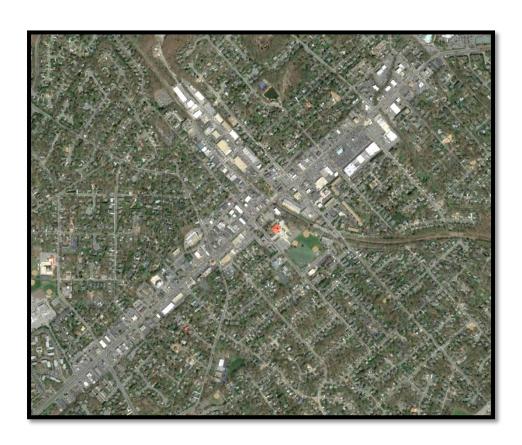


WATER SYSTEM STUDY: MAPLE AVENUE CORRIDOR

TOWN OF VIENNA, Virginia

FINAL REPORT



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NOVEMBER 2015



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I. INTRODUCTION

Whitman, Requardt, and Associates (WRA) has conducted a water capacity study for the Town of Vienna to analyze the present and future water demands of the Maple Avenue (Route 123) commercial district. The intent of this study is to evaluate the Town's current water distribution system and the ability to supply adequate fire flow during peak demand using a 24 hour extended simulation analysis. Based on data obtained from the current Town of Vienna water model, in this study WRA has made recommendations for infrastructure improvements to meet the present and future demands of the Maple Avenue commercial district.

II. BACKGROUND:

For the analysis, the Town of Vienna and Fairfax Water provided the following information:

- WaterGEMS V8i Water Model
- GIS Shapefiles
- Town of Vienna Water System Maps
- Current Pressure Reducer Valve (PRV) Pressure Settings and Elevations
- Current Tank Elevations
- Town of Vienna Historic Water Wholesale Purchase Data
- Town of Vienna Billing Records
- Town of Vienna Water Consumption Records
- Fire Flow Testing Data
- Pressure Logger Data

The original water model was developed by WRA in 2006 and updated in 2009. The model included two elevated tanks, one ground storage tank and one pump station. This model was calibrated for steady state and extended period simulations (EPS). This model was also used to estimate the water age in the system.

Demand and Population Projection Assumptions:

WRA reviewed all pertinent information including individual billing records but found it difficult to allocate demands due to the large volume of meter records compared to the simplicity of the model. Additionally, no population forecasting data was available for analysis.

Due to limited data to simulate future demands for the Maple Avenue commercial district, WRA and the Town of Vienna agreed that it would be most efficient to place a 30% increase on all present demands within the Maple Avenue commercial district. It was concluded that the demands in the current water model were sufficient since the corridor has not experienced significant development since the model was created.

Fire Flow Analysis Assumptions:

After creating present and future water demand scenarios, WRA evaluated the system capacity by selecting five locations (See Appendix A) along the Maple Avenue corridor and placing a three hour fire flow demand of 2700 GPM to simulate a fire flow during peak usage. When modeling fire flow demands for commercial developments, a range of 2500-3000 GPM is typically used by convention. After each simulated test, WRA evaluated the lowest pressures in the distribution system and noted the locations of the low pressures (See Appendix B and C).

III. MODELING & ANALYSIS:

To initialize the model, all of the pressure reducing valves were updated to the current settings supplied by The Town of Vienna; Table 1 shows the pressure settings:

Table 1: Town of Vienna PRV Settings					
PRV Name	Status	Size	Pressure		
Earl	OFF	6"	N/A		
Maple Avenue	ON	12"	68 PSI ¹		
Old Courthouse Road	ON	12"	60 PSI ¹		
Montmorency	ON	6"	54 PSI ²		
Vale	ON	6"	55 PSI		
Electric Avenue	ON	10"	45 PSI		
Park	ON	8"	52 PSI		

Notes:

- Pressure settings for the Maple Avenue and Old Courthouse Road PRVs could not be verified and are from the 2009 update of the model.
- Montmorency pressure setting is assumed to be 10 PSI below normal operating pressure.

Validation of the Model:

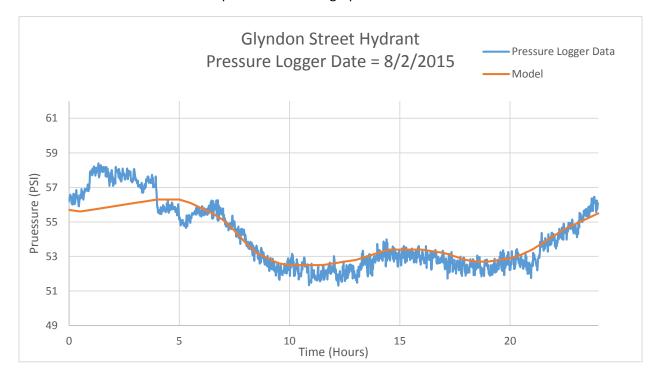
The accuracy of the updated base model (max day) was validated by using field fire flow tests and fire hydrant pressure logger data supplied by the Town of Vienna. WRA selected three fire flow tests which used at least two hydrants: a flow hydrant and residual hydrant for an accurate comparison.

The results of the fire flow tests when compared to 24 hour simulation in the model are as follows:

Fire Flow Test A:	Flow Hydrant ID = 38-3-20A		Residual Hydrant = 38-3-20			
West St. NW (5/11/2015)	Static PSI	Flow (GPM)	Static PSI	Residual PSI		
Field	N/A	1061	51	41		
Model	52	1061	50	47		
Fire Flow Test B:	Flow Hydrar	nt ID = 38-3-54	Residual Hydrant ID = 38-3-98			
Emmanual Church (2/12/2015)	Static PSI	Flow (GPM)	Static PSI	Residual PSI		
Field	63	1007	63	45		
Model	57	1007	57	42		
Fire Flow Test C:	Flow Hydrant ID = 38-3-43		Flow Hydrant ID = 38-3-51		Residual Hydrant ID = 38-3-50	
Moseby's Landing (1/5/2015)	Static PSI	Flow (GPM)	Static PSI	Flow (GPM)	Static PSI	Residual PSI
Field	N/A	775	N/A	746	46	33
Model	41	775	41	746	41	29

Based on the results of the fire flow tests on existing fire hydrants, WRA observed that the pressures of residual hydrants in the field and model were within an acceptable range of 5-6 PSI.

WRA additionally used pressure logger data from a recently installed hydrant located on Glyndon Street between Locust Street and Cabin Road and compared the results to a 24 hour maximum day model simulation. The results of the comparison are in the graph below:



The graph demonstrates that the pressures recorded in the field closely match the pressures observed in the model. Given the data observed when comparing the fire flow tests and the pressure logger data to the results of the model, WRA is confident that the model will predict system pressures and capacity within acceptable limits.

Fire Flow Simulations:

After the model was updated and validated, five independent simulations were performed at five intersections along the Maple Avenue corridor; for each 24 hour simulation, a 3 hour 2700 GPM fire flow demand was placed at each intersection with the fire flow starting at 8 AM. The lowest pressure in the system was recorded for each 24 hour simulation as shown in Table 2.

After simulating the fire flow at present max daily demand, all demands within the Maple Avenue commercial district were increased by 30%. Five additional fire flows were then simulated with the same 3 hour 2700 GPM fire flow demands at the same five intersections along Maple Avenue (See Table 3).

IV. RESULTS:

Table 2: Present Max Daily Demand at Maple Avenue						
Fire Flow Simulation No.	3 HR Fire Flow @ Peak Demand (8 AM)	Location of Demand	Min. Pressure in System (PSI)	Critical? 1		
1	2700 GPM	Maple Ave/Nutley St.	27.1	No		
2	2700 GPM	Maple Ave/Pleasant St.	27.0	No		
3	2700 GPM	Maple Ave/ Center St.	26.7	No		
4	2700 GPM	Maple Ave/ Glyndon St.	26.3	No		
5	2700 GPM	Maple Ave/E St.	15.5	YES		

Table 3: 130% Max Daily Demand at Maple Avenue						
Fire Flow Simulation No.	3 HR Fire Flow @ Peak Demand (8 AM)	Location of Demand	Min. Pressure in System (PSI)	Critical? 1		
1	2700 GPM	Maple Ave/ Nutley St.	27.0	No		
2	2700 GPM	Maple Ave/Pleasant St.	26.9	No		
3	2700 GPM	Maple Ave/ Center St.	26.6	No		
4	2700 GPM	Maple Ave/ Glyndon St.	26.2	No		
5	2700 GPM	Maple Ave/E St.	15.2	YES		
5A ²	2700 GPM	Maple Ave/E St.	25.4	No		

Notes:

- 1. Critical pressure is considered below 20 PSI.
- 2. Model Simulation 5A is an alternative scenario where Hines Street/Follin Avenue 8-inch and 12-inch water mains were connected to improve system capacity.

Analysis of Results:

In Table 2, the minimum system pressures Fire Flow Simulations 1 through 4 ranged from 26.3 to 27.1 PSI which were all above the critical pressure of 20 PSI. Fire Flow Simulation 5 had a below critical pressure of 15.5 PSI. Referring to Appendix B, the area where the critical pressures occurred is east of Maple Avenue around Wolftrap Road and Kramer Drive.

In Table 3, when demands were increased by 30% within the Maple Avenue commercial district, the minimum pressure for Fire Flow Simulations 1 through 4 had a range of 26.2 to 27.0 PSI. As in the first scenario, all pressures were above critical pressure. Fire Flow Simulation 5 had a below critical pressure of 15.2 PSI. Referring to Appendix C, the area where the critical pressures occurred is in the same area, east of Maple Avenue around Wolftrap Road and Kramer Drive.

The most likely cause of the critical pressures in Fire Flow Simulation 5 for both max day scenarios is that the area east of Maple Avenue is at higher elevation; when demand is increased to 2700 GPM during a 3 hour fire flow the pressure cannot be sustained above 20 PSI due to limits of the system capacity.

Other Observations:

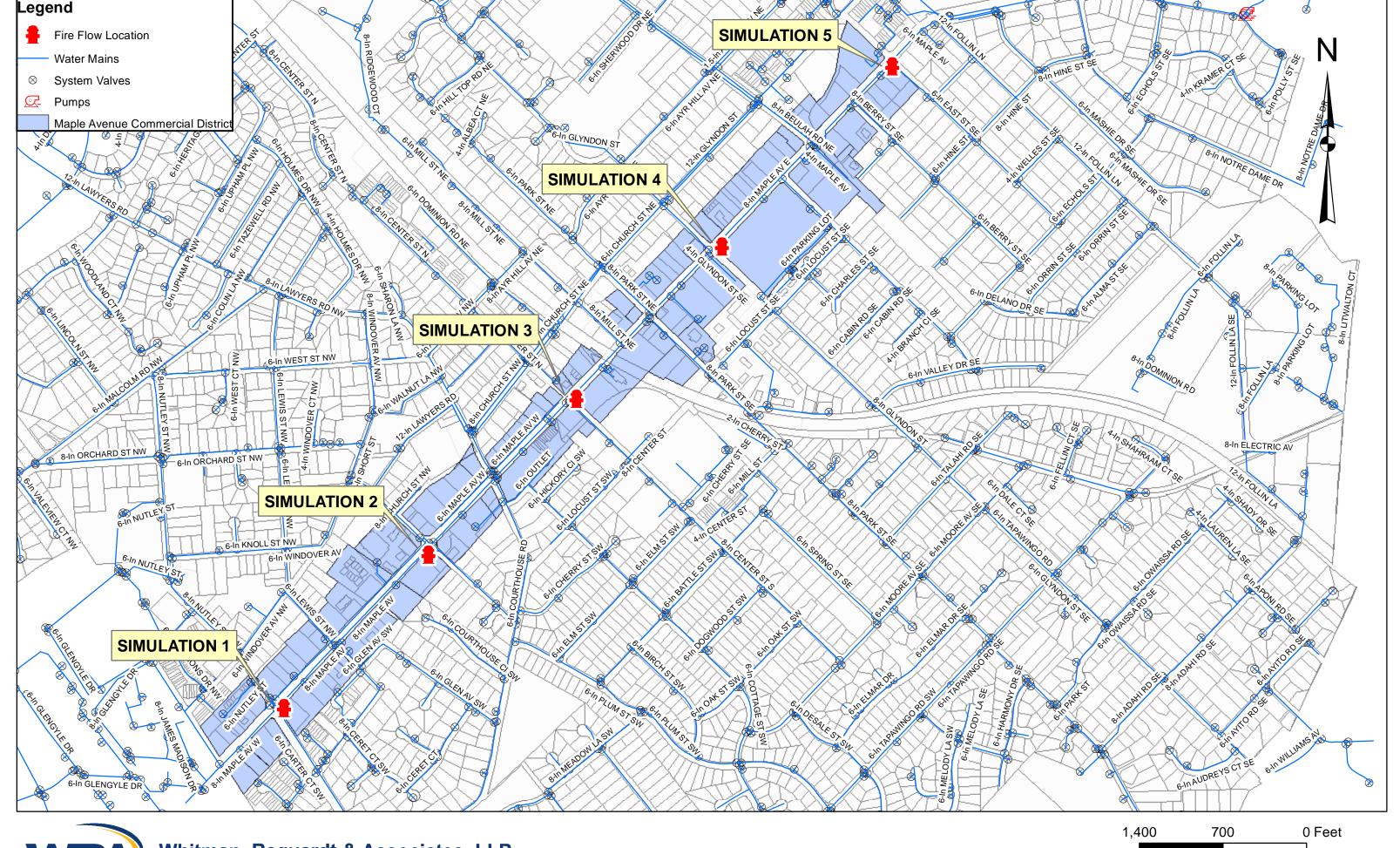
• When the model was run for all fire flow simulations, it was observed that domestic demand had little effect on the results.

V. <u>RECOMMENDATIONS:</u>

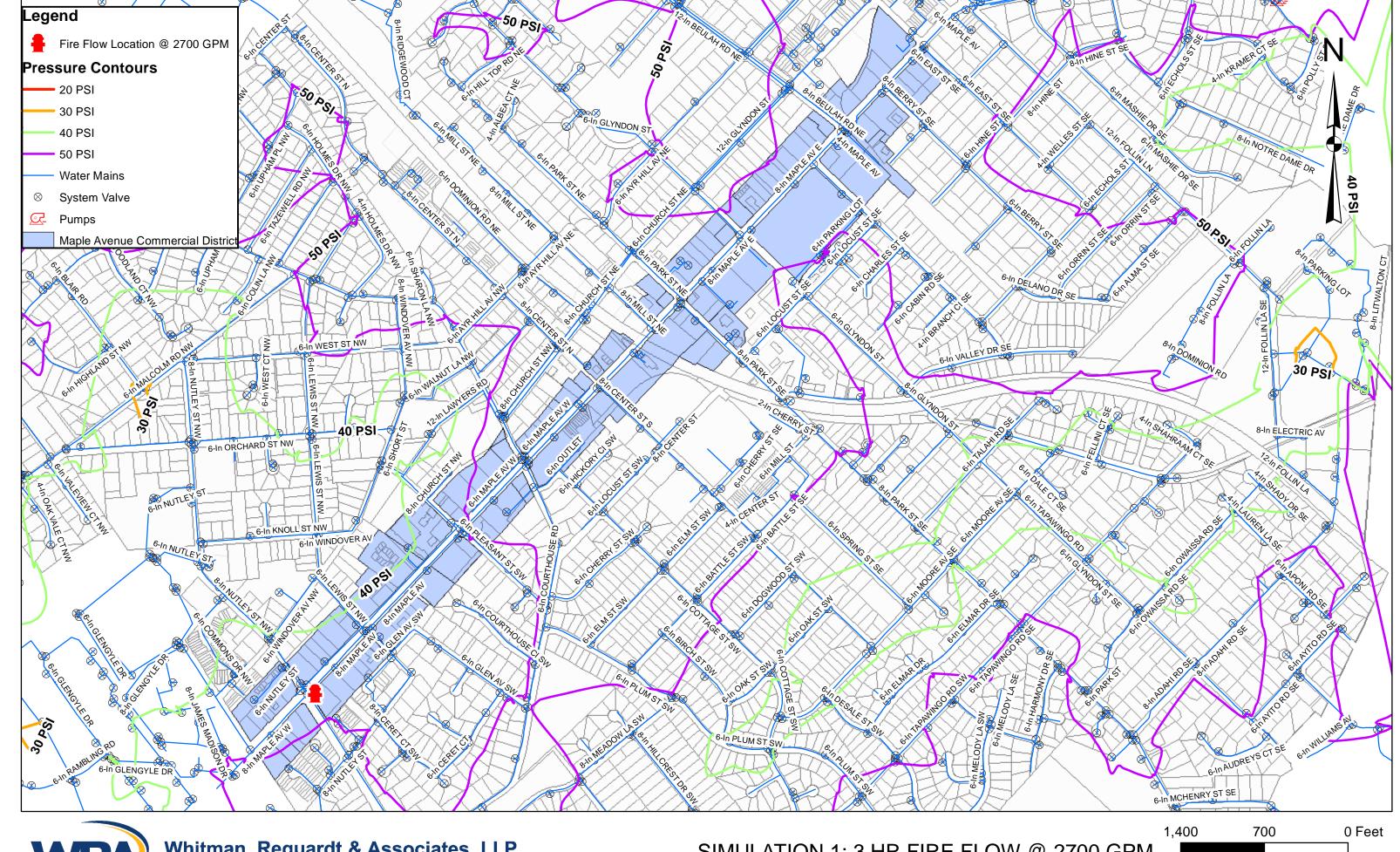
To alleviate critical pressures observed during Fire Flow Simulation 5 for both max day scenarios, WRA recommends connecting the 8-inch water main in Hines Street to the 12-inch water main in Follin Lane. According to water system maps, these water mains are currently not connected.

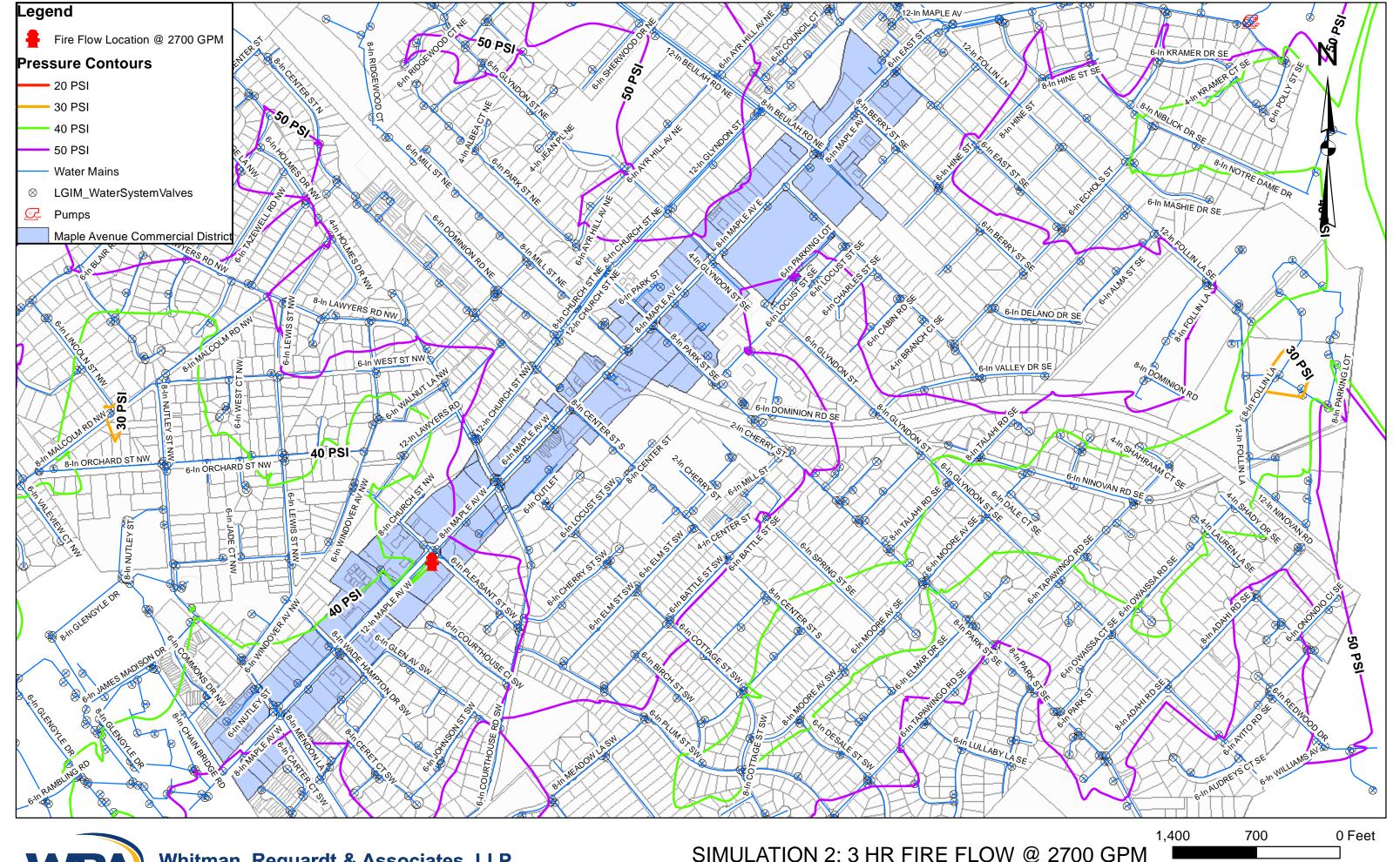
Referring to Table 3, Fire Flow Simulation 5A, when the two pipes were connected in the model, the lowest pressure in the system was raised from 15.2 PSI to 25.4 PSI. By connecting the two pipes, the 12-inch pipe would provide the extra capacity required when the uppermost part of the Maple Avenue is under fire flow demand.

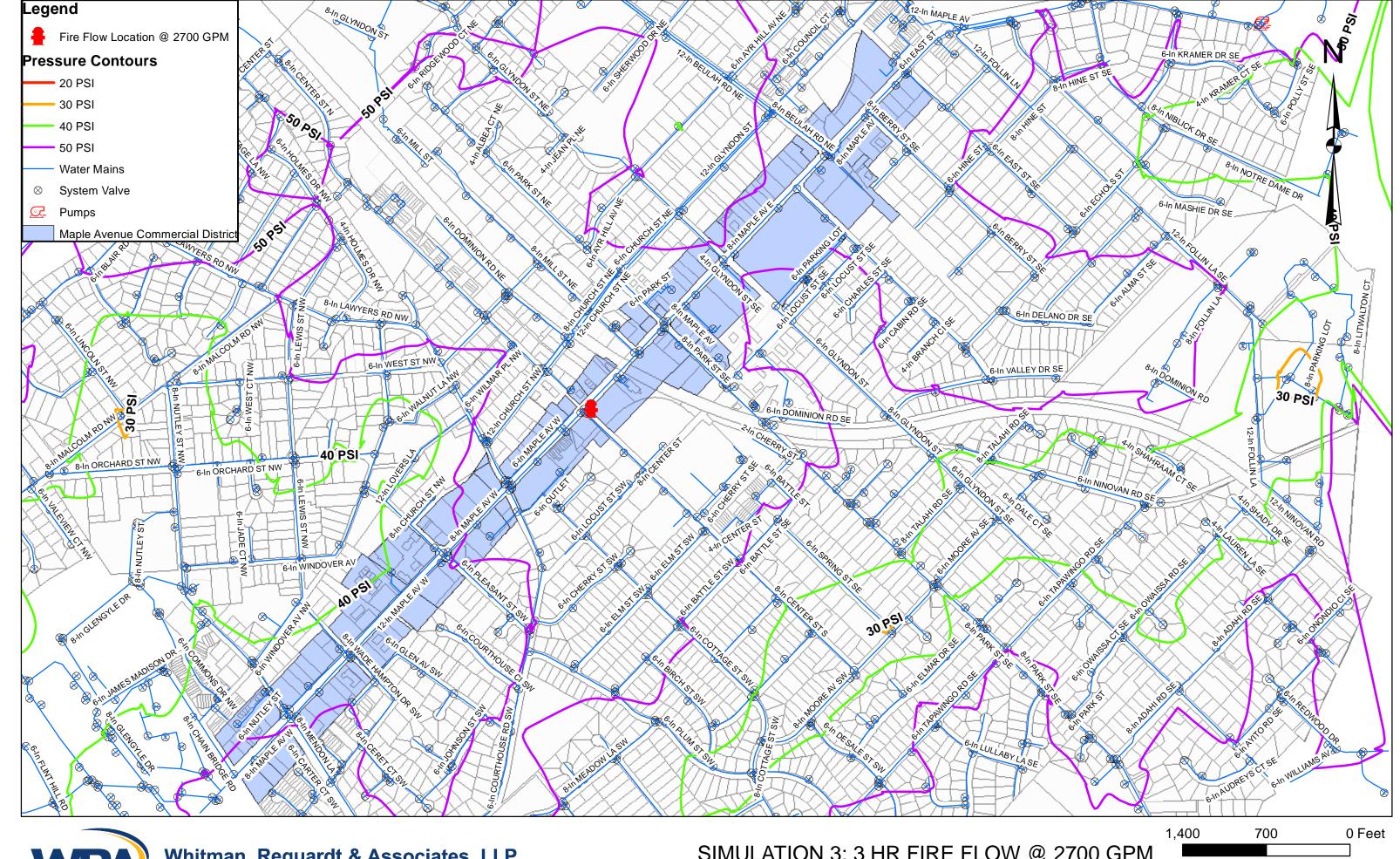
APPENDIX A MAPLE AVENUE HYDRANT SIMULATION LOCATIONS

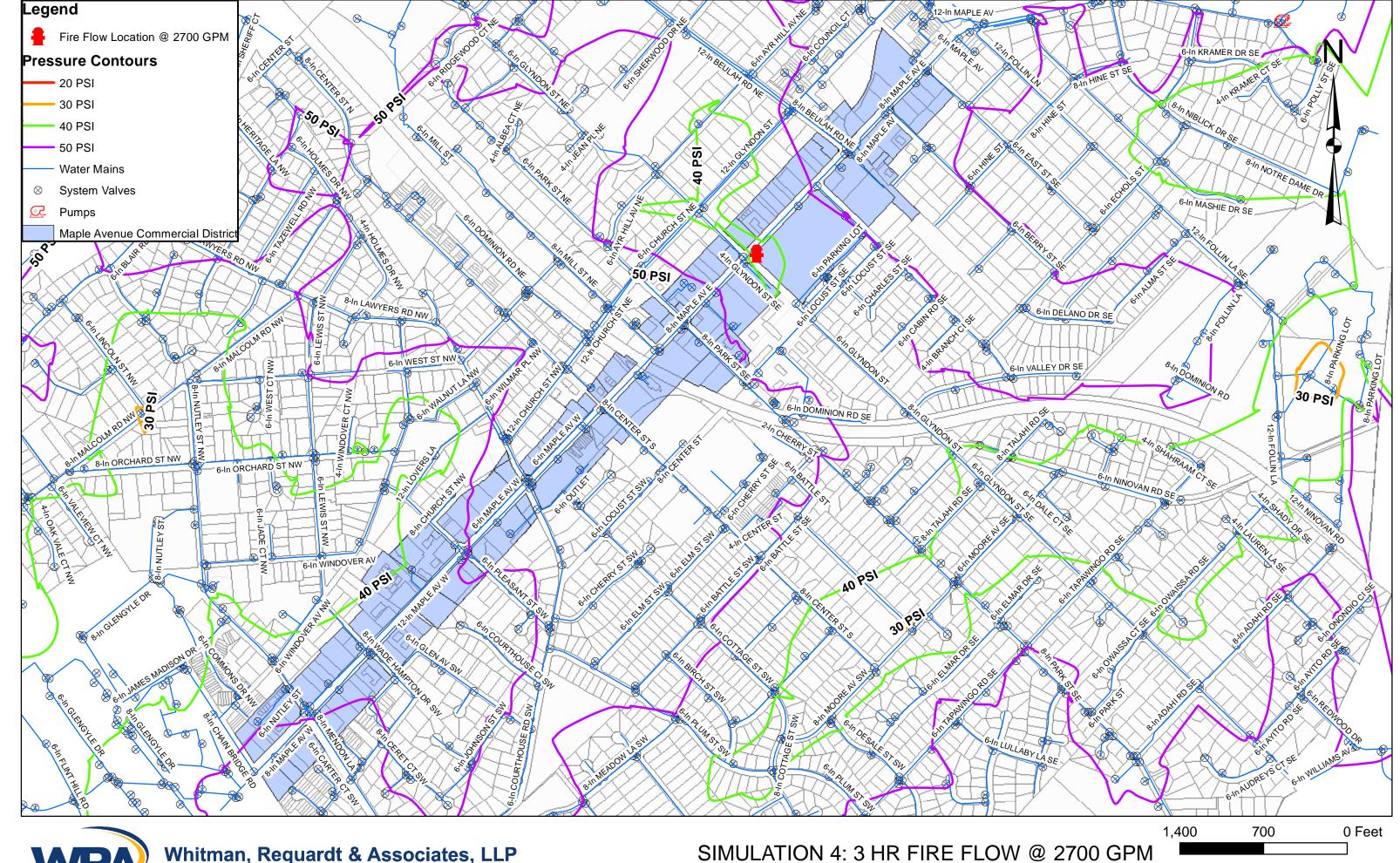


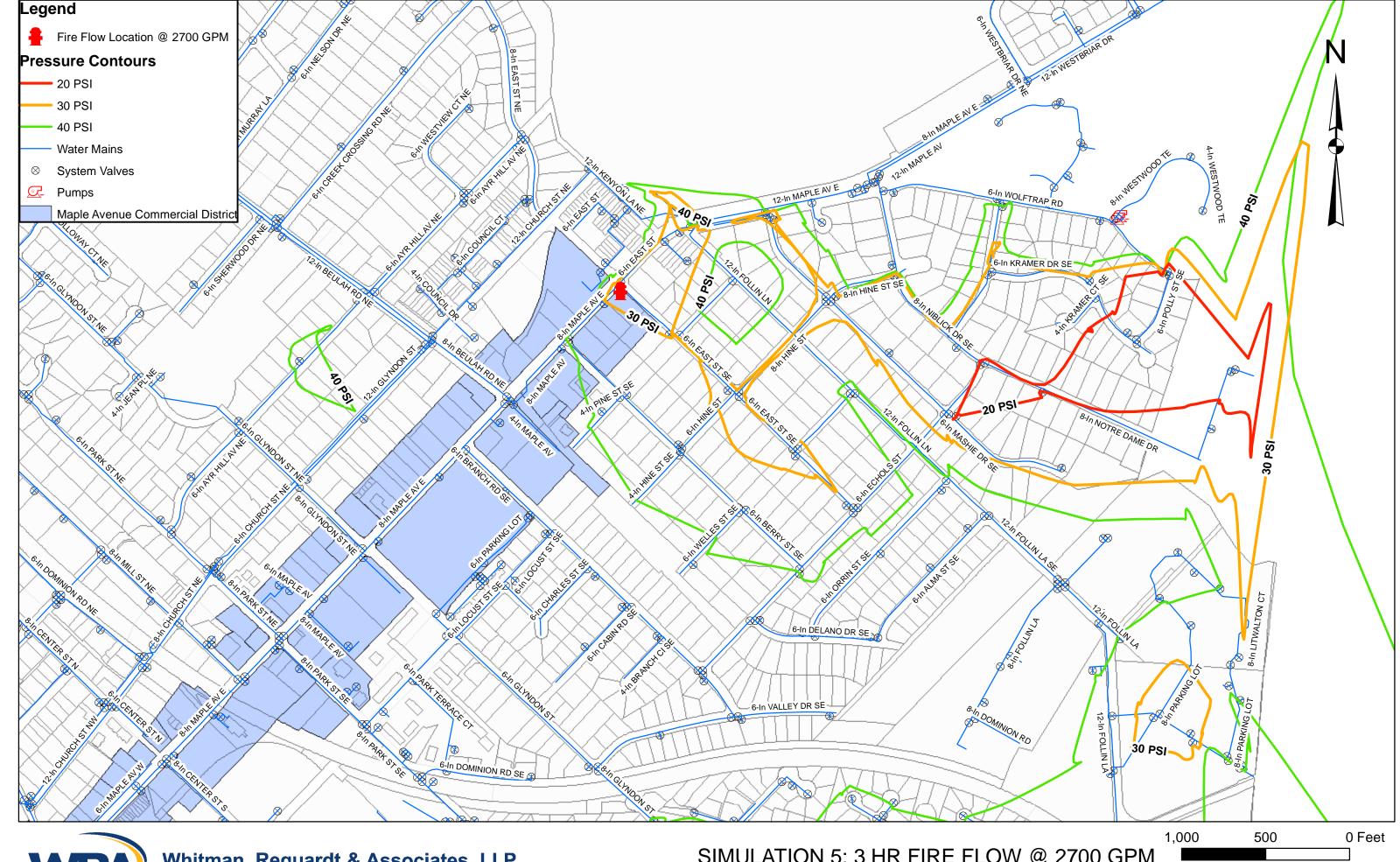
APPENDIX B PRESENT MAX DAILY DEMAND PRESSURE CONTOURS HYDRANT SIMULATIONS 1-5



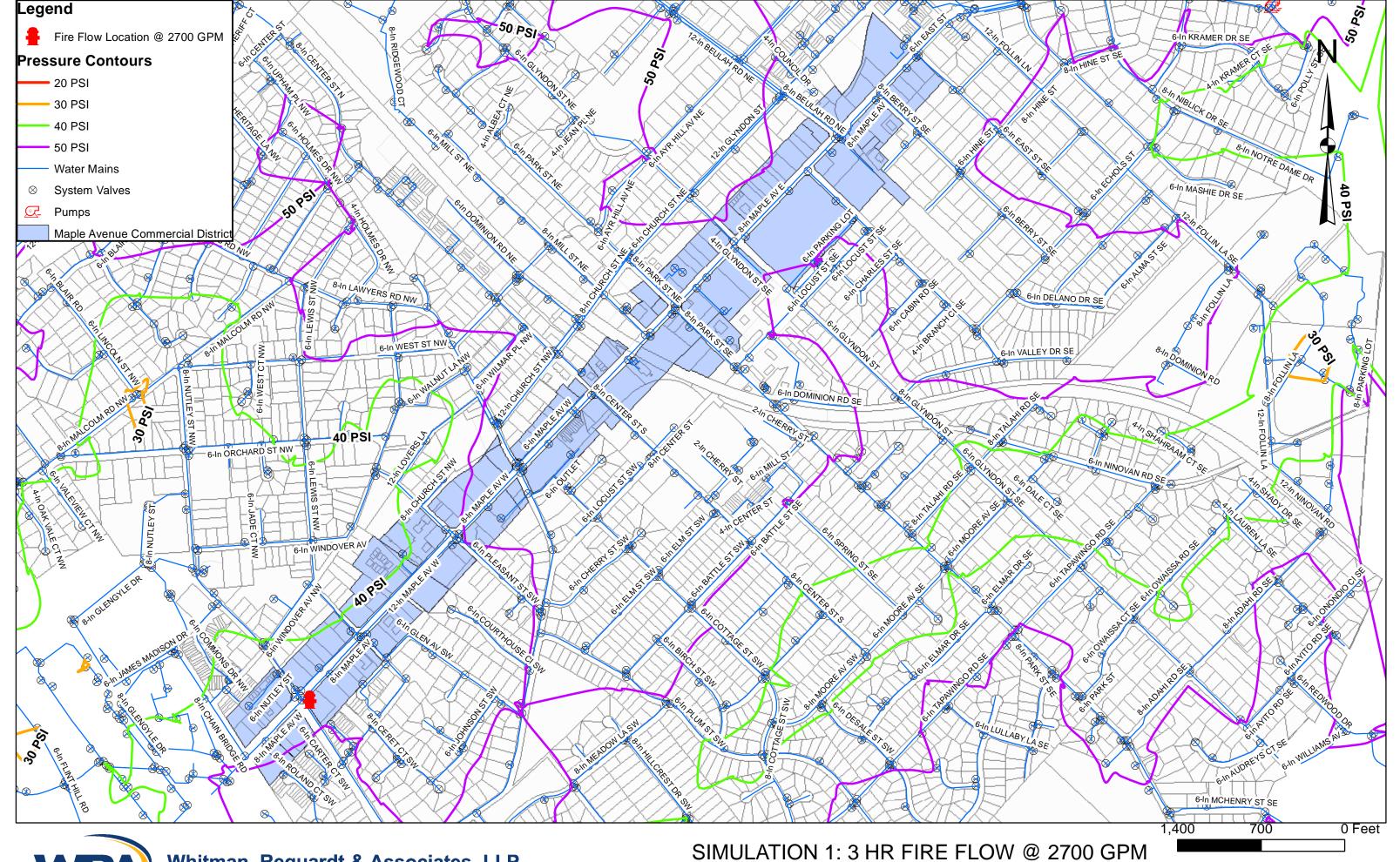


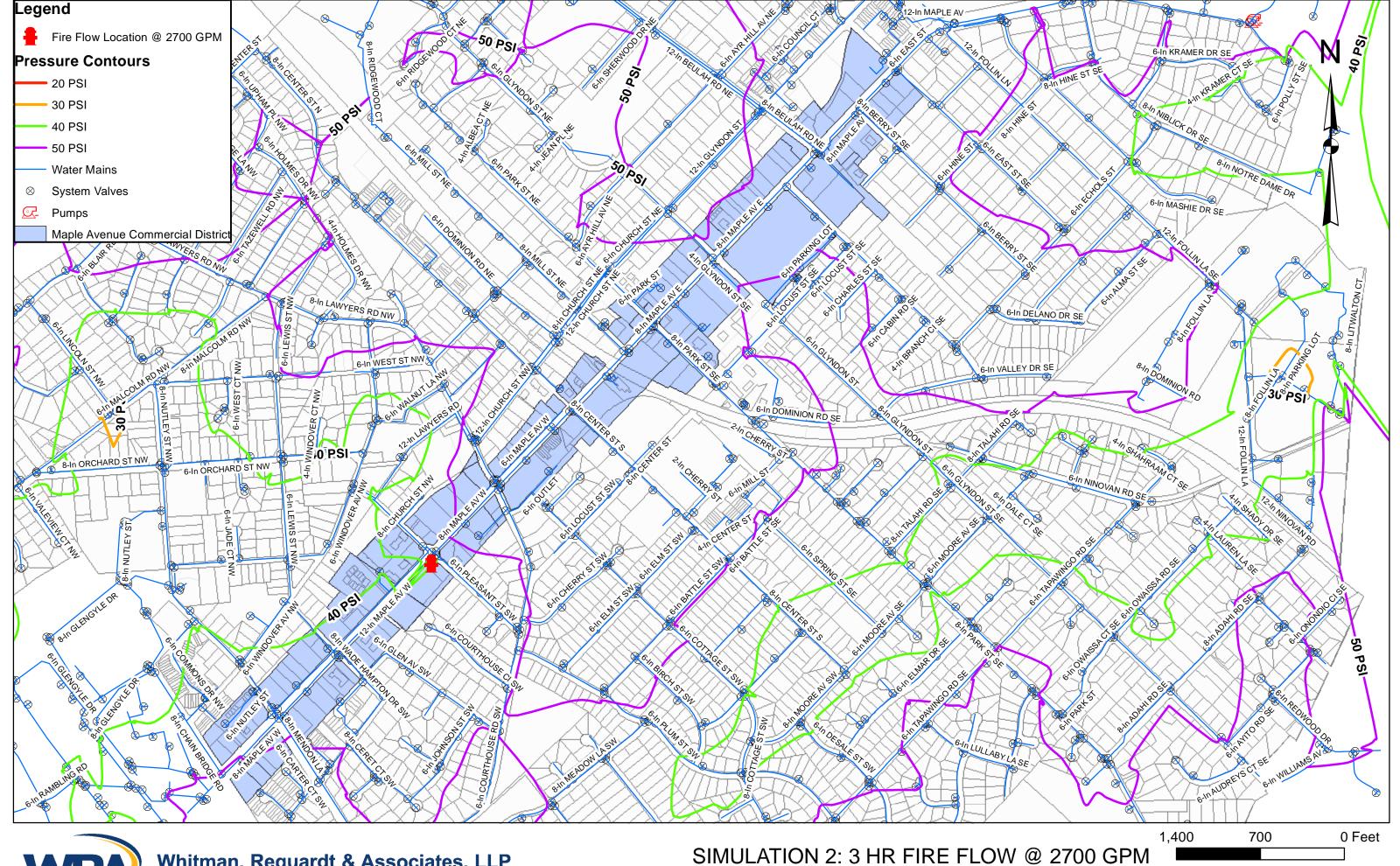


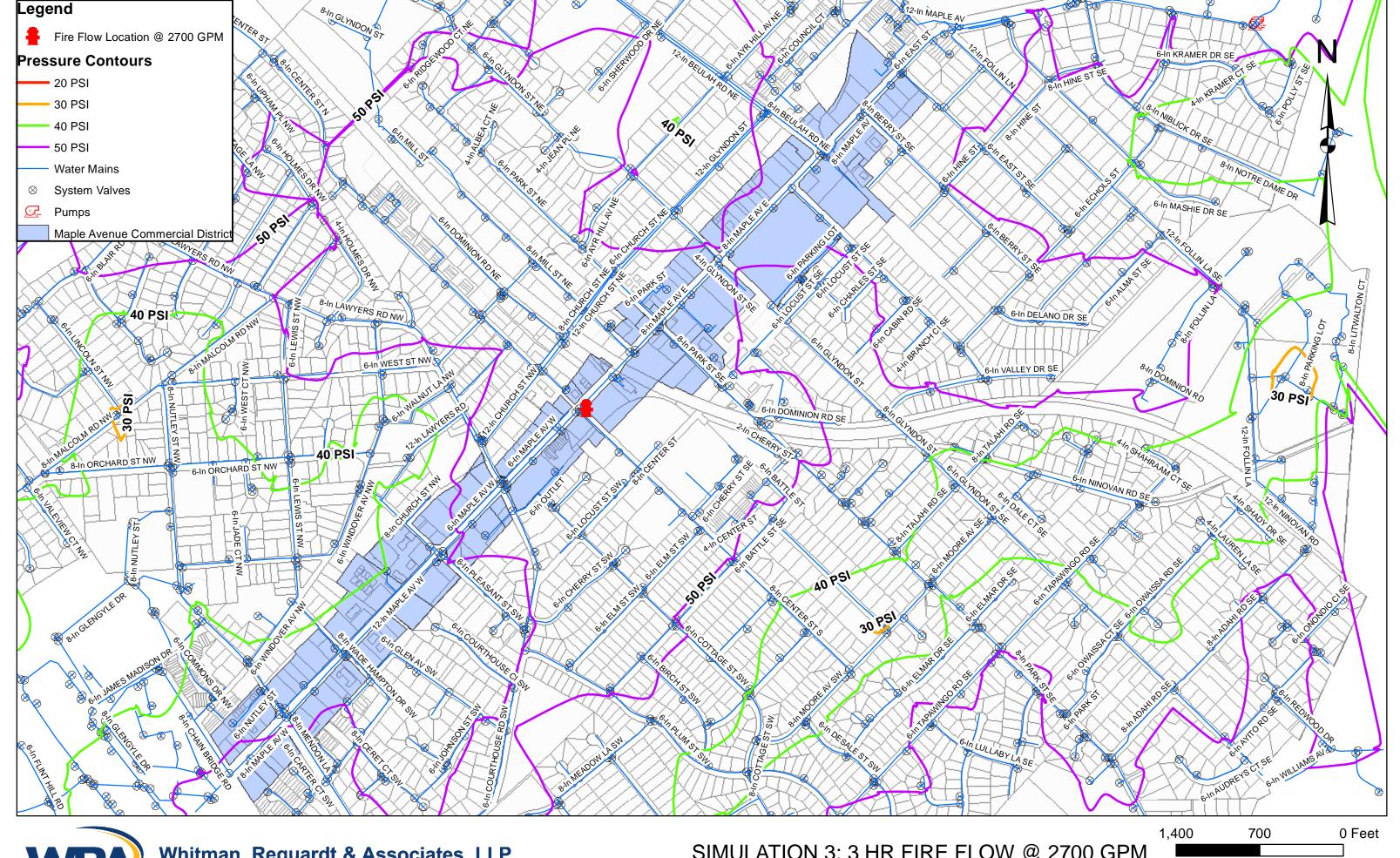


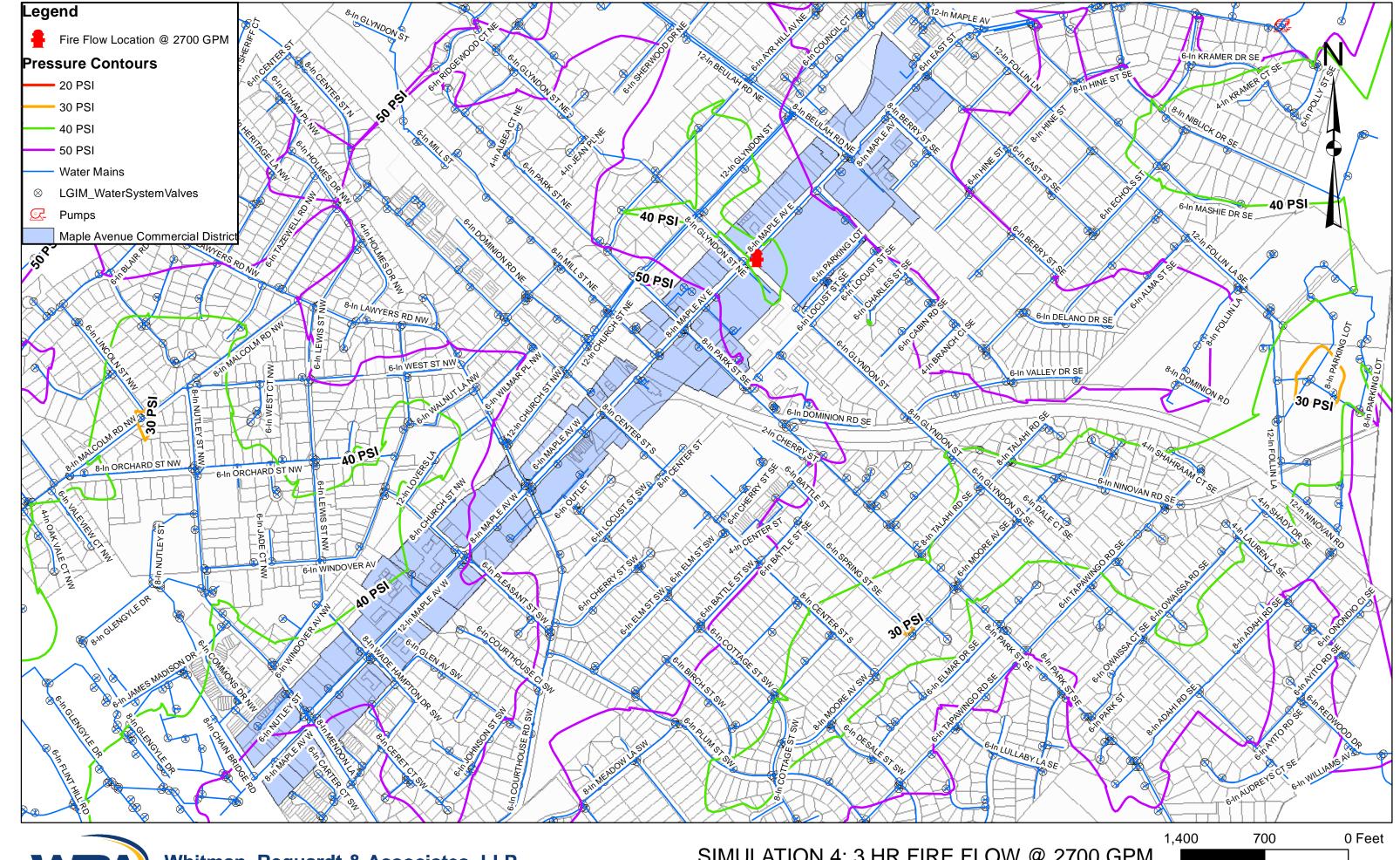


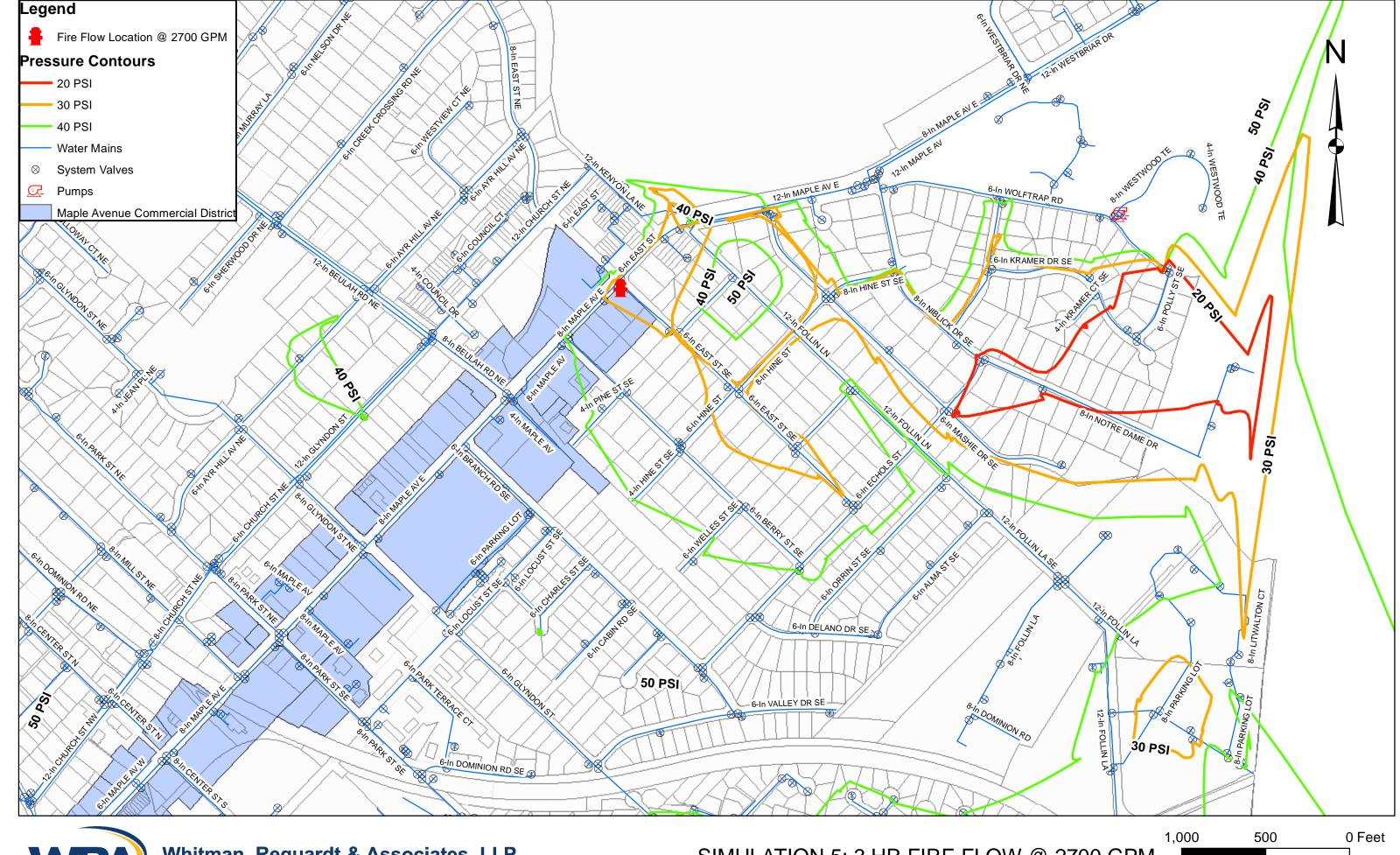
APPENDIX C 130% MAX DAILY DEMAND INCREASE PRESSURE CONTOURS HYDRANT SIMULATIONS 1-5 & ALTERNATE SIMULATION 5A

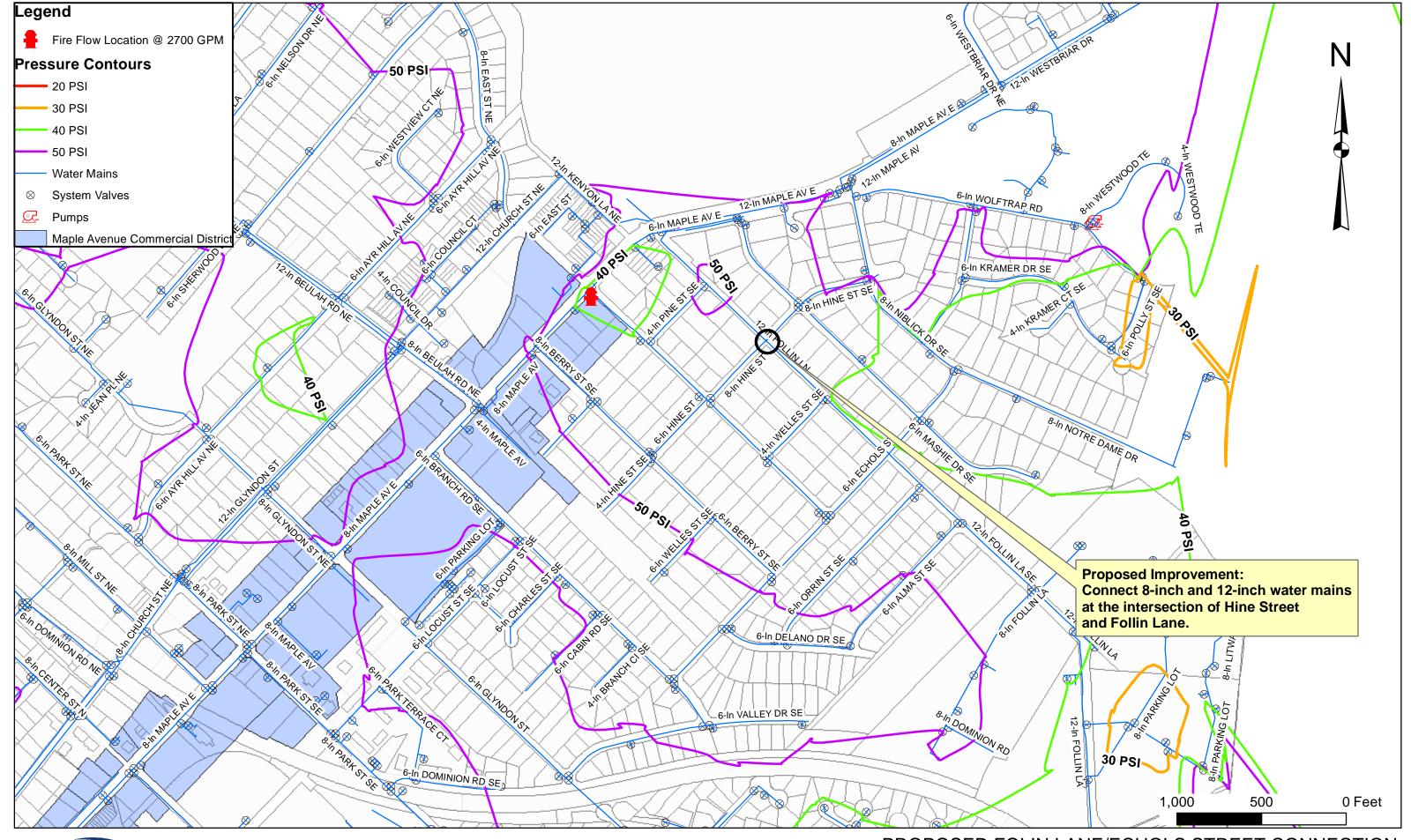














PROPOSED FOLIN LANE/ECHOLS STREET CONNECTION SIMULATION 5A: 3 HR FIRE FLOW @ 2700 GPM 130% MAX DAILY DEMAND INCREASE AT MAPLE AVENUE

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