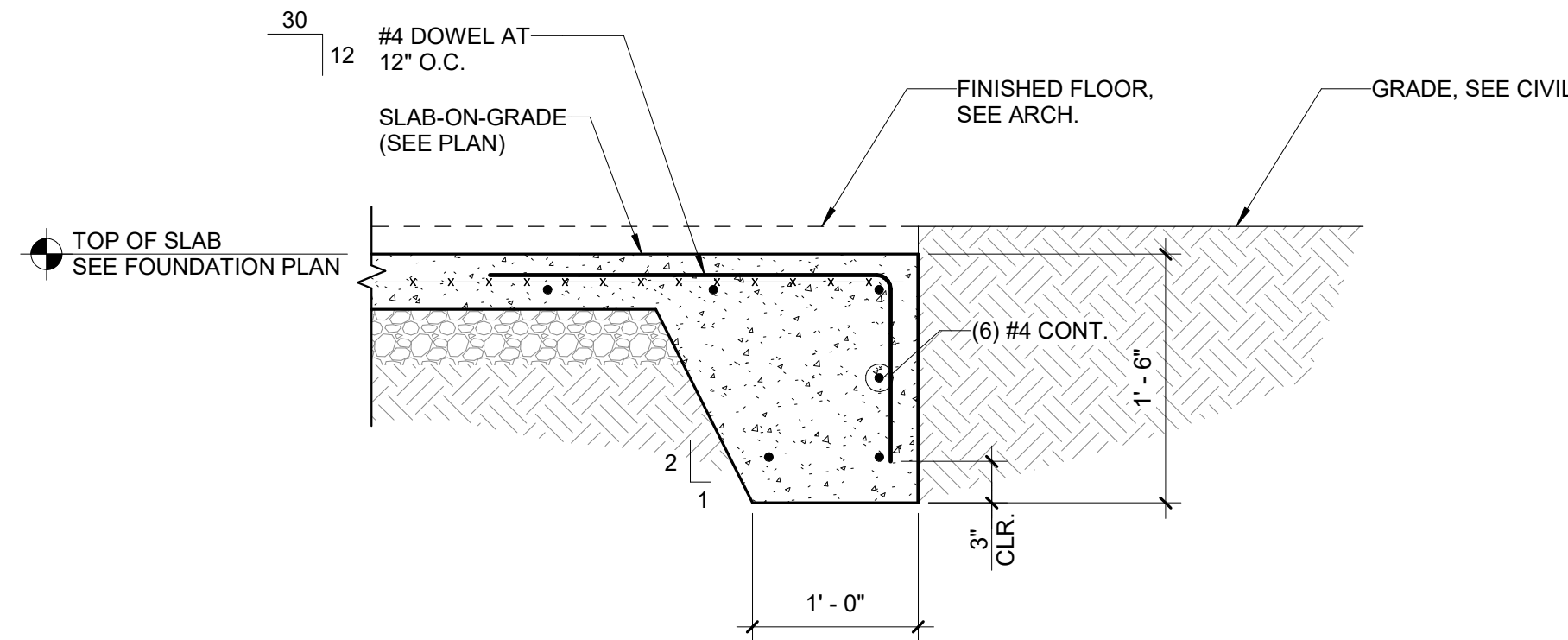
5 STEP FOOTING DETAIL

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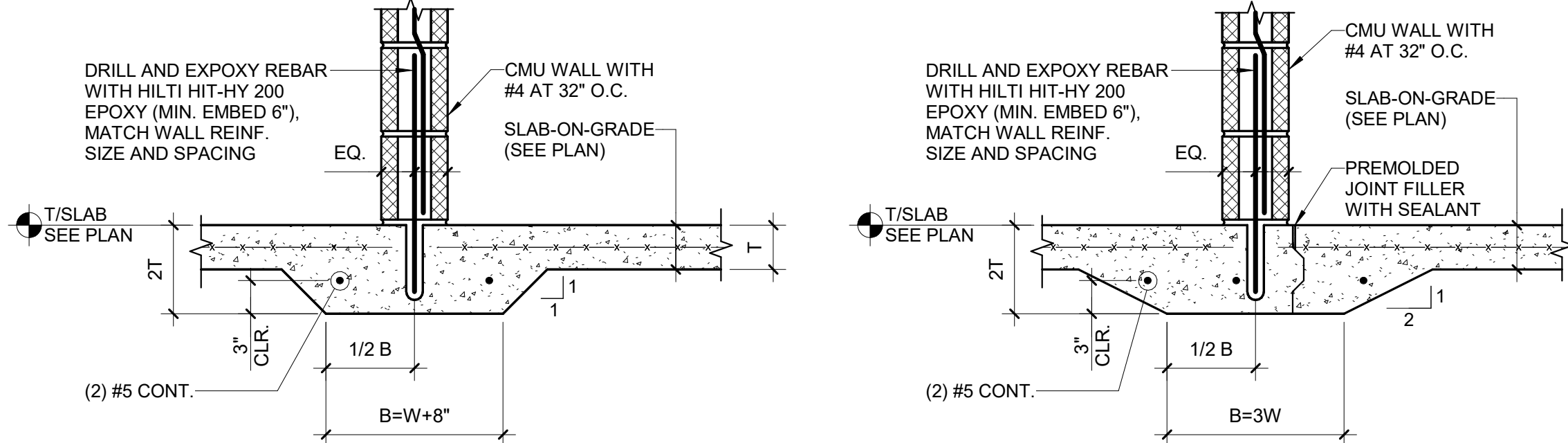
4 SLAB TURN-DOWN

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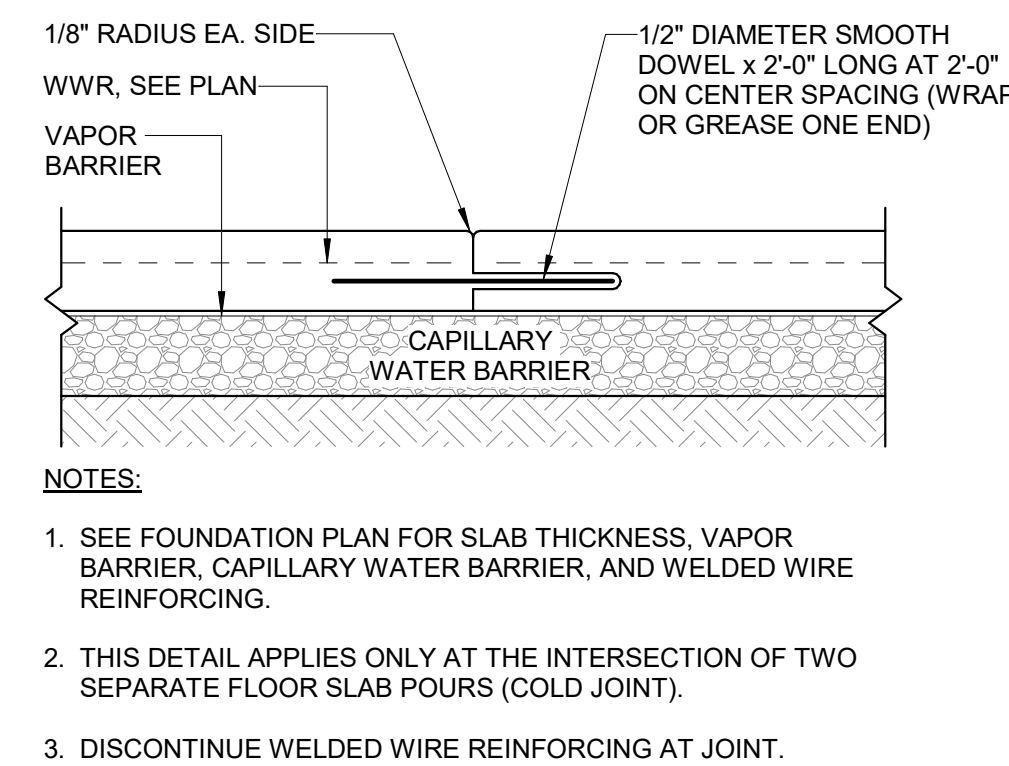
3 THICKENED SLAB AT CMU WALL

Scale: 1" = 1'-0"



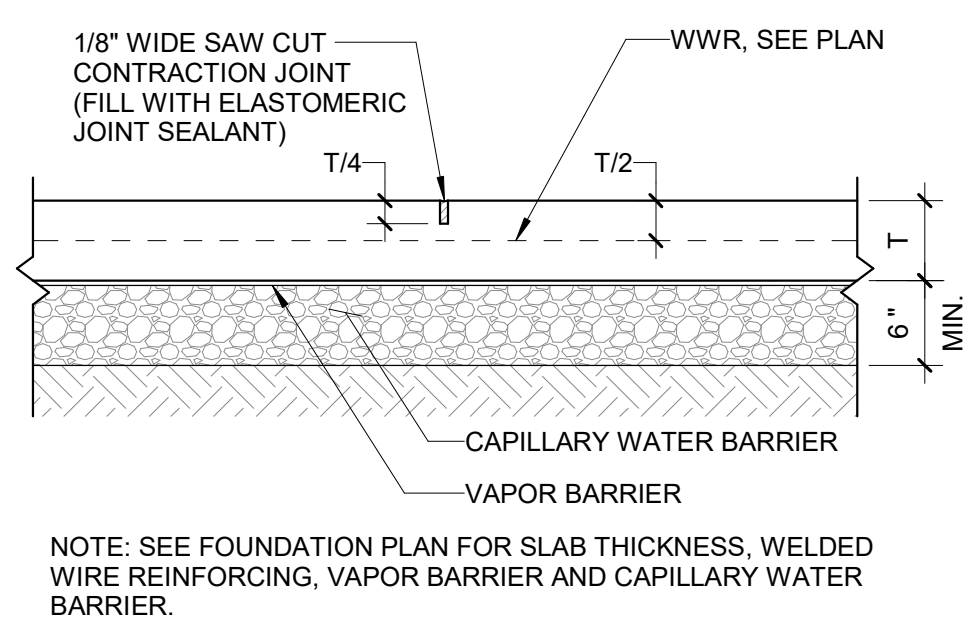
2 SLAB CONSTRUCTION JOINT

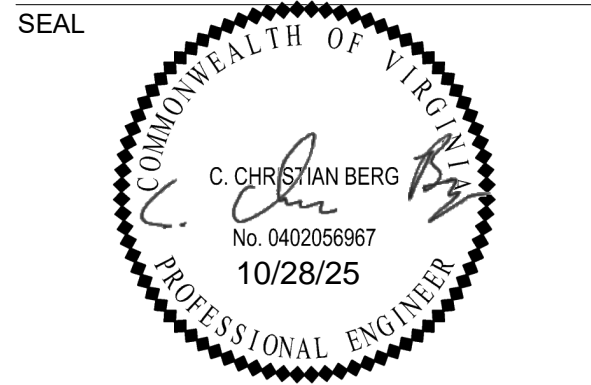
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1 SLAB CONTROL JOINT

Scale: 1" = 1'-0"





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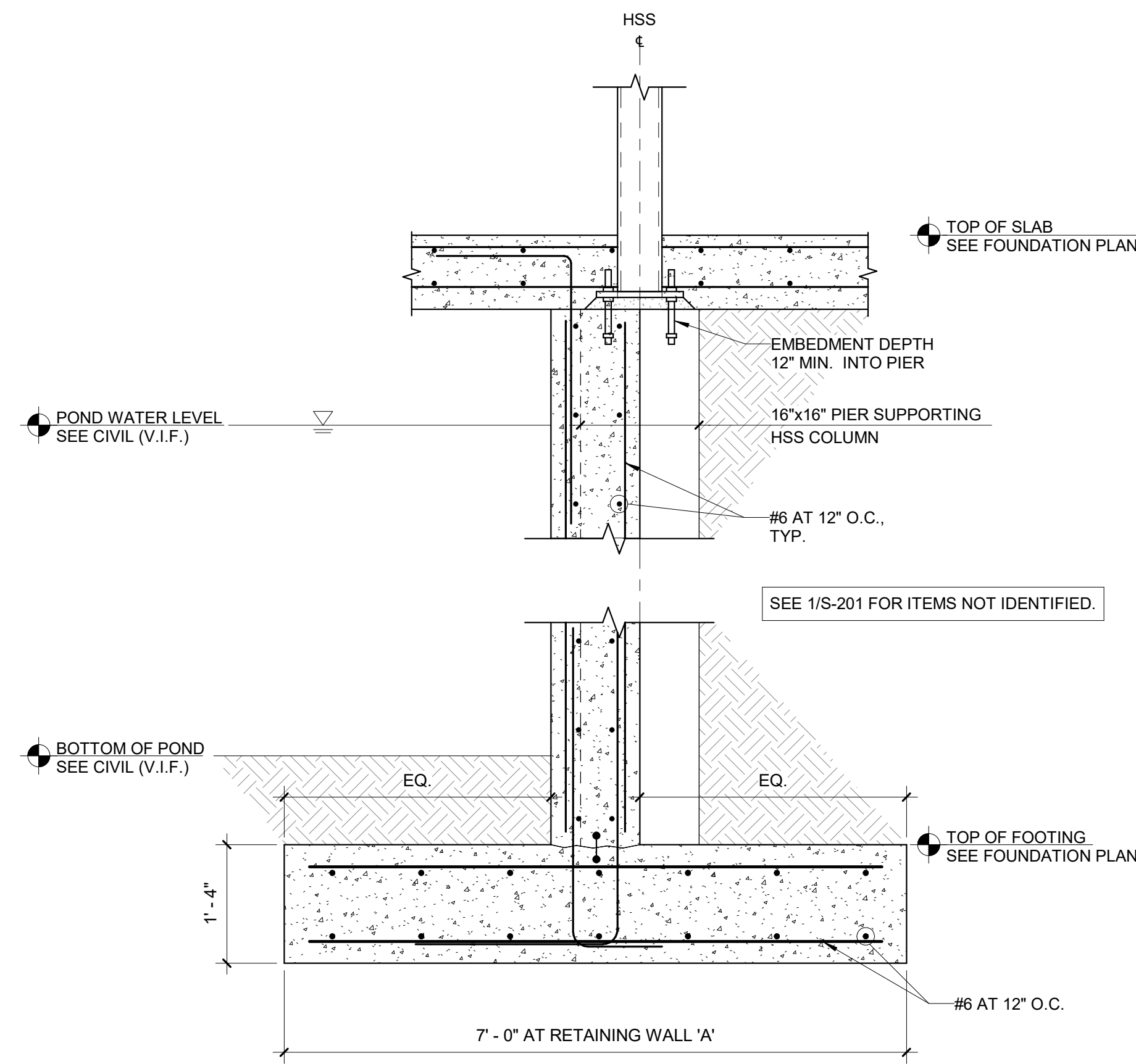
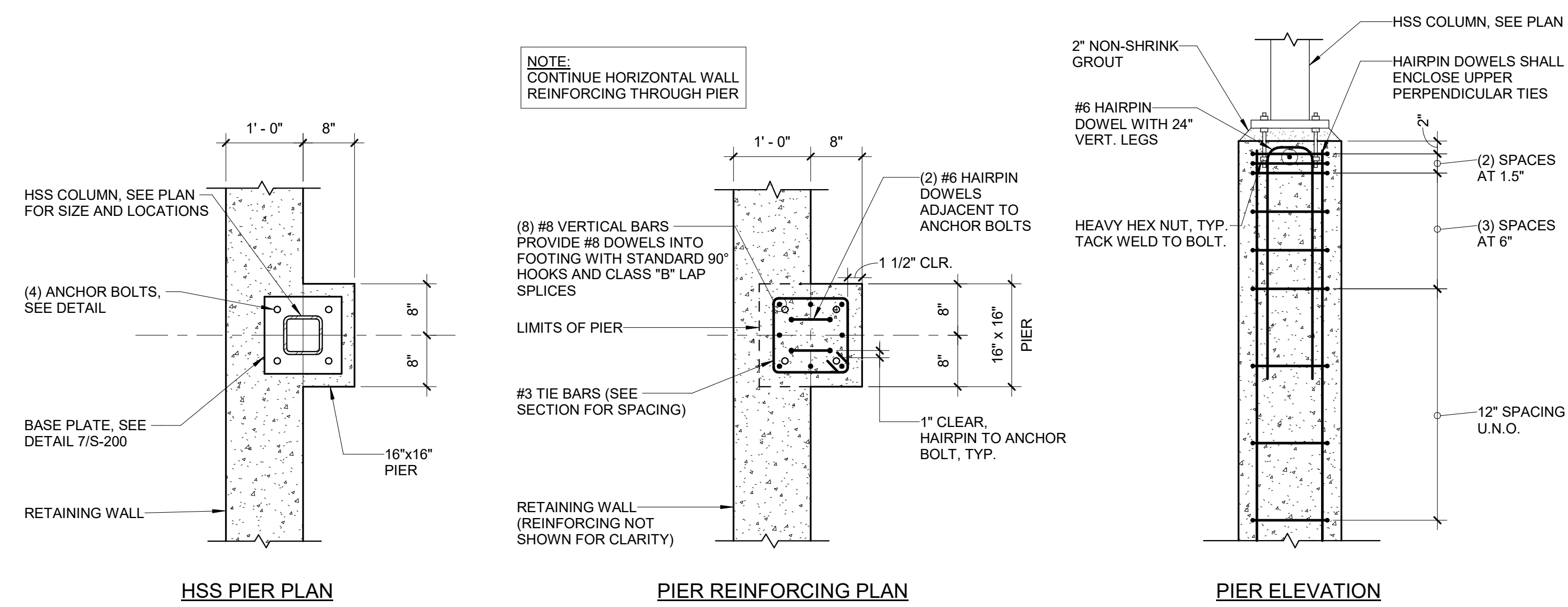
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DETAILS

PROJECT NO. 50188638

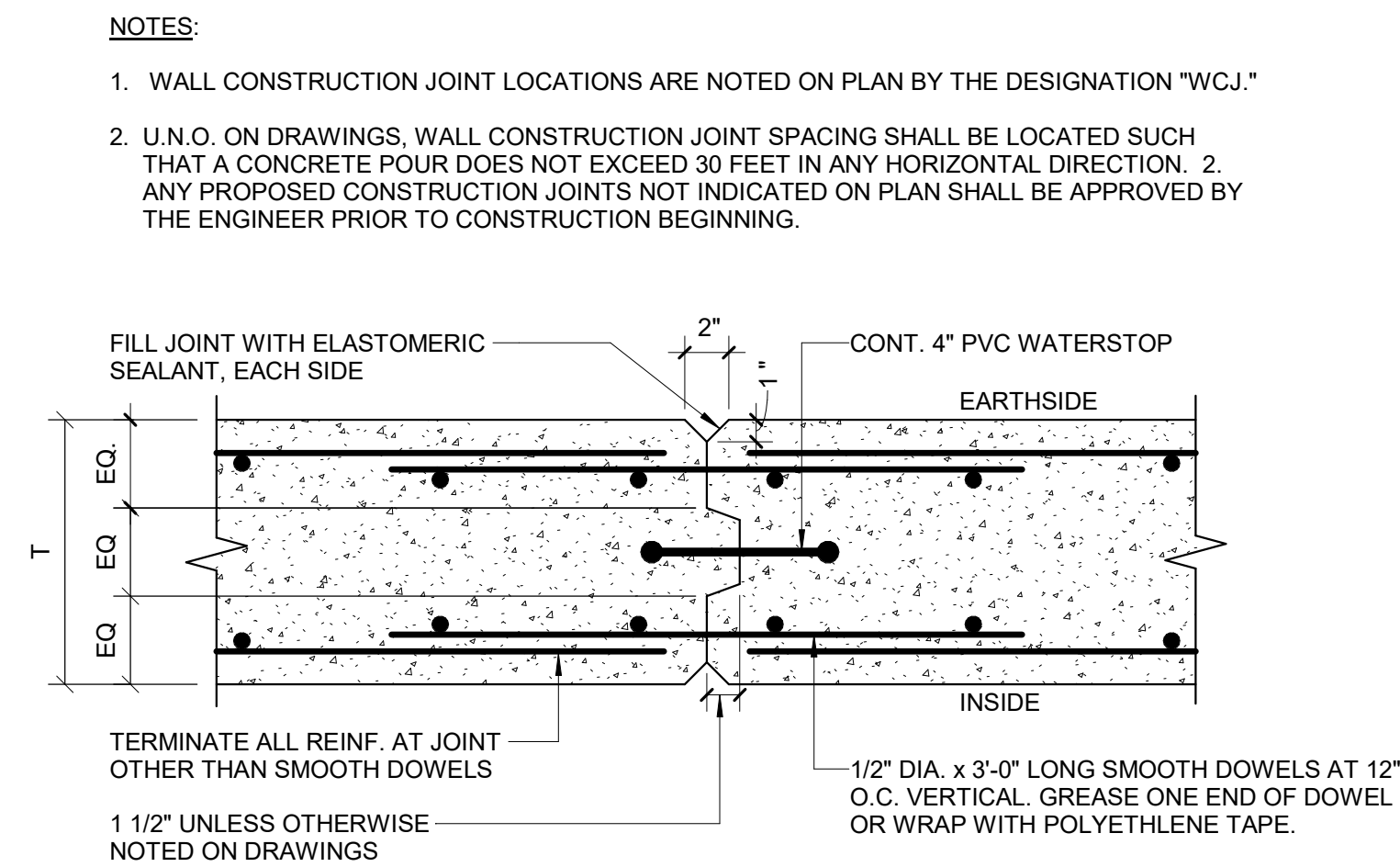
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SHEET NO.



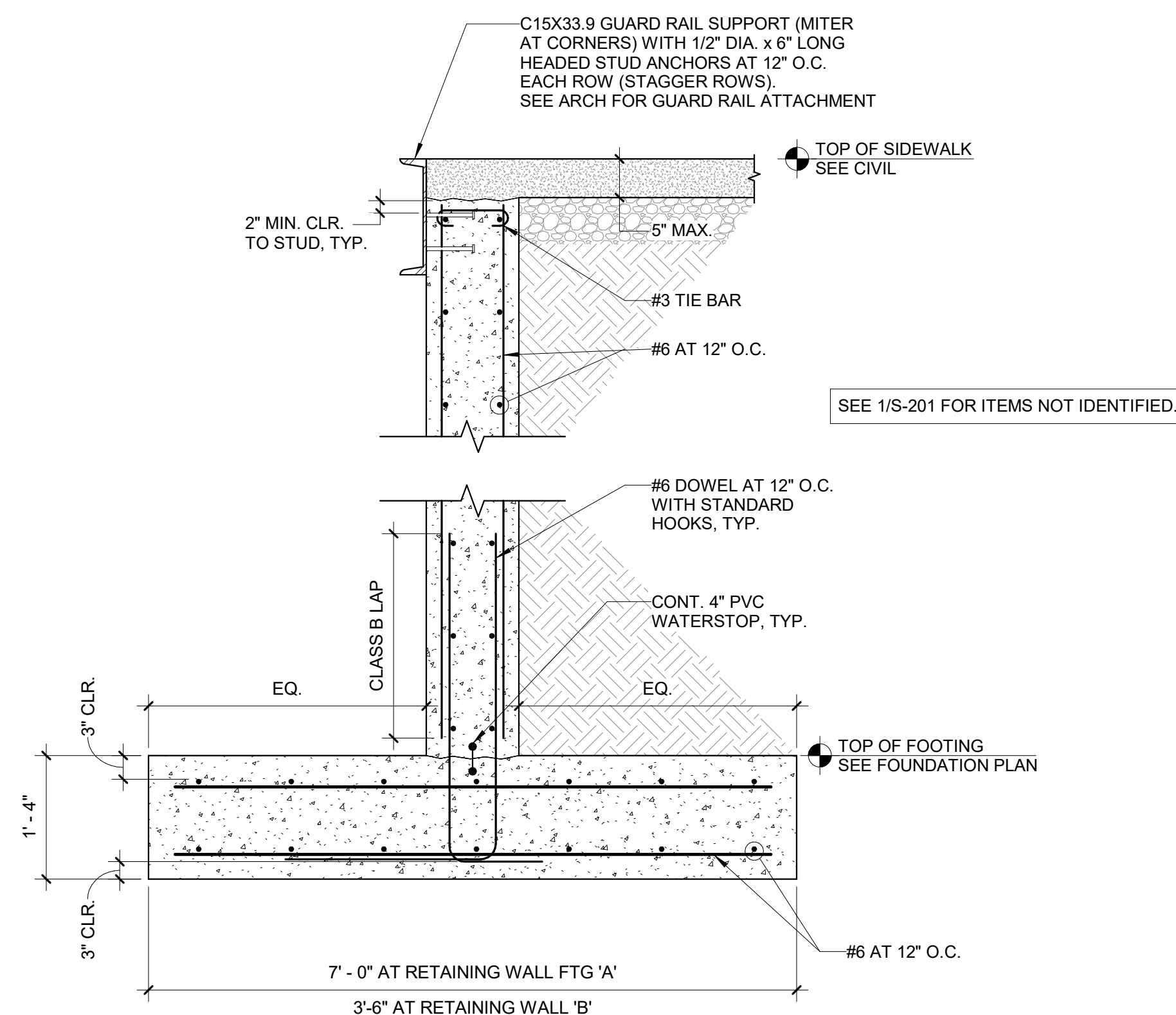
2 RETAINING WALL DETAIL AT COLUMN

Scale: 3/4" = 1'-0"



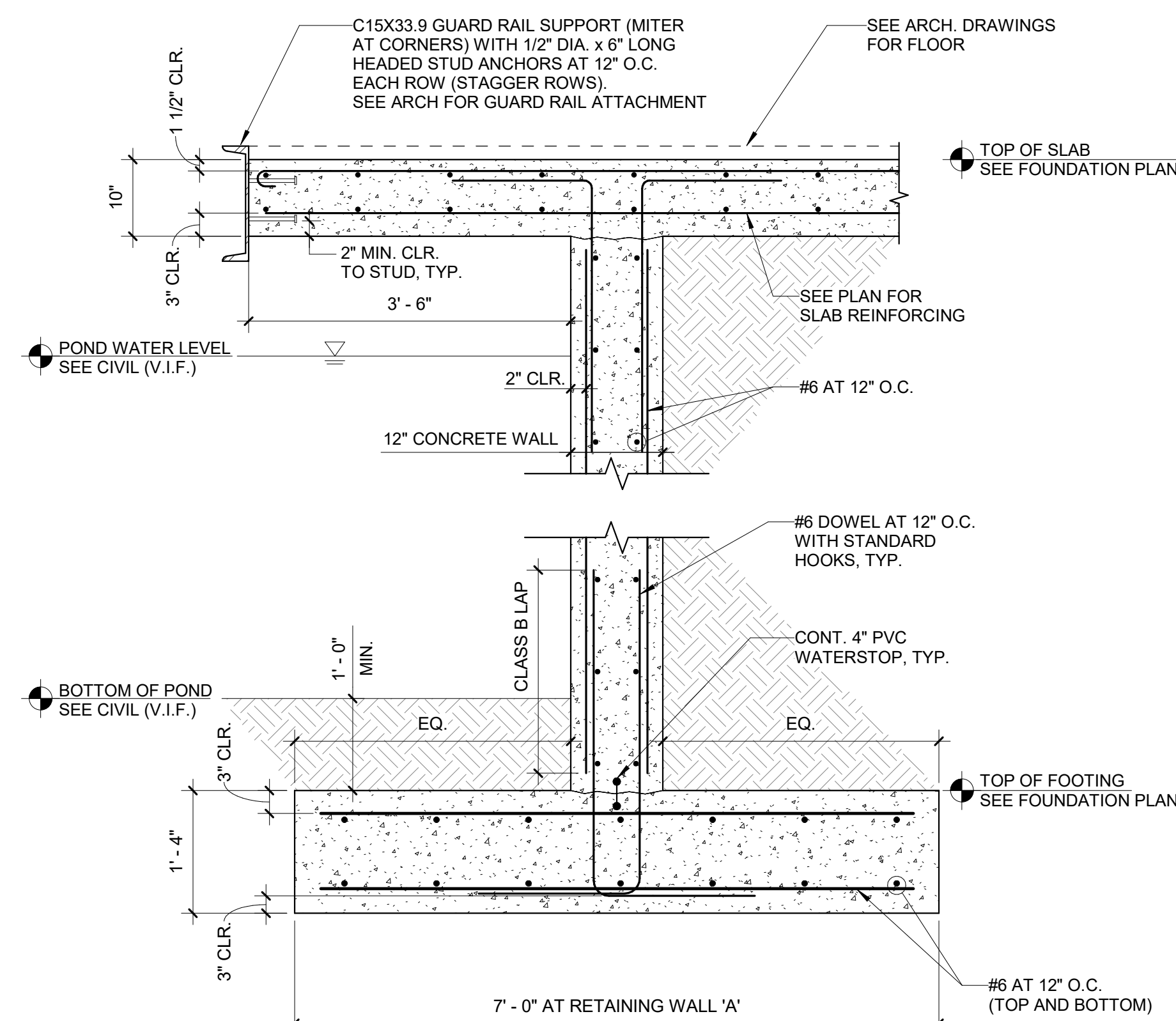
4 WALL CONSTRUCTION JOINT

Scale: 3" = 1'-0"



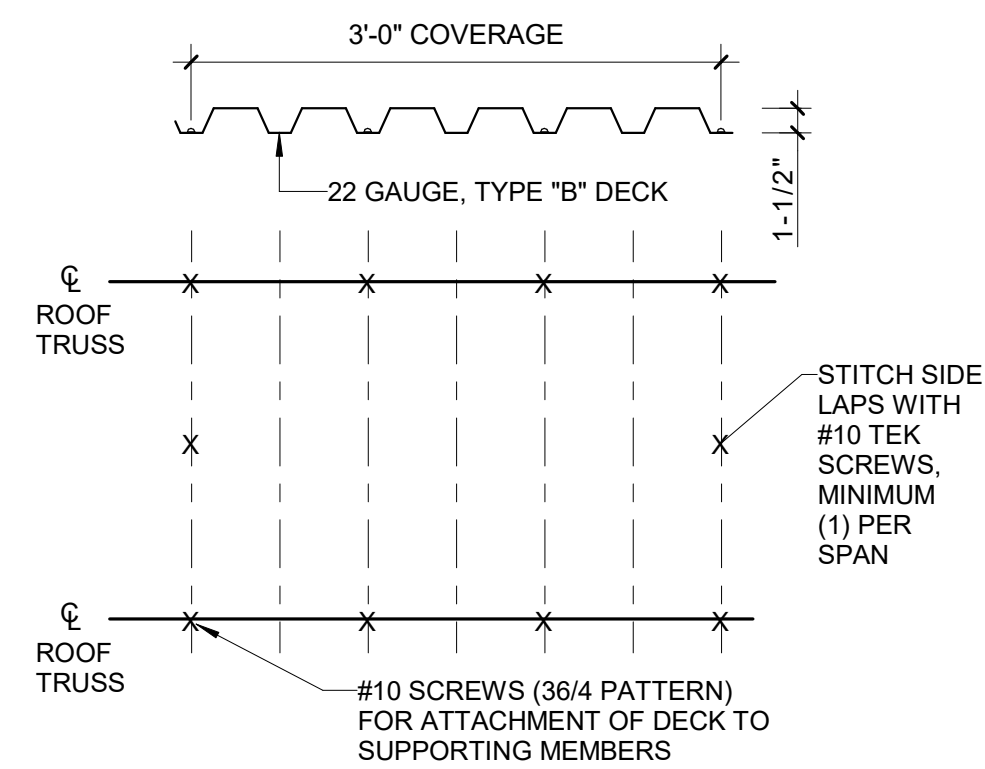
3 RETAINING WALL DETAIL AT SIDEWALK

Scale: 3/4" = 1'-0"

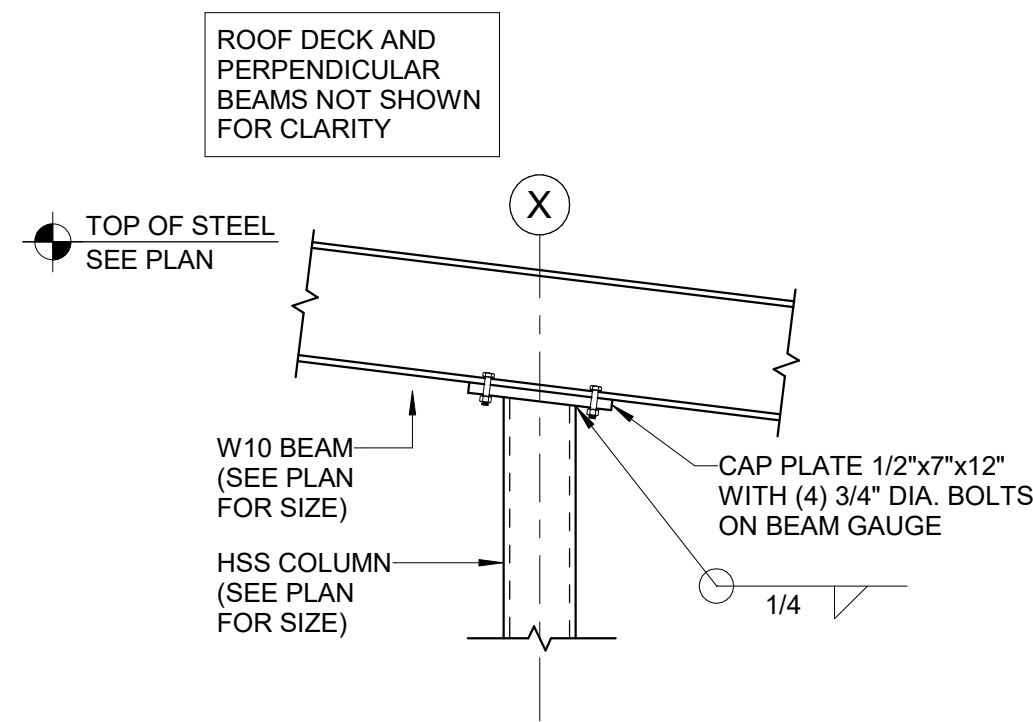


1 RETAINING WALL DETAIL

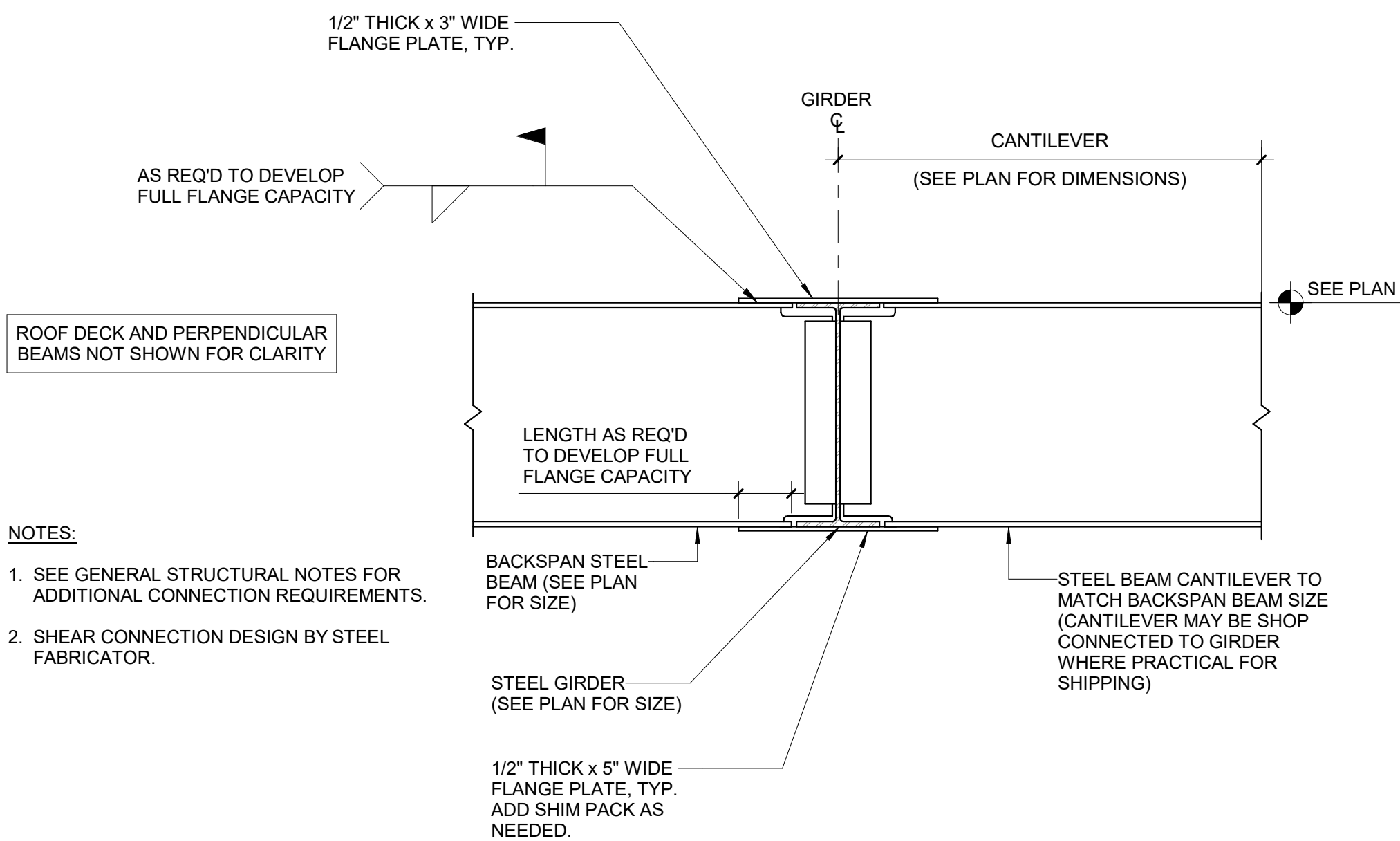
Scale: 3/4" = 1'-0"



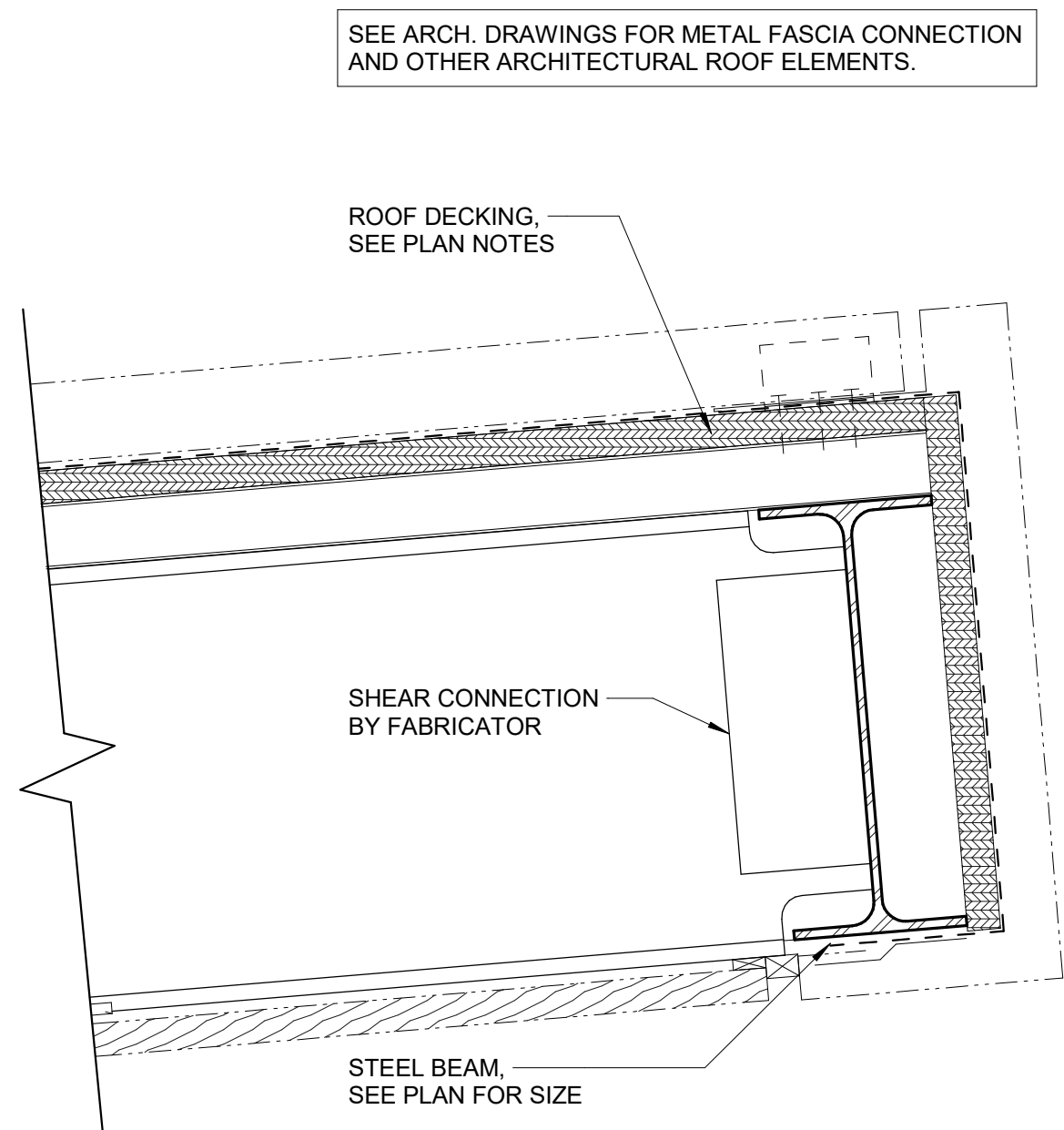
3 ROOF DECK FASTENER DETAIL
Scale: 1" = 1'-0"



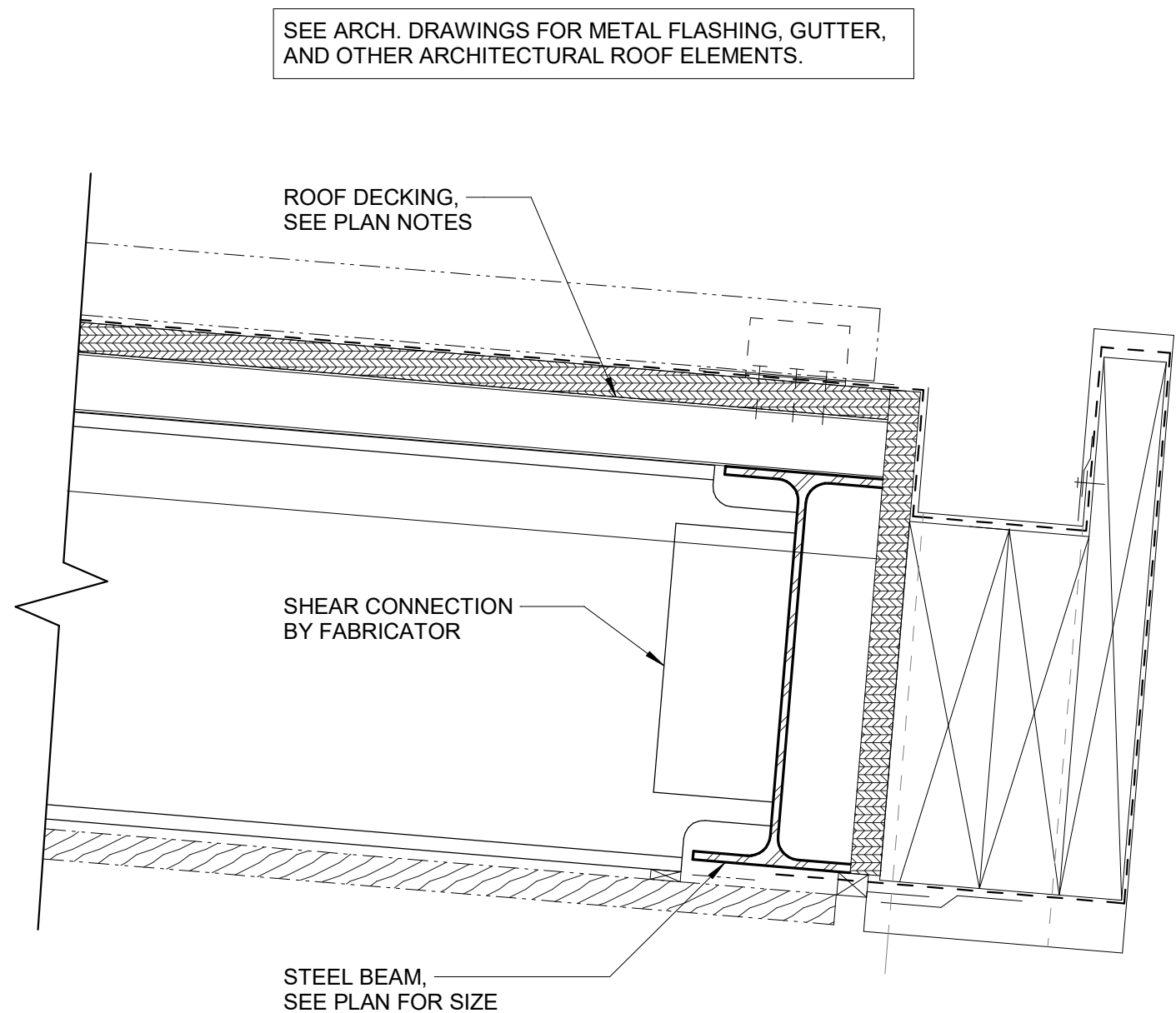
2 HSS COLUMN TO WF BEAM CONNECTION
Scale: 3/4" = 1'-0"



1 BEAM-BEAM MOMENT CONNECTION
Scale: 3/4" = 1'-0"



5 ROOF SECTION AT HIGH END
Scale: 3" = 1'-0"



4 ROOF SECTION AT LOW END
Scale: 3" = 1'-0"

SEAL



KEY PLAN

SCALE

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FRAMING DETAILS

PROJECT NO. 50188638

S-202

SHEET NO.

