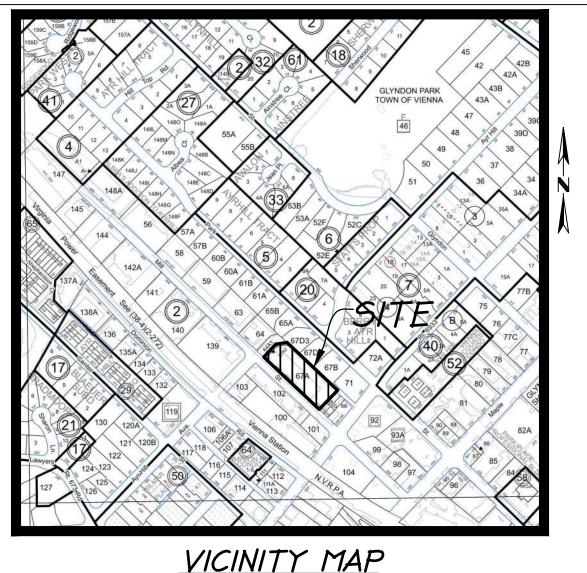
SOIL MAP

SOIL NAME

URBAN LAND

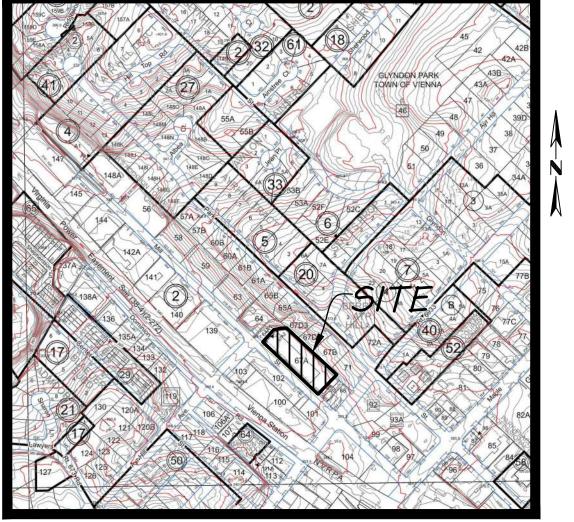


1"=500"

DRAINAGE POTENTIAL SOIL GROUP

N/A

EROSION HYDROLOGIC



TOPOGRAPHIC MAP

# CONSTRUCTION NOTES

SOIL #

95

THE PROPERTY DELINEATED IS LOCATED ON FAIRFAX COUNTY MAP No. 38-2 ((2)) PARCELS 67A \$ 67A2 AND ARE CURRENTLY ZONED CM. THE PROPERTIES SHOWN HEREON IS CURRENTLY IN THE NAME OF MILL STREET DEVELOPMENT ONE, LLC BY DEED RECORDED IN DEED BOOK 24554 AT PAGE 821 AMONG THE LAND RECORDS OF FAIRFAX COUNTY, VIRGINIA.

FOUNDATION |

N/A

SOIL INFORMATION

PROBLEM CLASS

- THE PROPERTY SHOWN HEREON LIES WITHIN A ZONE "X", AN AREA DETERMINED TO BE OUTSIDE THE 0.2% ANNUAL CHANCE FLOODPLAIN, AS DELINEATED ON FLOOD INSURANCE RATE MAP No. 51059C0145E, PANEL 145 OF 450, WITH AN EFFECTIVE DATE OF SEPTEMBER 17, 2010. THERE ARE NO RESOURCE PROTECTION AREAS ON THIS SITE AS SHOWN ON THE TOWN OF VIENNA CHESAPEAKE BAY PRESERVATION MAP.
- THIS TOPOGRAPHIC SURVEY ON THE PROPERTIES OF MILL STREET DEVELOPMENT ONE, LLC WAS COMPLETED UNDER THE DIRECT AND RESPONSIBLE CHARGE OF KEVIN D. VAUGHN FROM AN ACTUAL GROUND SURVEY MADE UNDER HIS SUPERVISION; THE ORIGINAL DATA WAS OBTAINED ON SEPTEMBER 27, 2018; AND THAT THIS PLAT MEETS MINIMUM ACCURACY STANDARDS UNLESS OTHERWISE NOTED. THE VERTICAL DATUM IS REFERENCED TO NGVD 29. THE CONTOUR INTERVAL IS TWO (2) FOOT
- UTILITY PLANS WERE NOT PROVIDED DURING THE PERFORMANCE OF THIS SURVEY. ADDITIONAL UTILITY FACILITIES AND/OR UNDERGROUND LINES MAY EXIST THAT WERE NOT EVIDENT OR IDENTIFIED. UTILITY PLANS NEED TO BE ACQUIRED AND COMPARED WITH THIS SURVEY PRIOR TO COMMENCING SITE DESIGN.
- THE EXISTING UNDERGROUND UTILITIES SHOWN HEREON ARE BASED UPON AVAILABLE INFORMATION. THE CONTRACTOR SHALL BE RESPONSIBLE FOR DETERMINING THE EXACT LOCATION OF ALL UTILITIES BEFORE COMMENCING WORK AND FOR ANY DAMAGES WHICH MAY OCCUR BY HIS FAILURE TO LOCATE OR PRESERVE THESE UNDERGROUND UTILITIES. IF DURING CONSTRUCTION OPERATIONS THE CONTRACTOR SHOULD ENCOUNTER UTILITIES OTHER THAN IN THOSE SHOWN ON THE PLANS, HE SHALL IMMEDIATELY NOTIFY THE ENGINEER AND TAKE NECESSARY AND PROPER STEPS TO PROTECT THE FACILITY AND ASSURE THE CONTINUANCE OF SERVICE.
- THE CONTRACTOR SHALL DIG TEST PITS 🕀 AS REQUIRED FOLLOWING NOTIFICATION AND MARKING OF ALL EXISTING UTILITIES BY MISS UTILITY TO VERIFY THE LOCATION AND DEPTH OF EXISTING UTILITIES. TEST HOLES TO BE PERFORMED AT LEAST 30 DAYS PRIOR TO START OF CONSTRUCTION. ANY DISCREPANCIES ARE TO BE REPORTED IMMEDIATELY TO THE OWNER AND ENGINEER. REDESIGN AND APPROVAL BY REVIEWING AGENCIES SHALL BE OBTAINED IF REQUIRED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR NOTIFYING THE OWNER AND THE ENGINEER OF ANY CHANGES OR CONDITIONS ATTACHED TO PERMITS OBTAINED FROM ANY AUTHORITY ISSUING PERMITS.
- THE CONTRACTOR SHALL VISIT THE SITE AND SHALL VERIFY EXISTING CONDITIONS PRIOR TO STARTING CONSTRUCTION. THE CONTRACTOR SHALL CLEAR THE SITE OF ALL TREES, BUILDINGS, FOUNDATIONS, ETC. WITHIN THE LIMITS OF CONSTRUCTION UNLESS
- OTHERWISE SPECIFIED, AND SHALL BE RESPONSIBLE FOR CAUSING EXISTING UTILITIES TO BE DISCONNECTED. FINISHED GRADES SHOWN IN PROFILE FOR FINISHED TOP OF CURB GRADES ON EXISTING ROADS SHALL BE FIELD ADJUSTED AS REQUIRED TO CONFORM TO THE INTENT OF THE TYPICAL SECTION USING THE EXISTING EDGE OF PAVEMENT AS THE CONTROL POINT. A SMOOTH GRADE SHALL BE MAINTAINED FROM THE CENTERLINE OF THE EXISTING RIGHT-OF-WAY TO THE FACE OF CURB TO PRECLUDE THE FORMING OF FALSE GUTTERS AND/OR THE PONDING OF WATER ON THE ROADWAY. THE EXISTING PAVEMENT SHALL BE RECAPPED AND/OR REMOVED AND REPLACED AS REQUIRED TO ACCOMPLISH THIS REQUIREMENT. CURB FORMS SHALL BE INSPECTED AND
- APPROVED FOR HORIZONTAL AND VERTICAL ALIGNMENT BY TOWN OF VIENNA INSPECTORS PRIOR TO PLACING OF CONCRETE. ALL AREAS, ON OR OFF-SITE, WHICH ARE DISTURBED BY THIS CONSTRUCTION AND WHICH ARE NOT PAVED OR BUILT UPON, SHALL BE ADEQUATELY STABILIZED TO CONTROL EROSION AND SEDIMENTATION. THE MINIMUM ACCEPTABLE STABILIZATION SHALL CONSIST OF PERMANENT GRASS, SEED MIXTURE TO BE AS RECOMMENDED BY THE TOWN AGENT. ALL SLOPES 3:1 AND GREATER SHALL BE SODDED
- AND PEGGED OR OTHERWISE STABILIZED IN A MANNER APPROVED BY THE TOWN OF VIENNA. EXISTING WELLS SHALL BE PERMANENTLY ABANDONED IN ACCORDANCE WITH VIRGINIA DEPARTMENT OF ENVIRONMENTAL QUALITY STANDARDS AND REQUIREMENTS.
- EXISTING SEPTIC FIELDS SHALL BE ABANDONED IN ACCORDANCE WITH VIRGINIA DEPARTMENT OF ENVIRONMENTAL QUALITY STANDARDS AND REQUIREMENTS. ALL OVER HEAD POLE LINES SHALL BE RELOCATED AS REQUIRED BY THE OWNING UTILITY COMPANIES. THE CONTRACTOR SHALL BE
- RESPONSIBLE FOR MAKING ALL ARRANGEMENTS AND COORDINATING ALL WORK REQUIRED FOR THE NECESSARY RELOCATIONS. THE CONTRACTOR IS TO VERIFY INVERT, SIZE AND LOCATION OF BUILDING UTILITY CONNECTIONS WITH THE MECHANICAL PLANS PRIOR TO PLACEMENT OF UNDERGROUND UTILITIES.
- NO UNDERGROUND SOILS INVESTIGATION HAS BEEN PERFORMED BY LAND DESIGN CONSULTANTS, INC. ALL SOILS INFORMATION PRESENTED AS PART OF THIS SITE PLAN HAS BEEN PREPARED BY OTHERS.
- EXISTING BUILDINGS, FENCES AND OTHER EXISTING PHYSICAL FEATURES ARE TO BE REMOVED AS INDICATED ON THE DEMOLITION PLAN. ALL REQUIRED DEMOLITION PERMITS WILL BE OBTAINED PRIOR TO DEMOLITION OF ANY FEATURES.
- 20. DAMAGE TO ANY EXISTING ENTRANCES, CURB AND GUTTER, PAVEMENT OR OTHER EXISTING STRUCTURES NOT PROPOSED TO BE DISTURBED WITH THIS DEVELOPMENT, WILL BE THE RESPONSIBILITY OF THE CONTRACTOR AND MUST BE REPAIRED TO THE SATISFACTION OF TOWN OF VIENNA AND ANY ADJOINING OWNERS THAT MAY BE AFFECTED.
- THE CONTRACTOR SHALL BE RESPONSIBLE FOR MAKING A SMOOTH TRANSITION TO EXISTING PAVEMENT.
- 22. ALL PRIVATE BUILDING CONNECTIONS ARE TO BE INSTALLED IN ACCORDANCE WITH THE CURRENT PLUMBING CODE.
- 23. TOPS OF EXISTING STRUCTURES WHICH REMAIN IN USE ARE TO BE ADJUSTED IN ACCORDANCE WITH THE GRADING PLAN. ALL PROPOSED STRUCTURE TOP ELEVATIONS ARE TO BE VERIFIED BY THE CONTRACTOR WITH THE SITE GRADING PLANS. IN CASE OF CONFLICT, THE GRADING PLAN SHALL SUPERSEDE PROFILE ELEVATIONS. MINOR ADJUSTMENTS TO MEET FINISHED GRADE ELEVATIONS MAY BE REQUIRED. 24. THE DESIGN, CONSTRUCTION, FIELD PRACTICES AND METHODS SHALL CONFORM TO THE REQUIREMENTS SET FORTH BY THE GOVERNING AGENCY AND ITS CURRENT SUBDIVISION ORDINANCE. FAILURE TO COMPLY WITH THE CODE, APPLICABLE MANUALS, PROVISIONS OF THE
- CONSTRUCTION AND ESCROW AGREEMENTS OR THE PERMITS SHALL BE DEEMED A VIOLATION. THE APPROVAL OF THESE PLANS SHALL IN NO WAY RELIEVE THE OWNER/DEVELOPER OR HIS AGENT OF ANY LEGAL RESPONSIBILITIES WHICH MAY BE REQUIRED BY THE CODE OF VIRGINIA OR ANY ORDINANCE ENACTED BY THE GOVERNING AGENCY.
- A MINIMUM PERMISSIBLE GRADE OF 1.00% IS REQUIRED FOR PAVEMENT TO ASSURE POSITIVE DRAINAGE. IF THERE IS EXISTING PAVEMENT WHICH IS TO REMAIN DISTURBED DURING CONSTRUCTION AND IS LESS THAN 1.00%, THEN THE CONTRACTOR IS TO CHECK TO MAKE SURE THE SITE AREA WILL HAVE ADEQUATE DRAINAGE.
- CONSTRUCTION STAKEOUT SHALL BE UNDER THE DIRECT SUPERVISION OF A LICENSED LAND SURVEYOR IN THE STATE OF VIRGINIA THIS PROPERTY SHALL BE SERVED BY PUBLIC WATER AND SANITARY SEWER.
- 29. AIR QUALITY PERMITS SHALL BE OBTAINED, IF REQUIRED, AND PROVIDED PRIOR TO ANY CLEARING, GRADING OR OTHER CONSTRUCTION. 30. NO GRAVES HAVE BEEN FOUND ON THIS SITE.
- 31. SUBBASE DEPTH IS BASED ON A CBR VALUE OF 10, BASED ON AN ACTUAL DETERMINATION PER SOIL TESTS (OR) AN ESTIMATE WHICH WILL BE REVISED ONCE THE SOIL TESTS OF SUBGRADE ARE PERFORMED. 32. THE METHODS AND MATERIALS USED IN THE CONSTRUCTION OF ALL ROADS SHALL CONFORM TO THE CURRENT TOWN OF VIENNA OR VDOT STANDARD SPECIFICATIONS, UNLESS HEREIN MODIFIED. ALL CONSTRUCTION WITHIN THE RIGHT-OF-WAY SHALL CONFORM TO TOWN
- OF VIENNA STANDARDS. 33. THE SITE PLAN WAS PREPARED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE GEOTECHNICAL REPORT.
- ALL CLEANOUTS WITHIN PAVED AREAS SHALL BE FITTED WITH A TRAFFIC-RATED COVER AND ADJUSTED TO FINISHED GRADE. ALL RETAINING WALLS EQUAL TO OR GREATER THAN 3' IN HEIGHT SHALL REQUIRE A SEPARATE BUILDING PERMIT AND HANDRAIL. A BUILDING PERMIT SHALL BE OBTAINED FROM THE BUILDING DIVISION PRIOR TO
- CONSTRUCTION OF THE RETAINING WALLS. THE FEE TITLE OWNER SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF ALL PRIVATE DRAINAGE, STORM WATER MANAGEMENT, AND BEST MANAGEMENT PRACTICES FACILITIES AND SYSTEMS IN ACCORDANCE WITH THE MAINTENANCE AGREEMENT TO ENSURE THAT THEY FUNCTION PROPERLY.

SUBJECT TO OTHER LIMITATIONS, THE FEE TITLE OWNER MAY LANDSCAPE THE EASEMENT TO INCLUDE VEGETATION, SIGNS AND FENCES, PROVIDED THAT DRAINAGE AND THE TOWN OR THE OWNER'S ABILITY TO ACCESS THE EASEMENT IS NOT COMPROMISED AND THAT THE TOWN IS NOT IN ANY WAY RESPONSIBLE FOR THE REPAIRS OF THESE LANDSCAPE ITEMS EVEN IF DAMAGED BY TOWN FORCES.

# WETLANDS PERMITS CERTIFICATION

I HEREBY CERTIFY THAT ALL WETLANDS PERMITS REQUIRED BY LAW WILL BE OBTAINED PRIOR TO COMMENCING WITH LAND DISTURBING ACTIVITIES.

OWNER/DEVELOPER MIXIMA A POSMI TITLE

# SIGNATURE MANAGERS MEMBER NOTE: PERMITS MUST BE PRESENTED TO THE COUNTY INSPECTOR PRIOR TO LAND

# \*\*Included in overall parking NOTE: REQUIRED PARKING FOR SELF STORAGE BASED ON THE 5TH ED. ITE MANUAL. (0.10 PER 1,000SF GSF) TRUP. OLLI STURAGL TACILITT 4 STORIES 129,916 G.S.F. FULLY SPRINKLERED

17

19

ZONING TABULATIONS

# EMPLOYEES PER SHIFT = 1

ANTICIPATED # STORAGE UNITS = 1,150 UNITS

PARKING TABULATIONS

SPACES PROVIDED

\*1 loading space required

GSF SPACES PER 1,000 SF SPACES REQUIRED

Self-Storage Facility

SITE AREA

Max Building Height Prop. Building Height Lot Coverage

131,084

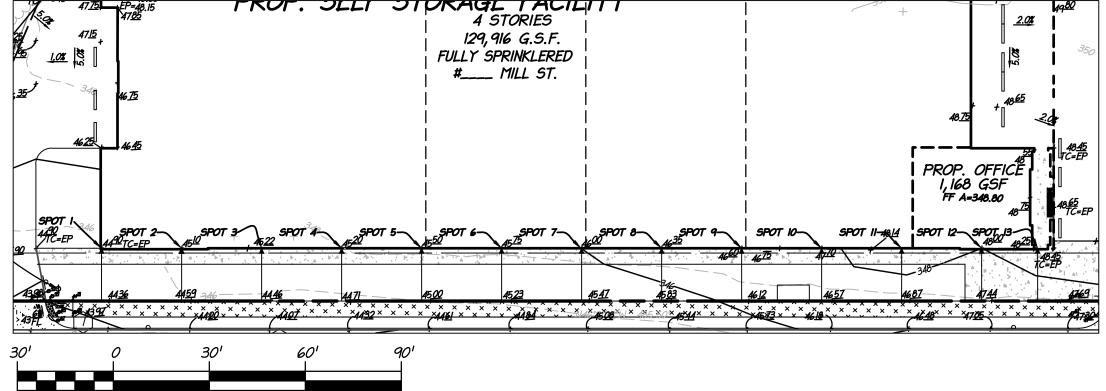
Proposed Use Front Yard Side Yard Rear Yard

SELF-STORAGE

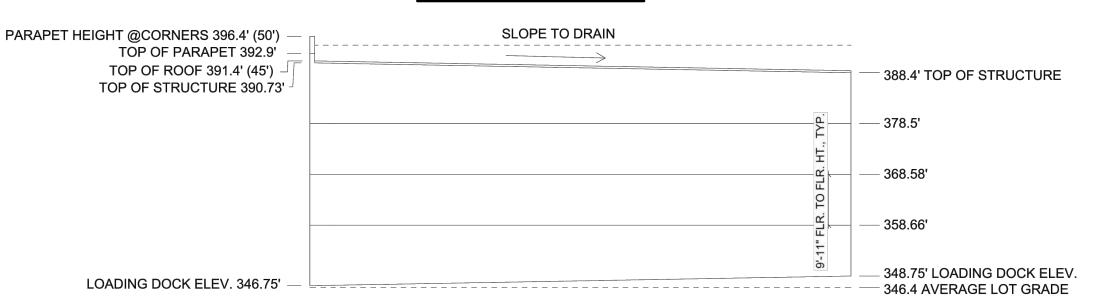
TOTAL SPACES PROVIDED

Handicap Accessible Required Handicap Accessible Provided\*\*

LOADING\* STANDARD 54,705# (1.26 Ac.)



### SIDE ELEVATION



# FRONT ELEVATION

BUIL	DING HEIGHT	COMPUTAT	IONS
SPOT	PRE-EX.	F.L.G.	DEV.
1	345.21	344.90	-0.31
2	348.24	345.10	-3.14
3	348.08	345.22	-2.86
4	348.01	345.20	-2.81
5	347.70	345.50	-2.20
6	347.97	345.75	-2.22
7	348.30	346.00	-2.30
8	348.42	346.35	-2.07
9	348.83	346.60	-2.23
10	349.17	347.70	-1.47
11	349.20	348.14	-1.06
12	349.39	348.00	-1.39
C. Contract		The Contract of the Contract o	119/11/2005

13 349.30 348.25 -1.05

- F.L.G. FINISHED LOT GRADE - DEV. - DEVIATION OF F.L.G. FROM PRE-EXISTING GRADE - BUILDING HEIGHT EXCLUDES PARAPETS
- AVERAGE FRONT GRADE = 346.40' - PROPOSED TOP OF ROOF = 391.40'
- MAX BUILDING ELEV = 391.40' - TOP OF PARAPET ALONG FACE = 392.90' - TOP OF PARAPET AT CORNER = 396.40'
- TOTAL HEIGHT TO ROOF = 45'
- TOTAL HEIGHT TO TOP OF PARAPET AT FRONT CORNER = 50'

### OVERALL PROJECT NARRATIVE:

THE PROPOSED SITEPLAN, APPROX. 1.26 ACRES, SHALL PROVIDE FOR THE CONSTRUCTION OF A SELF-STORAGE FACILITY. THE EXISTING ON-SITE STRUCTURES SHALL BE RAZED. THE SCOPE OF THIS PROJECT SHALL INCLUDE CONSTRUCTION OF PRIVATE PARKING AND DRIVE AISLE, OFF SITE PARKING, MILLING AND OVERLAY OF MILL STREET, AN UNDERGROUND SWM FACILITY, STORM & SANITARY SEWER AND WATERLINE. THE LIMIT OF WORK AREA IS APPROXIMATELY ±1.7 ACRES.

### FIRE MARSHAL NOTES

2,951 GPM AVAILABLE AVAILABLE FIRE FLOW SOURCE OF FIRE FLOW INFO. \_\_\_\_\_TOWN OF VIENNA (12/09/2019) TYPE OF CONSTRUCTION - USBC \_\_\_

PROPOSED BUILDING HEIGHT \_\_\_\_\_ BUILDING TO BE FULLY SPRINKLERED YES | NO

IF YES, CHECK APPROPRIATE STANDARD: NFPA 13 ☑ ; NFPA 13D □ ; NFPA 13R □ PROPOSED NUMBER OF FLOORS 4

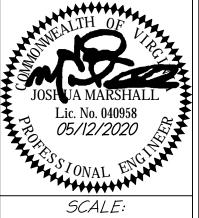
# SHEET INDEX

SHEET No. DESCRIPTION COVER SHEET CONSTRUCTION DETAILS EXISTING CONDITIONS & DEMOLITION PLAN GRADING PLAN MILL STREET FRONTAGE DETAILED GRADING PLAN SITE PLAN MILL & OVERLAY PLAN EROSION & SEDIMENT CONTROL - PHASE I EROSION & SEDIMENT CONTROL - PHASE II EROSION & SEDIMENT CONTROL NARRATIVES & DETAILS UTILITY PROFILE & SANITARY SEWER LATERAL TABLE IOA-IOC. DRY UTILITY RELOCATION WATERLINE PROFILES FIRE LANE PLAN AUTOTURN EXHIBIT STORM SEWER PROFILES & COMPUTATIONS PRE AND POST STORM SEWER MAPS PRE AND POST SWM DIVIDE MAPS SWM COMPUTATIONS & RELEASE RATES SWM ROUTINGS & HYDROGRAPHS ADS STORMTECH DETAILS 20-21. 22. VRRM SPREADSHEET *2*3. PHOTOMETRIC PLAN LANDSCAPE PLAN # DETAILS

TREE PRESERVATION PLAN FEE CALCULATION: SITE PLAN - \$2,000 PLUS \$30 FOR EACH 1,000 SF OF GROSS FLOOR AREA OF NEW CONSTRUCTION. GROSS FLOOR AREA = 130,000 SF FEE = 2,000 + (130,000/1000)\*30 = \$5,900.00

OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE. Lic. No. 040958 05/12/2020

I HEREBY CERTIFY THAT



AS NOTED

of 25

AUGUST 2, 2019 DRAFT: CHECK: CBS PDRFILE NUMBER:

16220-2-0

P:\PY 2016\16220-2-0 Mill Street, N.E. - 215-241\ENG\SITEPLAN\16220-2-0-CVR.dwg

### ZONING ORDINANCE WAIVER & MODIFICATION



December 17, 2015

Resolution Escrow Services, LLC 7164 Columbia Gateway Drive, Suite 210-B Columbia, MD 21046 Attn: Richard Sterm

RE: Zoning Compliance Letter for 211-241 Mill Street NE, Vienna, VA (to be referred to as "The Property")

### To Whom It May Concern,

Please allow this letter to serve as an official Zoning Compliance letter for the property located at 211-241 Mill Street NE, Tax Map Numbers 38-2 ((2)) 70, 38-2 ((2)) 69, 38-2 ((2)) 67A and 38-2 ((2)) 67A2, currently under the ownership of Mill Street Development One LLC.

The four parcels are zoned CM, Limited Industrial. Uses permitted in the C-2 zone (as well as C-1A and C-1 zones) are permitted in the CM zone as long as those uses shall conform to any specific requirements or limitations in the CM zone. Limited manufacturing and industrial uses, as listed under Section 18-118 of the Town Code, are also permitted in the CM zone. The code section for the CM zone, Article 15 of Chapter 18 of the Town Code, has been attached.

The Property is not located in any Resource Management Area or Resource Protection Area, There are no outstanding notices of violation applicable to the Property.

The Property is not subject to any Special Development Approvals, Special Exceptions, Variances or Special Use Permits.

There are no legal nonconforming uses or conditions at the Property. The Property is also not subject to the satisfaction of any proffered conditions.

If you have any further questions regarding this property, please do not hesitate to contact my

Patrick Mulhern, AICP Director of Planning and Zoning Town of Vienna, VA

> 127 Center Street, South • Vienna, Virginia 22180 p: (703) 255-6341 • f: (703) 255-5729 • TTY711 www.viennava.gov

January 3, 2020

Mr. Mike Gallagher, P.E. Director of Public Works Mr. Frank Simeck, CZA Director of Planning & Zoning Zoning Administrator Town of Vienna 127 Center Street, South Vienna, VA 22180

Re: Mill Street Self Storage Request for Zoning Ordinance Modification and Waiver LDC Project #16220-2-0

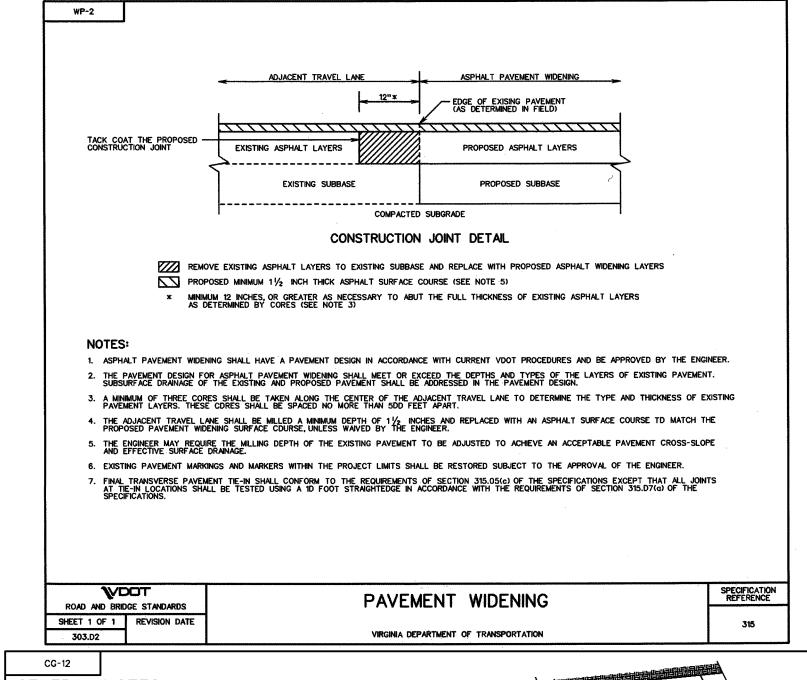
This is a request for a waiver and modification to two provisions set forth in the Zoning Ordinance and as outlined on the associated site plan for the above-referenced application. This waiver and modification are as follows:

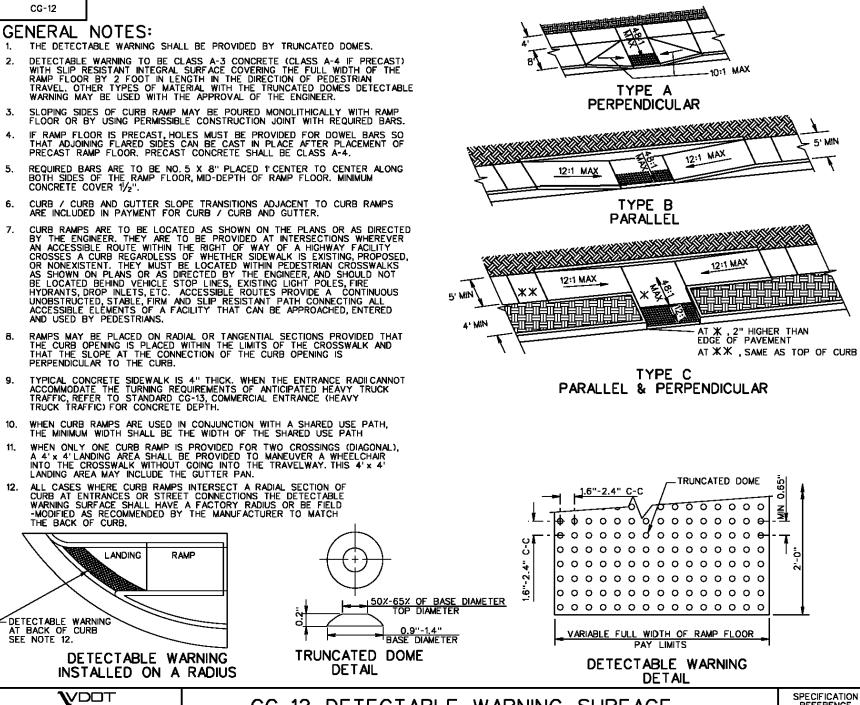
- 1. The applicant respectfully requests a waiver of the masonry screen wall requirement where upon any commercial use is conducted shall be screened from all adjacent property in any detached residential zones as set forth by Section 18-172 of the Zoning Ordinance. A masonry screen wall is required by the Zoning Ordinance along rear property line of the application. The construction of the wall as codified requires the installation of a frost footer which may damage the root structure of the adjacent, offsite mature trees and vegetation. It is our intent to ensure this existing vegetation remains in excellent condition.
- 2. The applicant respectfully requests a modification to the loading space size requirement for the site as set forth by Section 18-132 of the Zoning Ordinance. This modification contemplates a changing the requirement of the loading space based on width of the building to instead provide two spaces of at least 25 feet in depth by 15 feet in width. The existing condition of the application boasts no existing loading spaces. As the primary function of the building will serve as self storage it is anticipated that the large vehicle traffic will consist of smaller box trucks in lieu of larger delivery vehicles at the site.

If you have any questions, feel free to call me at 703-680-4585.

Sincerely,

Paul Reynolds, P.E. Project Manager

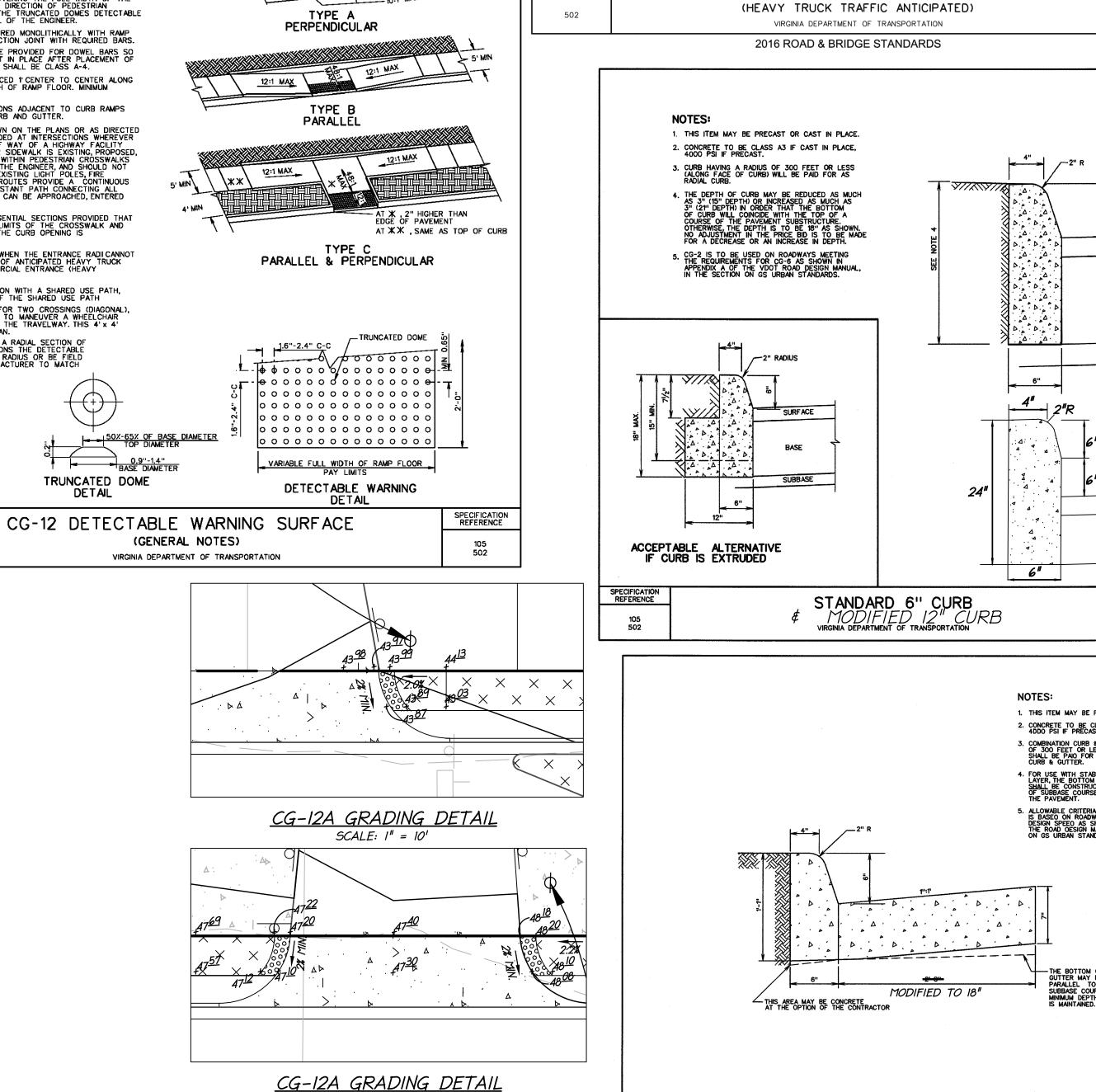




ROAD AND BRIDGE STANDARDS

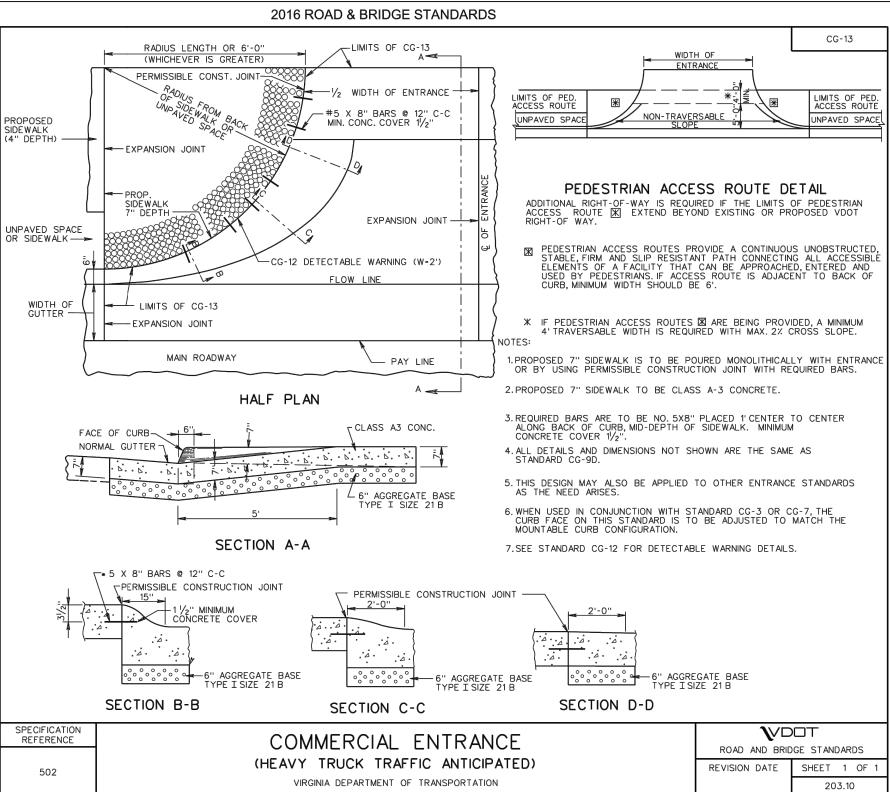
SHEET 1 OF 5 REVISION DATE

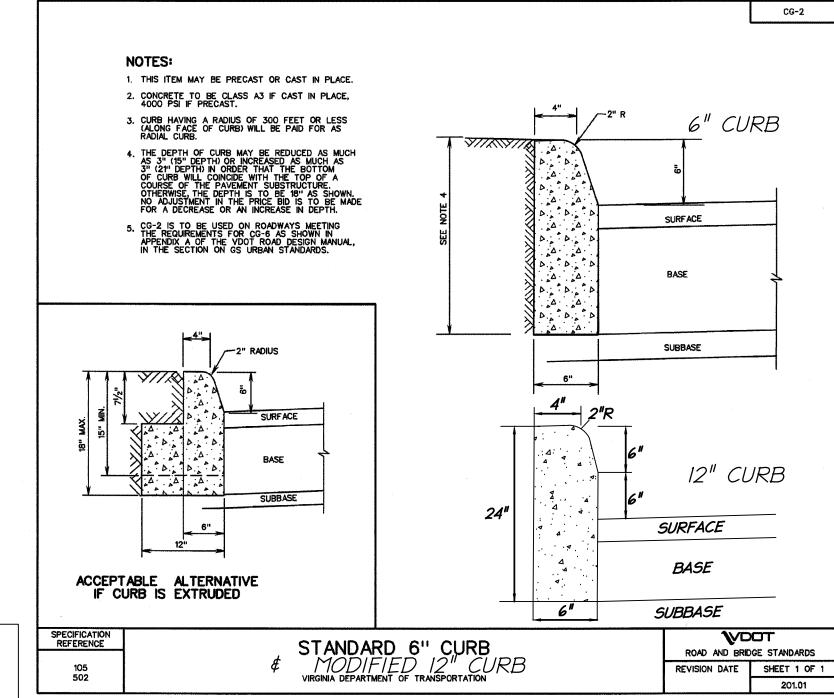
203.05

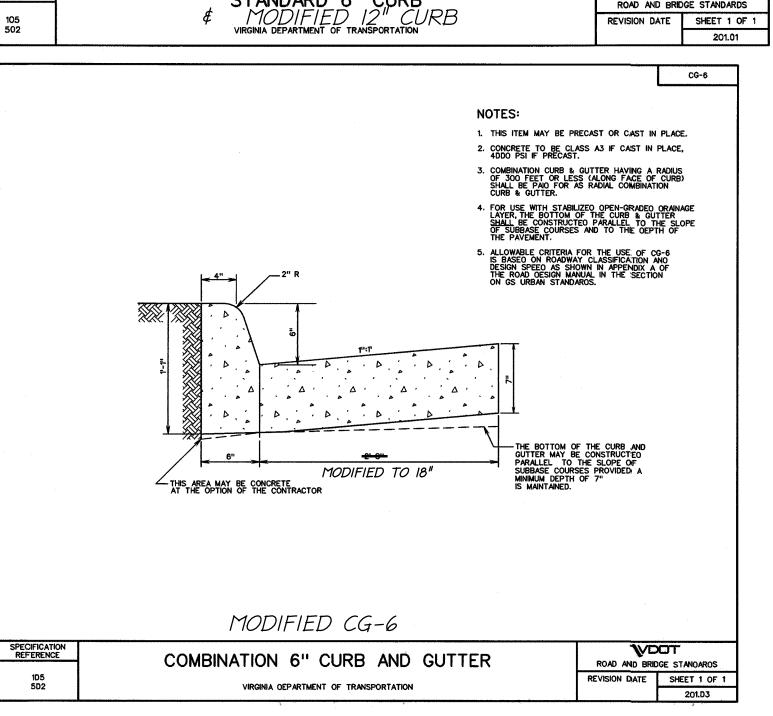


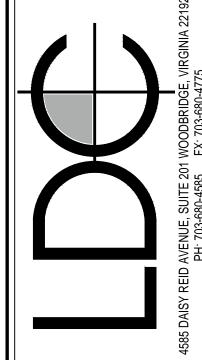
SCALE: I" = 10'

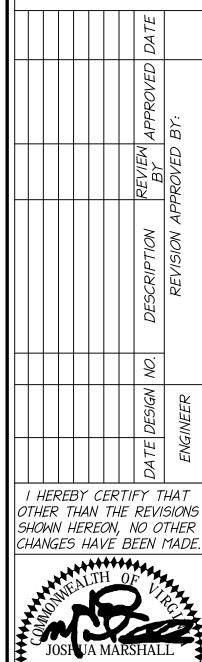
GRAPHIC SCALE:





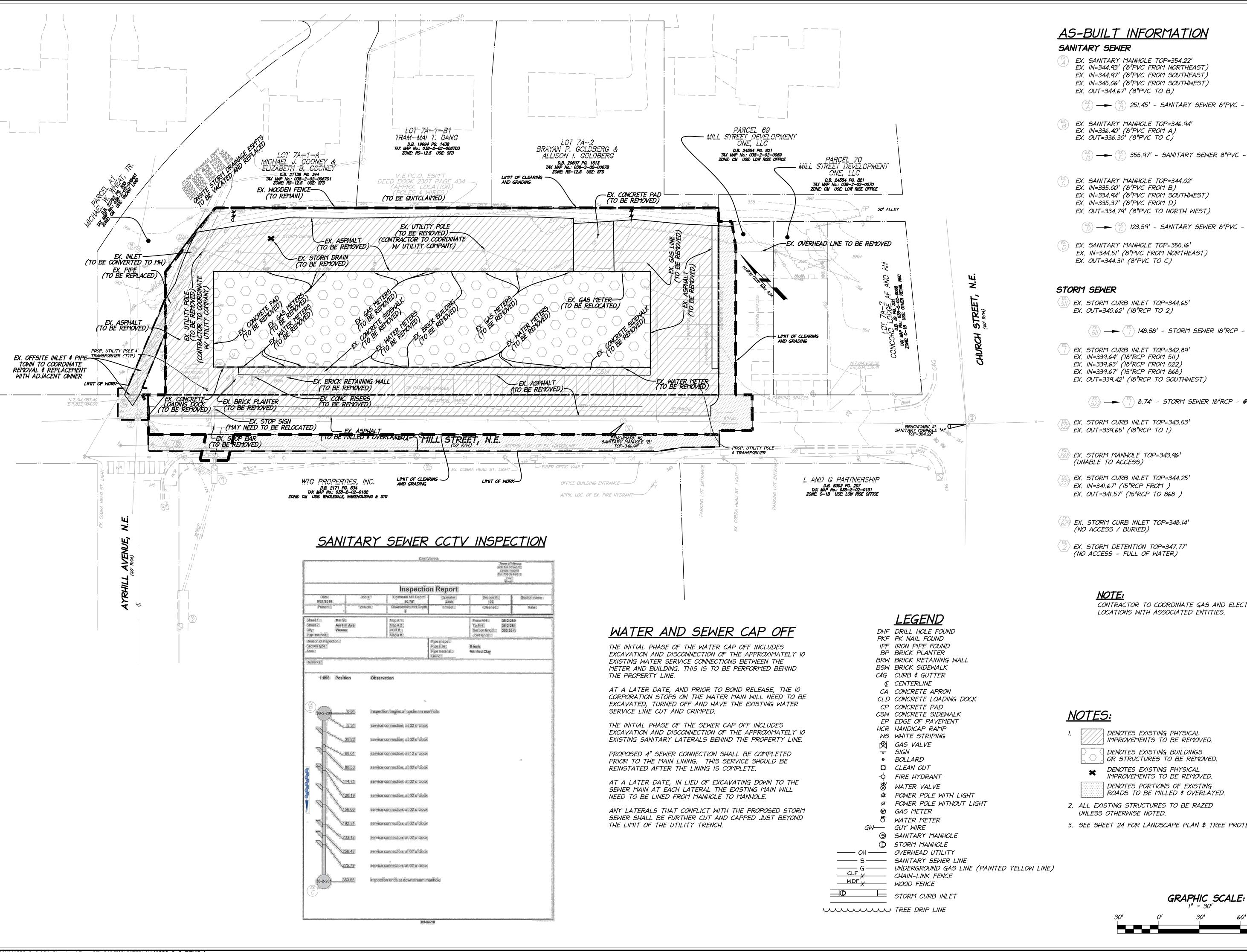






of 25 AUGUST 2, 2019 DRAFT: CHECK: FILE NUMBER: 16220-2-0

P:\PY 2016\16220-2-0 Mill Street, N.E. - 215-241\ENG\SITEPLAN\16220-2-0-CVR.dwg



 $\binom{\mathcal{E}(A)}{A} \longrightarrow \binom{\mathcal{E}(A)}{B}$  251.45' - SANITARY SEWER 8"PVC - @ 3.29%

(EX) 355.97' - SANITARY SEWER 8"PVC - @ 0.37%

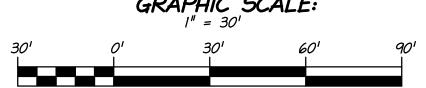
<sup>™</sup>D) — (<sup>™</sup>C) 123.59' - SANITARY SEWER 8"PVC - @ 7.23%

(EX.) → (EX.) 148.58' - STORM SEWER 18"RCP - @ 0.66%

 $\langle \frac{5}{5} \rangle \longrightarrow \langle \frac{5}{7} \rangle$  8.74' - STORM SEWER 18"RCP - @ 0.23%

CONTRACTOR TO COORDINATE GAS AND ELECTRIC METER

3. SEE SHEET 24 FOR LANDSCAPE PLAN \$ TREE PROTECTION NOTE.



SHEET 3 of 25 AUGUST 2, 2019 DRAFT:

> PDRFILE NUMBER: 16220-2-0

CHECK:

I HEREBY CERTIFY THAT

OTHER THAN THE REVISIONS

SHOWN HEREON, NO OTHER

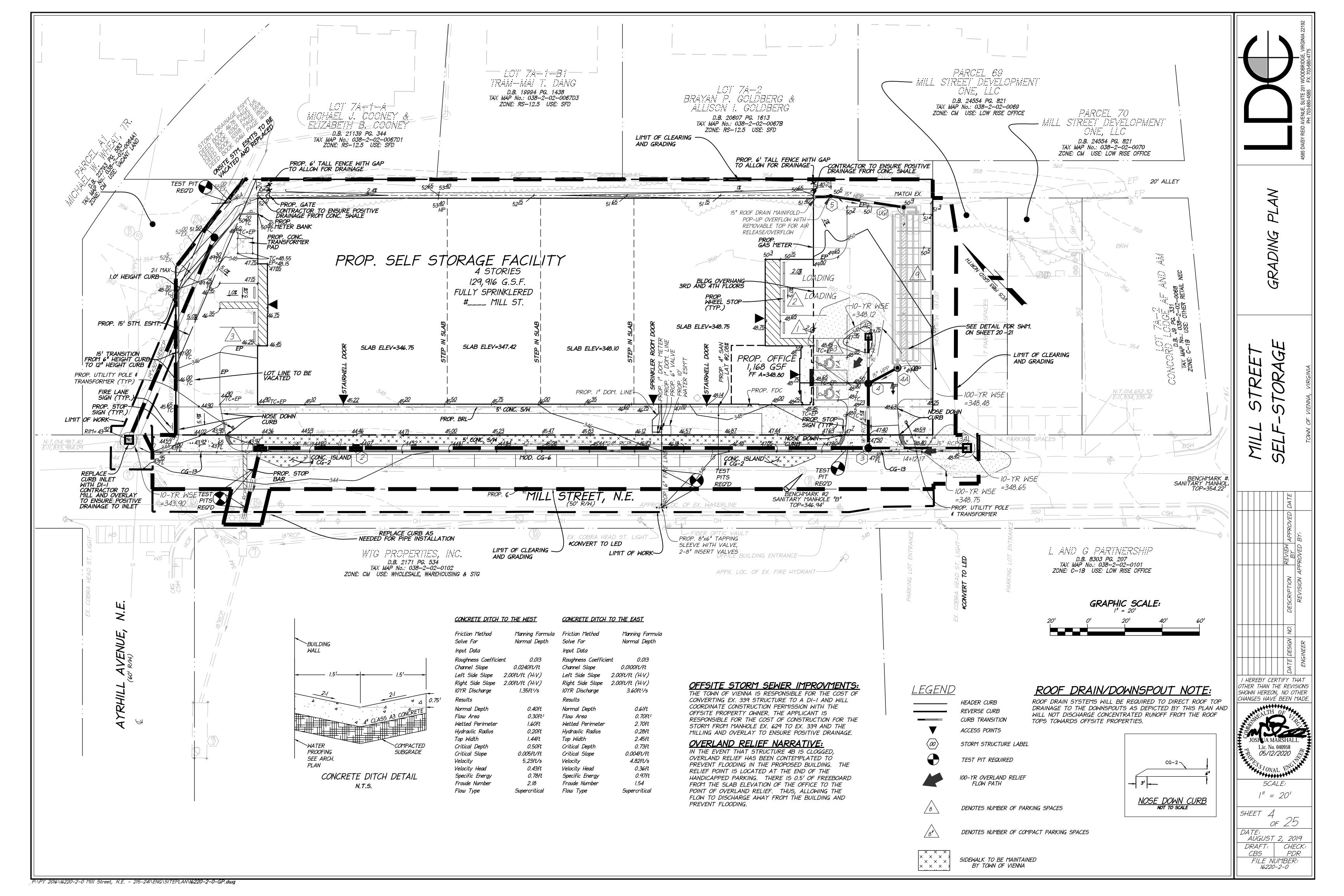
CHANGES HAVE BEEN MADE.

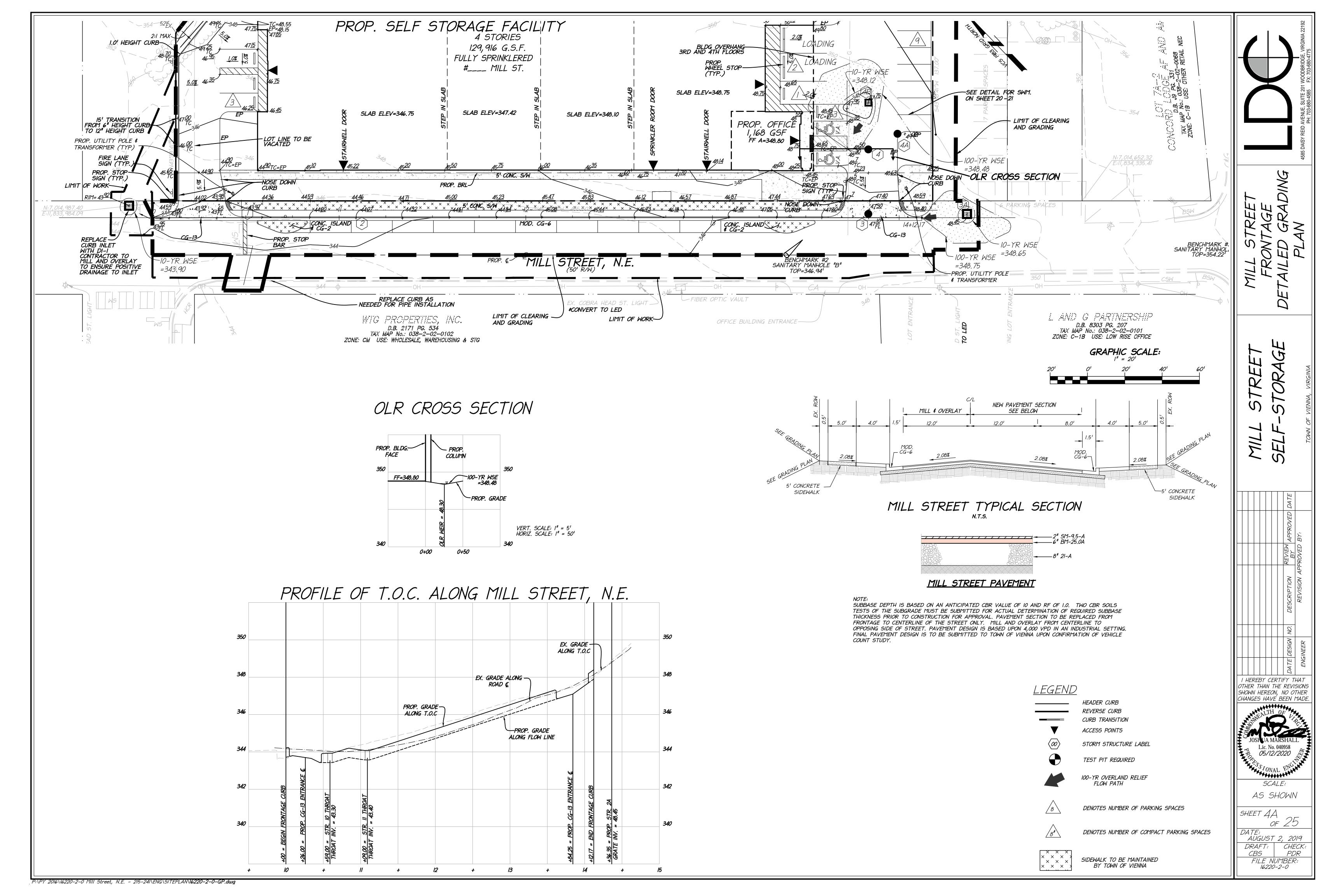
05/12/2020

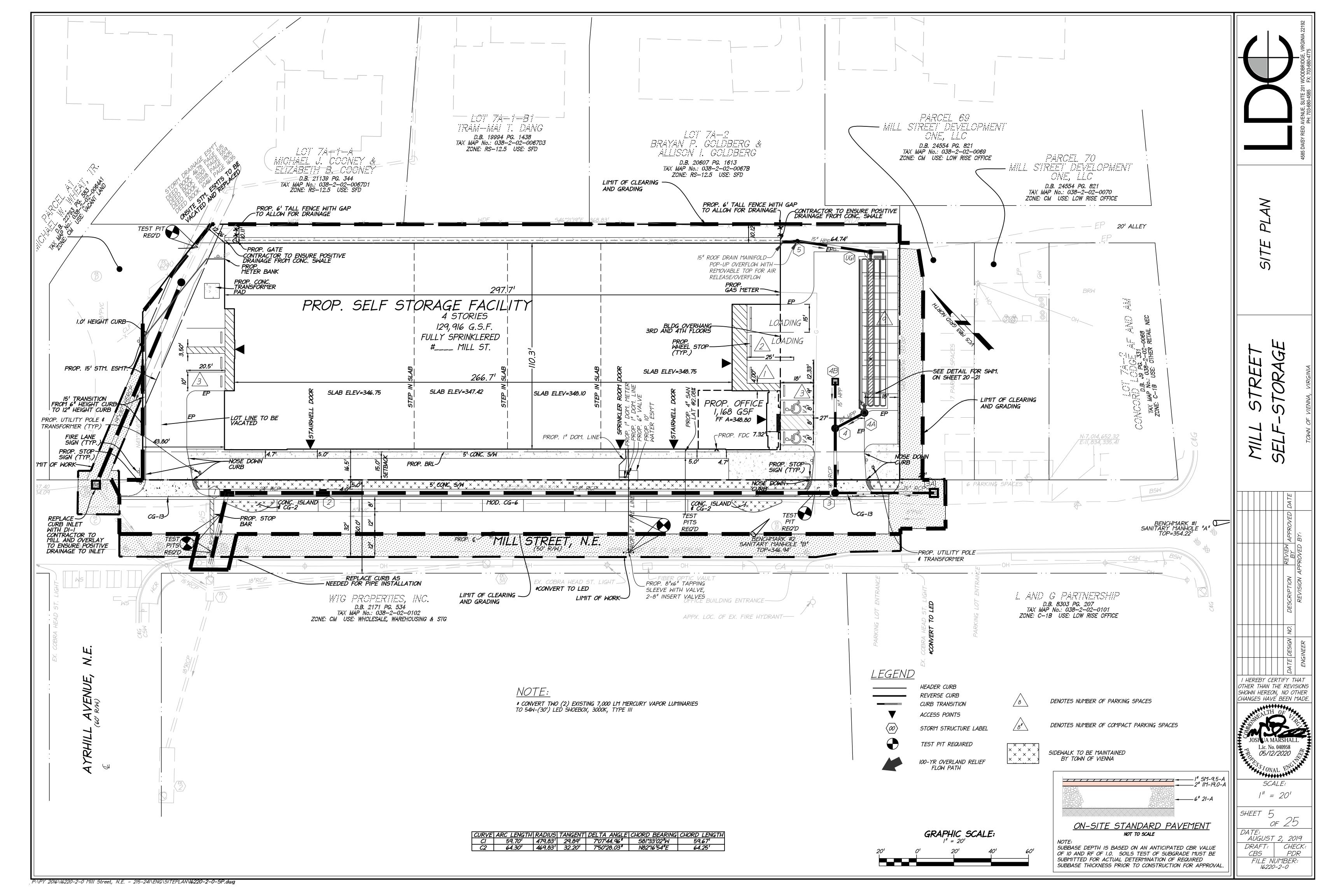
SCALE:

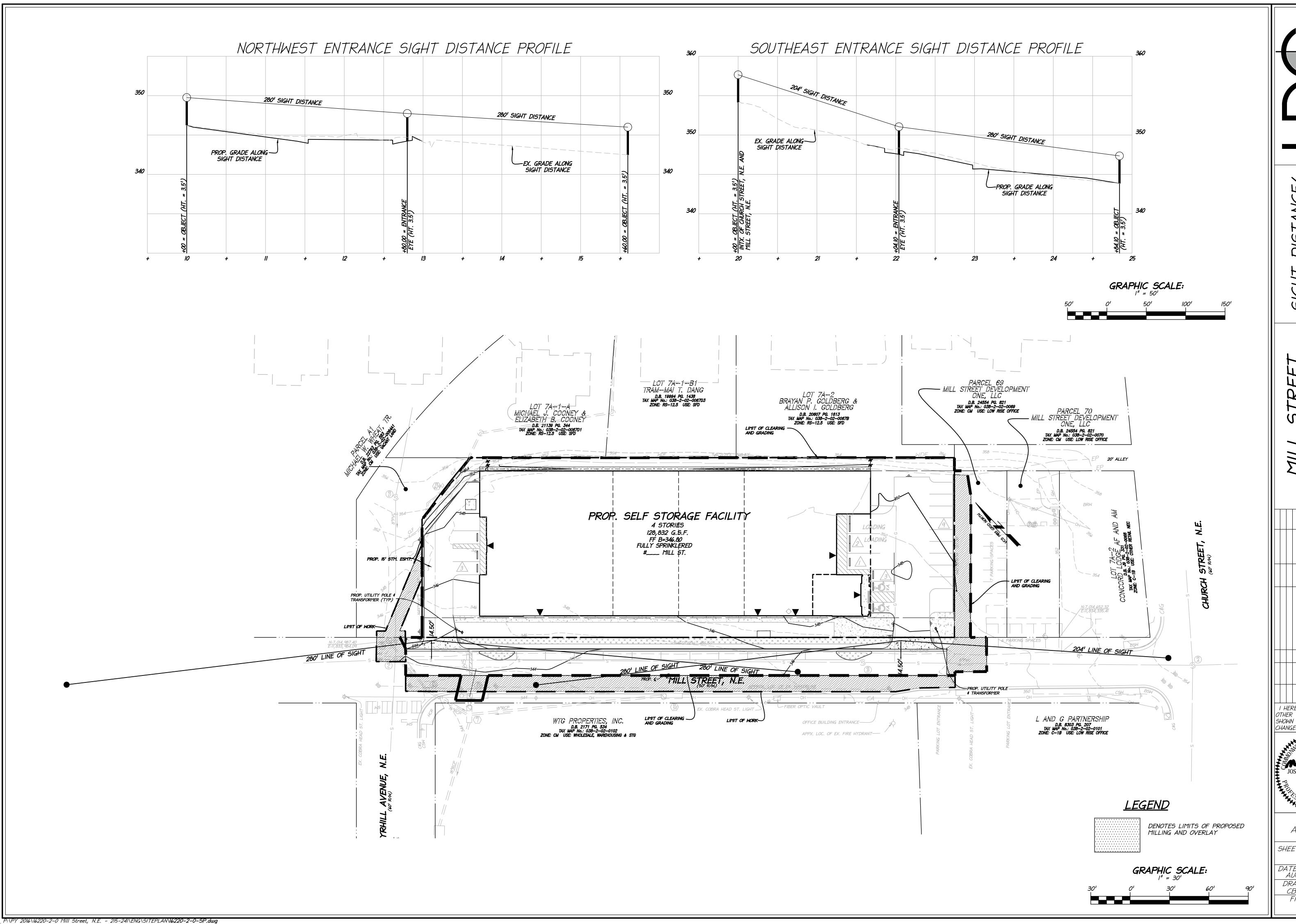
I'' = 30'

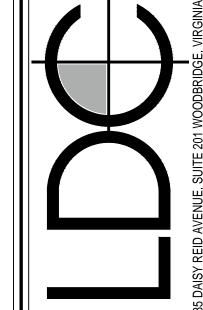
ANTONIA PARTY











SIGHT DISTANCE/ MILL & OVERLAY PI AN

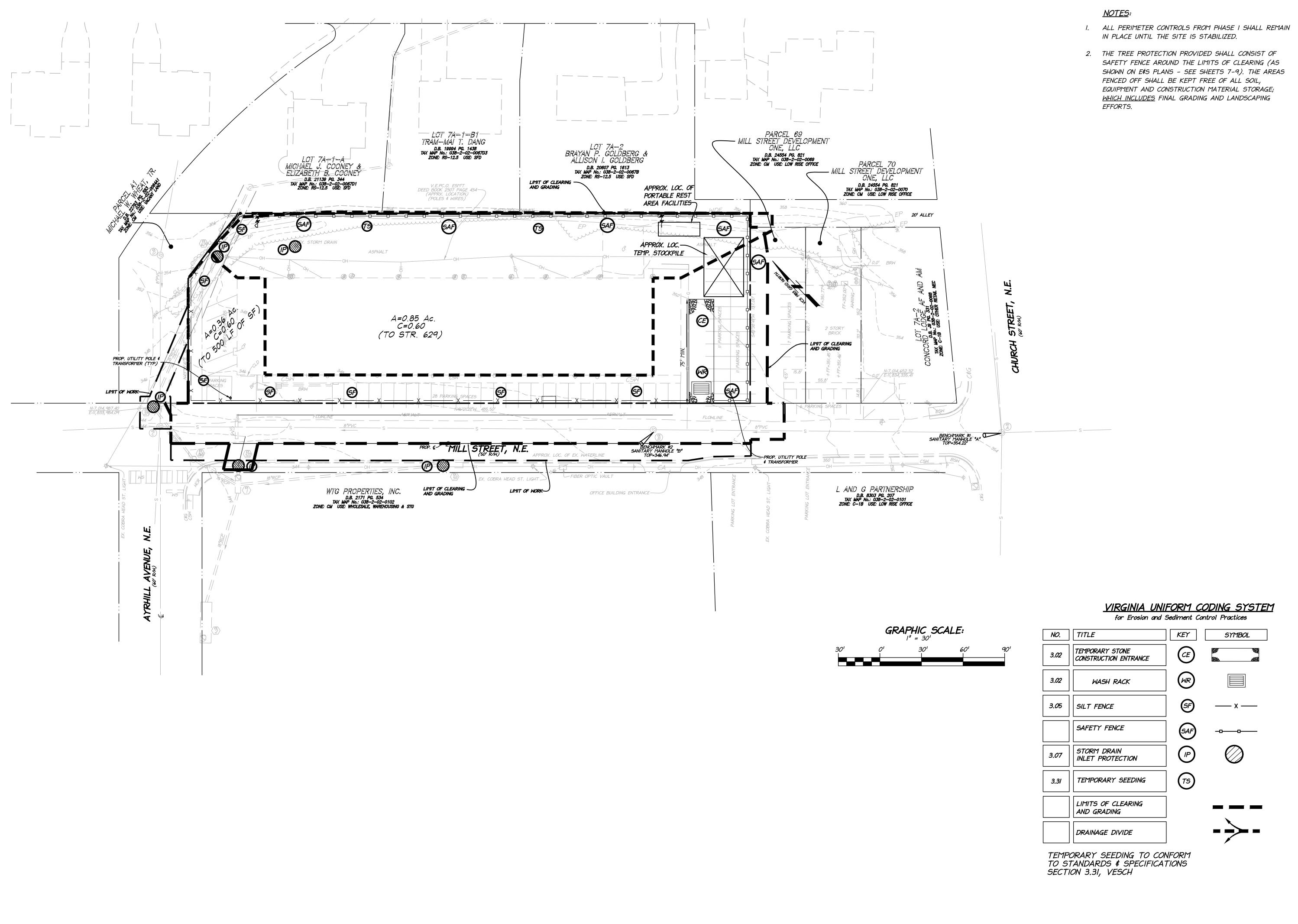
MILL STREET SELF-STORAGE

APPROVED BY:

JOSHUA MARSHALL
Lic. No. 040958
05/12/2020

SCALE: AS SHOWN

SHEET 6 OF 25 DATE: AUGUST 2, 2019

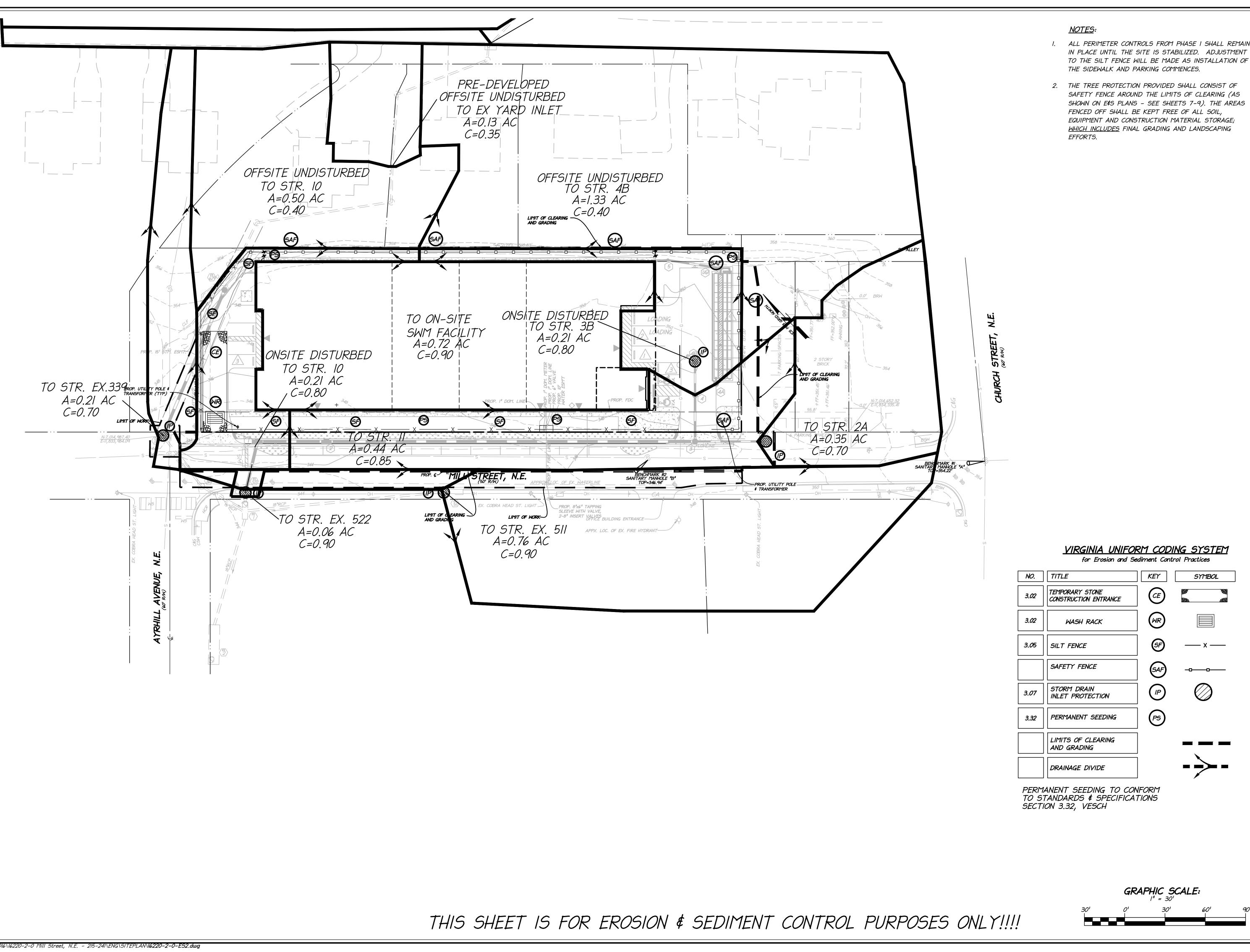


THIS SHEET IS FOR EROSION & SEDIMENT CONTROL PURPOSES ONLY!!!!

P:\PY 2016\16220-2-0 Mill Street, N.E. - 215-241\ENG\SITEPLAN\16220-2-0-ESI.dwg

I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.

of 25



I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.

# SHEET 8 OF 25

### EROSION AND SEDIMENT CONTROL NARRATIVE

### PROJECT DESCRIPTION

THE PROPOSED SITEPLAN, APPROX. 1.26 ACRES, SHALL PROVIDE FOR THE CONSTRUCTION OF ONE SELF STORAGE BUILDING. ALL EXISTING, ON-SITE STRUCTURES SHALL BE RAZED. THE SCOPE OF THIS PROJECT SHALL INCLUDE CONSTRUCTION OF PRIVATE PARKING AND DRIVE AISLE, MILLING AND OVERLAY, UNDERGROUND SWM FACILITY, STORM SEWER, SANITARY LATERAL AND WATERLINE. THE TOTAL DISTURBED AREA APPROXIMATELY ±1.7 ACRES.

### EXISTING SITE CONDITIONS:

THE SITE AREA CONSISTS OF AN OVERGROWN WOOD FENCE ALONG THE NORTHEAST SIDE OF THE SITE, AND AN EXISTING I STORY BRICK BUILDING WITH TYPICAL PHYSICAL IMPROVEMENTS. THERE ARE STEEP SLOPES ON SHARED PROPERTY LINE AT THE NORTH EAST.

### ADJACENT PROPERTIES:

### NORTHERN BOUNDARY:

THE NORTH SIDE OF THE SUBJECT PROPERTY IS BOUND BY A DRIVE AISLE TO A SHARED PARKING LOT FOR RETAIL USE.

### EASTERN BOUNDARY:

EASTERN SIDE OF THE SUBJECT PROPERTY IS BOUND BY RESIDENTIAL SINGLE FAMILY HOMES.

### **SOUTHERN BOUNDARY:**

SOUTHERN SIDE OF THE SUBJECT PROPERTY IS BOUND BY A SHARED PARKING LOT FOR RETAIL USE.

### **WESTERN BOUNDARY:**

WESTERN SIDE OF THE SUBJECT PROPERTY IS BOUND BY MILL STREET, N.E..

### EROSION CONTROL PROGRAM:

THE AREAS TO BE CLEARED SHALL BE DESIGNATED IN ACCORDANCE WITH THE APPROVED PLAN. PRIOR TO THE COMMENCEMENT OF THE ASSOCIATED CLEARING AND GRADING, THE SITE INSPECTOR MUST BE NOTIFIED AT LEAST TWO DAYS IN ADVANCE. PERMANENT OR TEMPORARY SOIL STABILIZATION SHALL BE APPLIED TO DENUDED AREAS WITHIN SEVEN DAYS AFTER THE FINAL GRADE IS ESTABLISHED ON ANY PORTION OF THE SITE. TEMPORARY SOIL STABILIZATION SHALL BE APPLIED WITHIN SEVEN (7) DAYS FOR AREAS THAT WILL REMAIN DENUDED FOR SEVEN (7) DAÝS OR GREATER. PERMANENT STABILIZATION SHALL BE APPLIED TO AREAS THAT WILL BE DORMANT FOR GREATER THAN SEVEN (7) DAYS ONCE FINAL GRADE IS ESTABLISHED. ALL REVEGETATION SHALL BÈ ACCOMPLISHED IMMEDIATELY. MAXIMUM SLOPES OF 2:1 SHALL BE USED FOR FOR GRADING PURPOSES.

### CRITICAL EROSION AREAS

THE STEEP SLOPES ALONG THE NORTHEAST SIDE OF THE PROPERTY ARE THE ONLY CRITICAL EROSION AREAS ON THIS SUBJECT SITE.

### <u>EROSION AND SEDIMENT CONTROL MEASURES</u>

UNLESS OTHERWISE INDICATED, ALL VEGETATIVE AND STRUCTURAL EROSION AND SEDIMENT CONTROL PRACTICES SHALL BE CONSTRUCTED AND MAINTAINED ACCORDING TO MINIMUM STANDARDS AND SPECIFICATIONS OF THE HANDBOOK. THE MINIMUM STANDARDS OF THE VESCH SHALL BE ADHERED TO UNLESS OTHERWISE WAIVED OR APPROVED BY A VARIANCE.

### STRUCTURAL PRACTICES

SILT FENCE & SUPER SILT FENCE BARRIERS - 3.05 SILT FENCE SEDIMENT BARRIERS WILL BE INSTALLED DOWNSLOPE OF AREAS WITH MINIMAL GRADES TO FILTER SEDIMENT-LADEN RUNOFF FROM SHEET FLOW AS INDICATED ON SHEETS 4 AND 5. SUPER SILT FENCE SHALL ALSO PROVIDE TREE PROTECTION IN AREAS WHERE IT IS ADJACENT TO VEGETATION TO BE

### PRESERVED. 2. TEMPORARY CONSTRUCTION ENTRANCE - 3.02

A TEMPORARY CONSTRUCTION ENTRANCE WITH A WASH RACK SHALL BE INSTALLED WHERE THE ACCESS AREA INTERSECTS WITH CENTER STREET. DURING MUDDY CONDITIONS, DRIVERS OF CONSTRUCTION VEHICLES WILL BE REQUIRED TO WASH THEIR WHEĖLS BEFORE ENTERING THE HIGHWAY.

# 3. STORM DRAIN INLET PROTECTION - 3.07

ALL STORM SEWER INLETS SHALL BE PROTECTED DURING CONSTRUCTION. SEDIMENT-LADEN WATER SHALL BE FILTERED BEFORE ENTERING THE STORM SEWER

### 4. TREE PROTECTION - 3.38

TREE PROTECTION WILL BE INSTALLED ADJACENT TO AREAS OF VEGETAION TO BE PRESERVED. SUPER SILT FENCE WILL ALSO ACT AS TREE PROTECTION WHERE INSTALLED ADJACENT TO AREAS OF VEGETATION TO BE PRESERVED.

### VEGETATIVE PRACTICES

TOPSOILING (STOCKPILE) - 3.30

TOPSOIL WILL BE STRIPPED FROM AREAS TO BE GRADED AND STOCKPILED FOR LATER USE. STOCKPILE LOCATIONS SHALL BE LOCATED ON-SITE AND ARE TO BE STABILIZED WITH TEMPORARY VEGETATION.

### 2. TEMPORARY SEEDING - 3.31

ALL DENUDED AREAS WHICH WILL BE LEFT DORMANT FOR EXTENDED PERIODS OF TIME SHALL BE SEEDED WITH FAST GERMINATING TEMPORARY VEGETATION IMMEDIATELY FOLLOWING GRADING. SELECTION OF THE SEED MIXTURE WILL DEPEND ON THE TIME OF YEAR IT IS APPLIED. SEE TABLES 3.3IB \$ 3.3IC ON THIS SHEET. 3. PERMANENT SEEDING - 3.32

ESTABLISHMENT OF PERENNIAL VEGETATIVE COVER BY PLANTING SEED ON ROUGH GRADED AREAS THAT WILL NOT BE BROUGHT TO FINAL GRADE FOR A YEAR OR MORE OR WHERE PERMANENT, LONG LIVED VEGETATIVE COVER IS NEEDED ON FINE GRADED AREAS.

4. EROSION CONTROL BLANKETS - 3.36 OR MULCH - 3.35

EROSION CONTROL BLANKETS WILL BE INSTALLED OVER FILL SLOPES WHICH HAVE BEEN BROUGHT TO FINAL GRADE AND HAVE BEEN SEEDED TO PROTECT THE SLOPES FROM RILL AND GULLY EROSION AND TO ALLOW SEED TO GERMINATE PROPERLY. MULCH (STRAW OR FIBER) WILL BE USED ON RELATIVELY FLAT AREAS AND WILL BE APPLIED AS A SECOND STEP IN THE SEEDING OPERATION.

### MANAGEMENT STRATEGIES

WILL BE CLEANED UP AND REMOVED.

CONSTRUCTION WILL BE SEQUENCED SO THAT GRADING OPERATIONS CAN BEGIN AND END AS QUICKLY AS POSSIBLE.

2. SEDIMENT TRAPPING MEASURES WILL BE INSTALLED AS A FIRST STEP IN GRADING AND WILL BE SEEDED AND MULCHED IMMEDIATELY FOLLOWING INSTALLATION.

3. TEMPORARY SEEDING AND OTHER STABILIZATION WILL FOLLOW IMMEDIATELY AFTER GRADING.

4. AREAS WHICH ARE NOT TO BE DISTURBED WILL BE CLEARLY MARKED BY FLAGS, SIGNS, ETC.

5. THE JOB SUPERINTENDENT SHALL BE RESPONSIBLE FOR THE INSTALLATION AND MAINTENANCE OF ALL EROSION AND SEDIMENT CONTROL PRACTICES. 6. AFTER ACHIEVING ADEQUATE STABILIZATION, THE TEMPORARY EAS CONTROLS

### TREE PRESERVATION NOTE

THE PROTECTION PROVIDED SHALL CONSIST OF FENCING OR OTHER APPROVED MEASURES AROUND THE TREE TO THE DRIP LINE AS DEPICTED ON THE PLAN AND AREAS ENCLOSED KEPT FREE OF ALL SOIL, EQUIPMENT AND CONSTRUCTION MATERIAL STORAGE; WHICH INCLUDES FINAL GRADING AND LANDSCAPING EFFORTS. NO EQUIPMENT SHALL BE ALLOWED ENTRY INTO THE FENCED ROOT ZONE AREA AT ANY TIME. THE AREA FENCED SHALL BE MULCHED WITH TWO INCHES OF SUITABLE MATERIAL AND THE TREES SHALL BE WATERED WITH ONE INCH OF WATER PER WEEK DURING THE GROWING SEASON IN ABSENCE OF ADEQUATE RAINFALL.

### NARRATIVE:

THE CONTROL PROGRAM ESTABLISHED HEREIN SHALL BE COMPLETED IN TWO PHASES.

<u>PHASE I</u> THE FIRST PHASE SHALL COMMENCE WITH THE INSTALLATION OF A CONSTRUCTION ENTRANCE AT MILL STREET. THE WASH RACK SHALL BE SUPPLIED BY A WATER TRUCK IN THE EVENT THAT AN EXISTING HYDRANT IS NOT AVAILABLE. SUBSEQUENTLY, THE PROPOSED PERIMETER CONTROLS (SILT FENCE, SUPER SILT FENCE AND SAFETY FENCING) AND INLET PROTECTION SHALL BE INSTALLED AS DEPICTED ON PHASE I PLANS. PERIMETER CONTROLS SHALL BE MAINTAINED AND UTILIZED DURING ALL PHASES OF CONSTRUCTION TO THE GREATEST EXTENT FEASIBLE. ONCE SAID CONTROLS ARE IN PLACE AN INSPECTION SHOULD BE REQUESTED FROM THE INSPECTOR. UPON APPROVAL OF SAID CONTROLS ROUGH GRADING OF THE SITE MAY COMMENCE. PLEASE NOTE THAT THE ESS PLAN PROVIDED HEREIN IS ONLY A GUIDE AND SHOULD BE UTILIZED AS SUCH. SHOULD DIFFERENT SITE CONDITIONS, WEATHER CONDITIONS OR OTHER UNFORESEEN CIRCUMSTANCES EXIST AT THE TIME OF CONSTRUCTION A MORE APPROPRIATE PLAN SHOULD BE IMPLEMENTED PROVIDED THE DEVIATIONS FROM THE ATTACHED PLAN ARE ACCEPTABLE TO THE SITE INSPECTOR. UPON COMPLETION AND INSTALLATION OF THE CONTROLS DEEMED APPROPRIATE FOR THE SITE CONDITIONS, PHASE 2 OF THE PROGRAM SHOULD COMMENCE.

### PHASE II

THE INSTALLATION OF PERIMETER CONTROLS AND THE APPROVAL BY THE SITE INSPECTOR SHALL PERMIT THE PHASE II PROGRAM TO COMMENCE. THE SITE GRADING, UTILITY AND ROAD CONSTRUCTION SHALL COMMENCE AND SLOPES STABILIZED AS NECESSARY. INLET PROTECTION SHALL BE INSTALLED ON ALL INLETS ONCE CONSTRUCTED. ALL FILL SLOPES SHALL REMAIN IN A ROUGHENED CONDITION TO REDUCE SHEET FLOWS AND THE RILL EROSION OF THE SLOPE.

ALL CRITICAL SLOPES SHALL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY UPON THE ESTABLISHMENT OF THE FINAL GRADE. IF NECESSARY EROSION CONTROL BLANKETS WILL BE INSTALLED AT FINAL GRADE AND SEEDED TO PROTECT THE SLOPES FROM RILL AND GULLY EROSION TO ALLOW THE GRASS SEED TO GERMINATE PROPERLY. SEEDED AREAS MUST BE CHECKED REGULARLY TO ENSURE THAT A GOOD STAND IS MAINTAINED. AREAS MUST BE FERTILIZED AND RESEEDED AS NECESSARY.

THE CONSTRUCTION ENTRANCE AND WASH RACK SHALL CONTINUE TO BE UTILIZED TO ASSIST IN THE REMOVAL OF SEDIMENT. ALL SIGNAGE TO REMAIN UNTIL CONSTRUCTION HAS BEEN COMPLETED.

ALL PERIMETER CONTROLS SHALL REMAIN UNTIL ALL ON-SITE MATERIALS ARE STABILIZED.

### PERMANENT STABILIZATION

ALL AREAS DISTURBED BY CONSTRUCTION SHALL BE STABILIZED WITH PERMANENT SEEDING IMMEDIATELY FOLLOWING FINISH GRADING. SEEDING SHALL BE DONE WITH KENTUCKY 31 TALL FESCUE ACCORDING TO STD. & SPEC. 3.32, 'PERMANENT SEEDING', OF THE HANDBOOK. EROSION CONTROL BLANKETS WILL BE INSTALLED OVER FILL SLOPES WHICH HAVE BEEN BROUGHT TO FINAL GRADE AND HAVE BEEN SEEDED TO PROTECT THE SLOPES FROM RILL AND GULLY EROSION AND TO ALLOW SEED TO GERMINATE PROPERLY. MULCH (STRAW OR FIBER) WILL BE USED ON RELATIVELY FLAT AREAS. IN ALL SEEDING OPERATIONS, SEED, FERTILIZER AND LIME WILL BE APPLIED PRIOR TO MULCHING.

### <u>STORMWATER MANAGEMENT</u>

CALCULATION OF RUNOFF BEFORE AND AFTER DEVELOPMENT INDICATES THAT THERE WILL BE A NET INCREASE IN PEAK RUNOFF AS A RESULT OF PROJECT DEVELOPMENT. CONSEQUENTLY, UNDERGROUND STORM WATER MANAGEMENT FACILITY SHALL BE PROVIDED TO ATTENUATE THE I, 2 \$ 10 YEAR RAINFALL EVENTS.

### <u>MAINTENANCE</u>

IN GENERAL, ALL EROSION AND SEDIMENT CONTROL MEASURES WILL BE CHECKED DAILY AND AFTER EACH SIGNIFICANT RAINFALL. THE FOLLOWING ITEMS WILL BE CHECKED IN PARTICULAR:

I. THE GRAVEL OUTLETS WILL BE CHECKED REGULARLY FOR SEDIMENT BUILDUP WHICH WILL PREVENT DRAINAGE. IF THE GRAVEL IS CLOGGED BY SEDIMENT, IT SHALL BE REMOVED AND CLEANED OR REPLACED.

2. THE SILT FENCE BARRIER WILL BE CHECKED REGULARLY FOR UNDERMINING OR DETERIORATION OF THE FABRIC. SEDIMENT SHALL BE REMOVED WHEN THE LEVEL OF SEDIMENT DEPOSITION REACHES HALF WAY TO THE TOP OF THE BARRIER.

3. THE SEEDED AREAS WILL BE CHECKED REGULARLY TO ENSURE THAT A GOOD STAND IS MAINTAINED. AREAS SHOULD BE FERTILIZED AND RESEEDED AS NECESSARY.

### GENERAL LAND CONSERVATION NOTES

I. NO DISTURBED AREA WHICH IS NOT ACTIVELY BEING WORKED SHALL REMAIN DENUDED FOR MORE THAN SEVEN CALENDAR DAYS UNLESS OTHERWISE AUTHORIZED BY THE DIRECTOR.

2. ALL E\$S CONTROL MEASURES APPROVED WITH THE PHASE I E\$S CONTROL PLAN SHALL BE PLACED AS THE FIRST STEP IN GRADING 3. ALL STORM AND SANITARY SEWER LINES NOT IN STREETS SHALL BE

SEEDED AND MULCHED WITHIN SEVEN DAYS AFTER BACKFILL. NO MORE THAN 500' SHALL BE OPEN AT ANY TIME. 4. ELECTRIC POWER, TELEPHONE AND GAS SUPPLY TRENCHES SHALL BE

COMPACTED, SEEDED AND MULCHED WITHIN SEVEN DAYS AFTER BACKFILL. 5. ALL TEMPORARY EARTH BERMS, DIVERSIONS AND SEDIMENT CONTROL DAMS SHALL BE SEEDED AND MULCHED FOR TEMPORARY VEGETATIVE COVER IMMEDIATELY (AS SOON AS POSSIBLE BUT NO LATER THAN 48 HR.) AFTER COMPLETION OF GRADING. STRAW OR HAY MULCH IS REQUIRED. ALL SOIL STOCKPILES SHALL BE SEEDED AND MULCHED WITHIN SEVEN DAYS AFTER GRADING

6. DURING CONSTRUCTION, ALL STORM SEWER INLETS SHALL BE PROTECTED BY SEDIMENT TRAPS, MAINTAINED AND MODIFIED DURING CONSTRUCTION PROGRESS AS REQUIRED.

7. ANY DISTURBED AREA NOT COVERED BY #I AND NOT PAVED, SODDED OR BUILT UPON BY NOVEMBER I, OR DISTURBED AFTER THAT DATE, SHALL BE MULCHED IMMEDIATELY WITH HAY OR STRAW MULCH AT THE RATE OF 2 TONS/ACRE AND OVERSEEDED BY APRIL 15.

8. AT THE COMPLETION OF ANY PROJECT CONSTRUCTION AND PRIOR TO BOND RELEASE, ALL TEMPORARY SEDIMENT CONTROLS SHALL BE REMOVED AND ALL DENUDED AREAS SHALL BE STABILIZED.

### **TABLE 3.32-D** SITE SPECIFIC SEEDING MIXTURES FOR PIEDMONT AREA Total Lbs. Per Acre

Minimum Care Lawn 175-200 lbs Commercial or Residential - Kentucky 31 or Turf-Type Tall Fescue 95-100% 0-5% - Improved Perennial Ryegrass Kentucky Bluegrass 0-5%

200-250 lbs.

100%

20 lbs

150 lbs

108 lbs

High-Maintenance Lawn - Kentucky 31 or Turf-Type Tall Fescue

General Slope (3:1 or less) 128 lbs Kentucky 31 Fescue - Red Top Grass 2 lbs

### Low-Maintenance Slope (Steeper than 3:1)

Seasonal Nurse Crop \*

Kentucky 31 Fescue

Red Top Grass 2 lbs Seasonal Nurse Crop \* 20 lbs. 20 lbs. - Crownvetch \*\*

February 16th through April ..... Annual Rye

May 1st through August 15th ..... Foxtail Millet

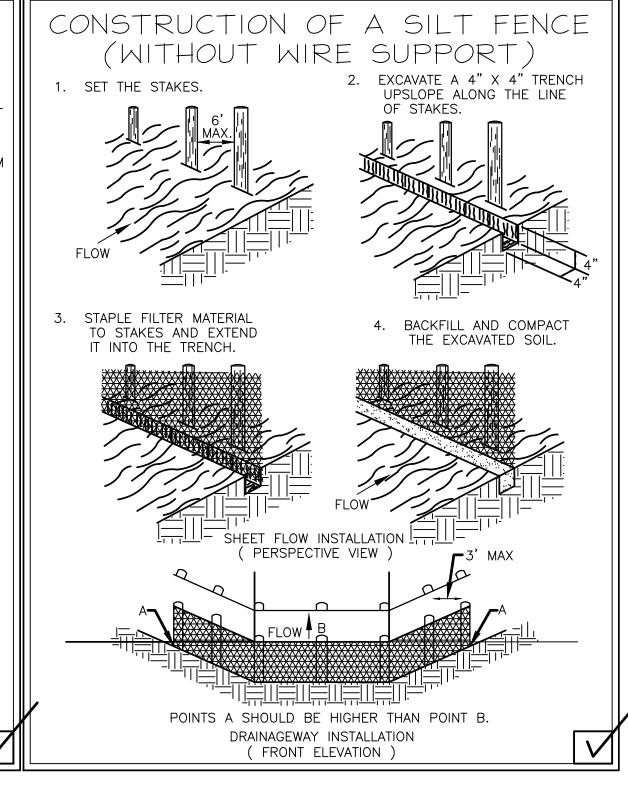
August 16th through October ..... Annual Rye

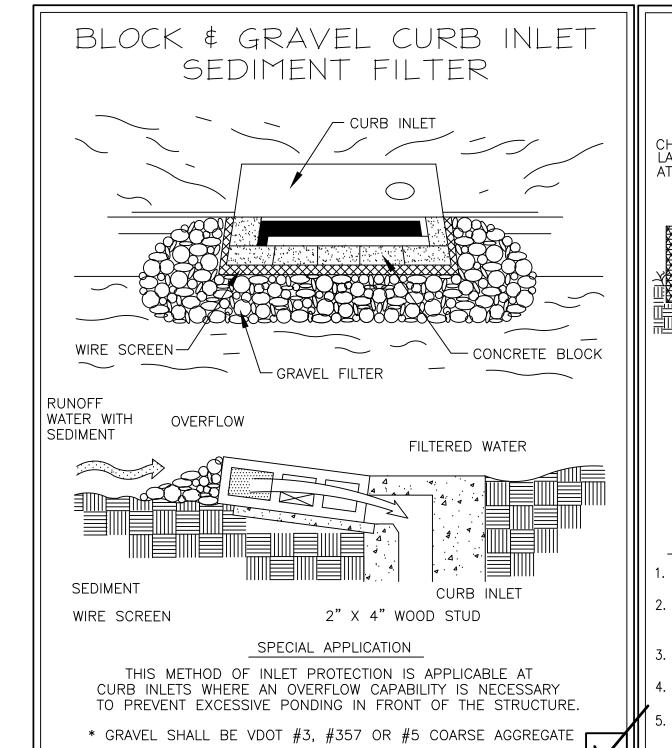
November through February 15th . . . . . . . . . . . Winter Rye

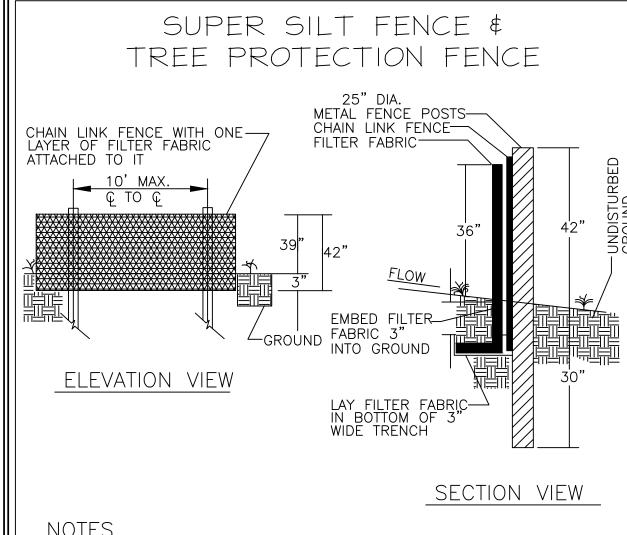
\*\* Substitute Sericea lespedeza for Crownvetch east of Farmville, Va. (May through September use hulled Sericea, all other periods, use unhulled Sericea). If Flatpea is used in lieu of Crownvetch, increase rate to 30 lbs./acre. All legume seed must be properly inoculated. Weeping Lovegrass may be added to any slope or low-maintenance mix during warmer seeding periods; add 10-20 lbs./acre in

\* Use seasonal nurse crop in accordance with seeding dates as stated below:

STONE CONSTRUCTION ENTRANCE 1. SET THE STAKES. PAVEMENT MOUNTABLE BERM (OPTIONAL) SIDE ELEVATION - EXISTING GROUND - WASHRACK AVEMENT COURSE AGGREGATE TO SEDIMENT TRAPPING DEVICE MUST EXTEND FULL WIDTH OF INGRESS AND EGRESS PLAN VIEW <del>---</del>3" мін. OPERATION 12' MIN. SECTION A-A FILTER CLOTH REINFORCED CONCRETE







CHAIN LINK FENCE SHALL BE FASTENED SECURELY TO FENCE POSTS WITH WIRE TIES.

. FILTER FABRIC SHALL BE FASTENED SECURELY TO CHAIN LINK FENCE WITH TIES SPACED HORIZONTALLY 24" AT THE TOP AND MIDSECTION.

3. PHYSICAL PROPERTIES OF THE FILTER FABRIC SHALL CONFORM TO THE LATEST EDITION OF THE VIRGINIA EROSION & SEDIMENT CONTROL HANDBOOK.

4. WHEN TWO SECTIONS OF FILTER FABRIC ADJOIN EACH OTHER, THEY SHALL BE OVERLAPPED BY 6". . MAINTENANCE SHALL BE PERFORMED AS NEEDED AND MATERIAL SHALL BE REMOVED WHEN SEDIMENT BUILD-UP REACHES 50% OF THE HEIGHT OF

### TABLE 3.31-C

### TEMPORARY SEEDING PLANT MATERIALS, SEEDING RATES, AND DATES

	SEEDING F	RATE	ľ	NORTH	a	S	OUTI	I <sup>b</sup>	PLANT
SPECIES	Асте	1000 ft <sup>2</sup>	3/1 to 4/30	5/1 to 8/15	8/15 to 11/1	2/15 to 4/30	5/1 to 9/1	9/1 to 11/15	CHARACTERISTICS
OATS (Avena sativa)	3 bu. (up to 100 lbs., not less than 50 lbs.)	2 lbs.	х	-	_	х	-	-	Use spring varieties (e.g., Noble).
RYE <sup>d</sup> (Secale cereale)	2 bu. (up to 110 lbs., not less than 50 lbs.)	2.5 lbs.	Х		Х	х	-	X	Use for late fall seedings, winter cover. Tolerates cold and low moisture.
GERMAN MILLET (Setaria italica)	50 lbs.	approx. 1 lb.	_	х	-	-	X	-	Warm-season annual. Dies at first frost. May be added to summer mixes.
ANNUAL RYEGRASS <sup>c</sup> (Lolium multi-florum)	60 lbs.	1½ lbs.	х	-	х	Х	-	Х	May be added in mixes. Will mow out of most stands.
WEEPING LOVEGRASS (Eragrostis curvula)	15 lbs.	51/2 ozs.	-	х	-	•	Х	-	Warm-season perennial. May bunch. Tolerates hot, dry slopes and acid, infertile soils. May be added to mixes.
KOREAN LESPEDEZA <sup>c</sup> (Lespedeza stipulacea)	25 lbs.	approx. 1½ lbs.	х	х	-	х	х	-	Warm season annual legume. Tolerates acid soils. May be added to mixes.

### **TABLE 3.31-B**

THE SUPER SILT FENCE.

### ACCEPTABLE TEMPORARY SEEDING PLANT MATERIALS

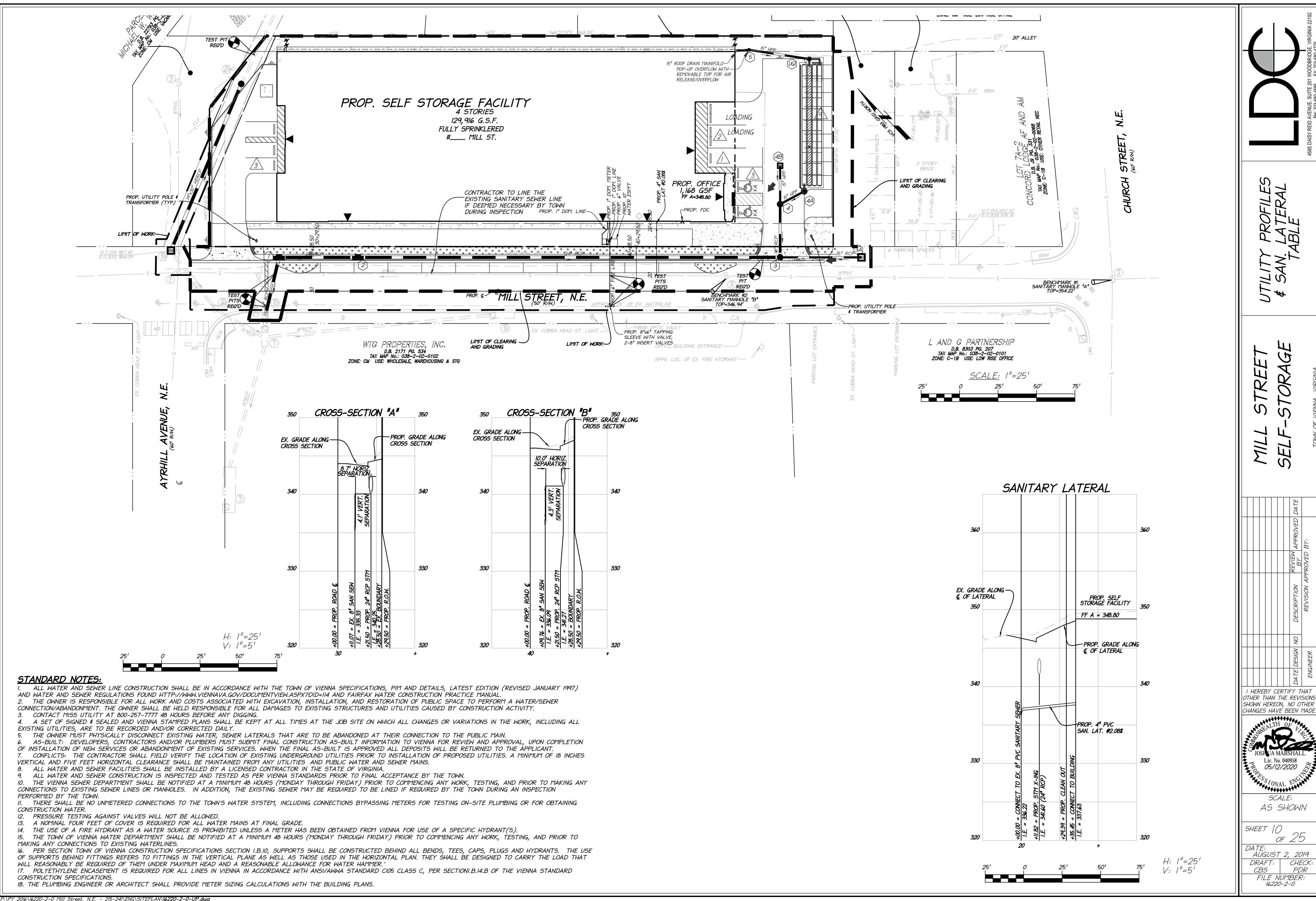
### MOTITOR DEPENDENCE FOR ALL DEGLAR

"QUIC	K REFERENCE FOR ALL REGIO	NS"
Planting Dates	<u>Species</u>	Rate (lbs./acre)
Sept. 1 - Feb. 15	50/50 Mix of Annual Ryegrass (Lolium multi-florum) & Cereal (Winter) Rye (Secale cereale)	50 - 100
Feb. 16 - Apr. 30	Annual Ryegrass (Lolium multi-florum)	60 - 100
May 1 - Aug 31	German Millet (Setaria italica)	50

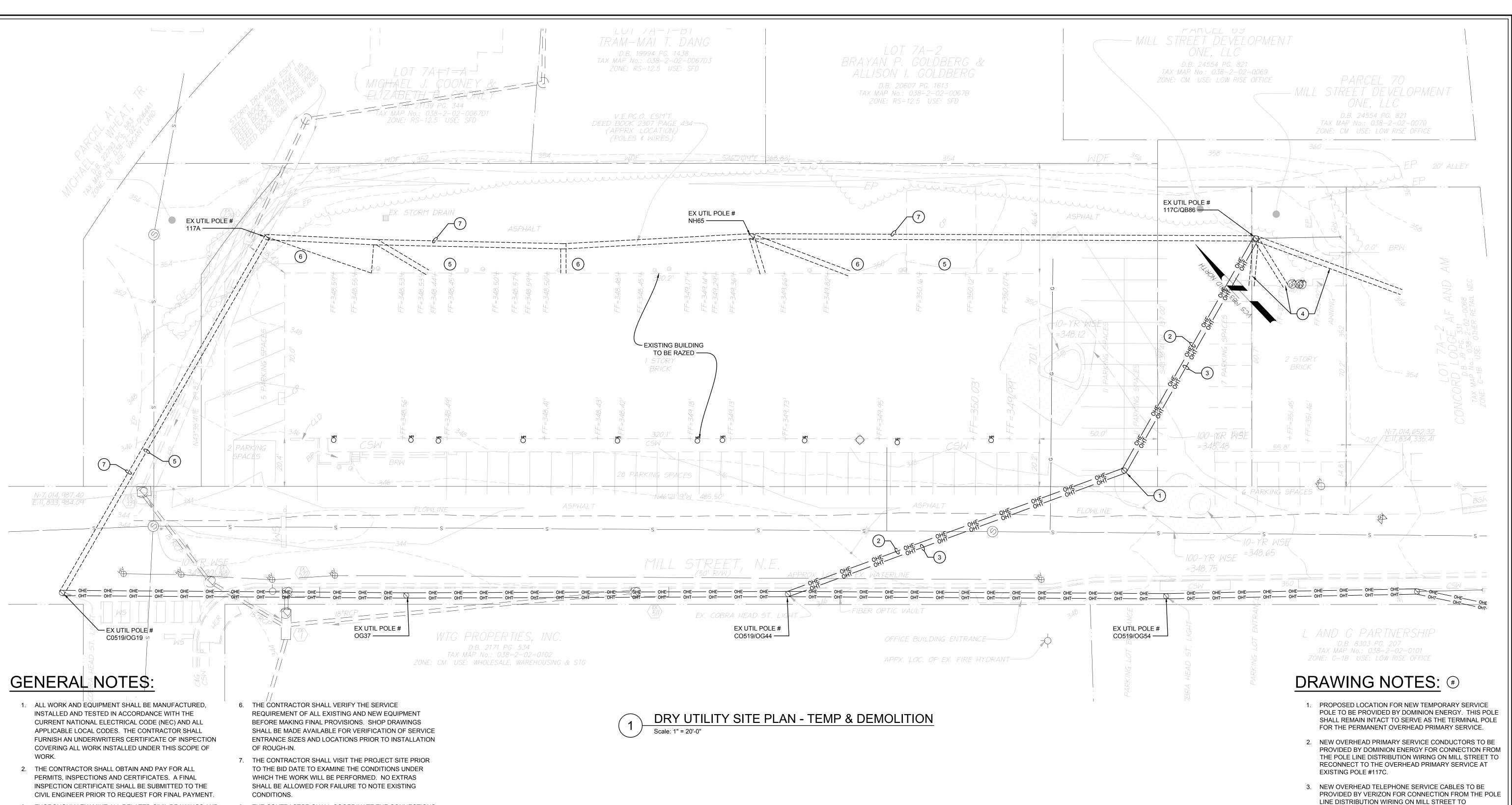
I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER

CHANGES HAVE BEEN MADE. Lic. No. 040958 *05/12/2020* \*\*\*\*\*

SCALE: SHEET of 25

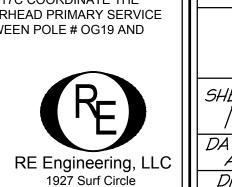


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- 3. THOROUGHLY EXAMINE ALL RELATED CIVIL DRAWINGS AND ARCHITECTURAL DRAWINGS OF THE PROPOSED BUILDING PRIOR TO THE COMMENCEMENT OF ANY WORK. COORDINATE ALL ELECTRICAL, TELECOMMUNICATIONS AND GAS SERVICE WORK WITH ALL OTHER SITE UTILITIES. COORDINATE THE TERMINATION LOCATIONS OF ALL UTILITY SERVICES WITH THE BASE BUILDING CONSTRUCTION DOCUMENTS.
- ALL UTILITY WORK SHALL BE PROVIDED IN ACCORDANCE WITH THE RULES, REGULATIONS AND STANDARDS OF THE LOCAL UTILITY COMPANIES INCLUDING BUT NOT LIMITED TO DOMINION ENERGY, VERIZON AND WASHINGTON GAS.
- 5. THESE PLANS AND DETAILS ARE INTENDED TO PROVIDE A BROAD OUTLINE OF THE WORK AND EQUIPMENT REQUIRED BUT ARE NOT INTENDED TO INCLUDE ALL DETAILS OF CONSTRUCTION.
- 8. THE CONTRACTOR SHALL COORDINATE THE CONNECTIONS TO EXISTING UTILITY SYSTEMS WITH THE OWNER AND OTHER FACILITIES AFFECTED BY THIS PROJECT. ALL OUTAGES SHALL BE CLOSELY COORDINATED WITH THE OWNER AND EXISTING FACILITY OPERATING SCHEDULES.
- 9. PROVIDE COMPLETE SHOP DRAWINGS FOR ALL NEW EQUIPMENT AND MATERIALS BEING PROVIDED FOR THE PROJECT. REFER TO THE CIVIL GENERAL NOTES FOR SHOP DRAWING SUBMITTAL PROCESS TO BE FOLLOWED.
- 10. UPON COMPLETION OF THE ROUGH-IN WORK FOR EACH UTILITY THE CONTRACTOR SHALL CONTACT THE RESPECTIVE UTILITY COMPANY REPRESENTATIVE AND OBTAIN APPROVAL OF THE WORK PRIOR TO PROCEEDING WITH CONCRETE ENCASEMENT, BACKFILL OR FURTHER INSTALLATION. ONLY AFTER ACCEPTANCE OF THE ROUGH-IN INSTALLATION SHALL THE WORK PROCEED.

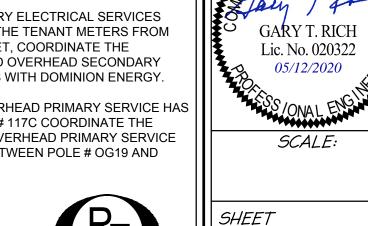
- RECONNECT TO THE OVERHEAD CABLES AT EXISTING POLE
- 4. ALL EXISTING ELECTRICAL SECONDARY SERVICE DROPS AND TELEPHONE CABLES SERVING 211 MILL STREET, 213 MILL STREET AND 146 CHURCH STREET SHALL REMAIN INTACT AND FUNCTIONAL.
- 5. AFTER ALL OF THE TELEPHONE SERVICES HAVE BEEN TERMINATED TO THE TENANTS FROM 223 THROUGH 241 MILL STREET, COORDINATE THE REMOVAL OF ALL ABANDONED OVERHEAD TELECOMMUNICATIONS WIRING FROM THE POLES AND CABLES BEHIND THE EXISTING BUILDING TO THE DEMARK LOCATIONS WITH VERIZON AND ANY OTHER TELECOMMUNICATIONS COMPANIES.
- 6. AFTER ALL OF THE SECONDARY ELECTRICAL SERVICES HAVE BEEN TERMINATED TO THE TENANT METERS FROM 223 THROUGH 241 MILL STREET, COORDINATE THE REMOVAL OF ALL ABANDONED OVERHEAD SECONDARY SERVICE DROP CONDUCTORS WITH DOMINION ENERGY.
- 7. AFTER THE TEMPORARY OVERHEAD PRIMARY SERVICE HAS BEEN ESTABLISHED AT POLE # 117C COORDINATE THE REMOVAL OF ALL EXISTING OVERHEAD PRIMARY SERVICE CONDUCTORS AND POLES BETWEEN POLE # OG19 AND POLE # 117C.



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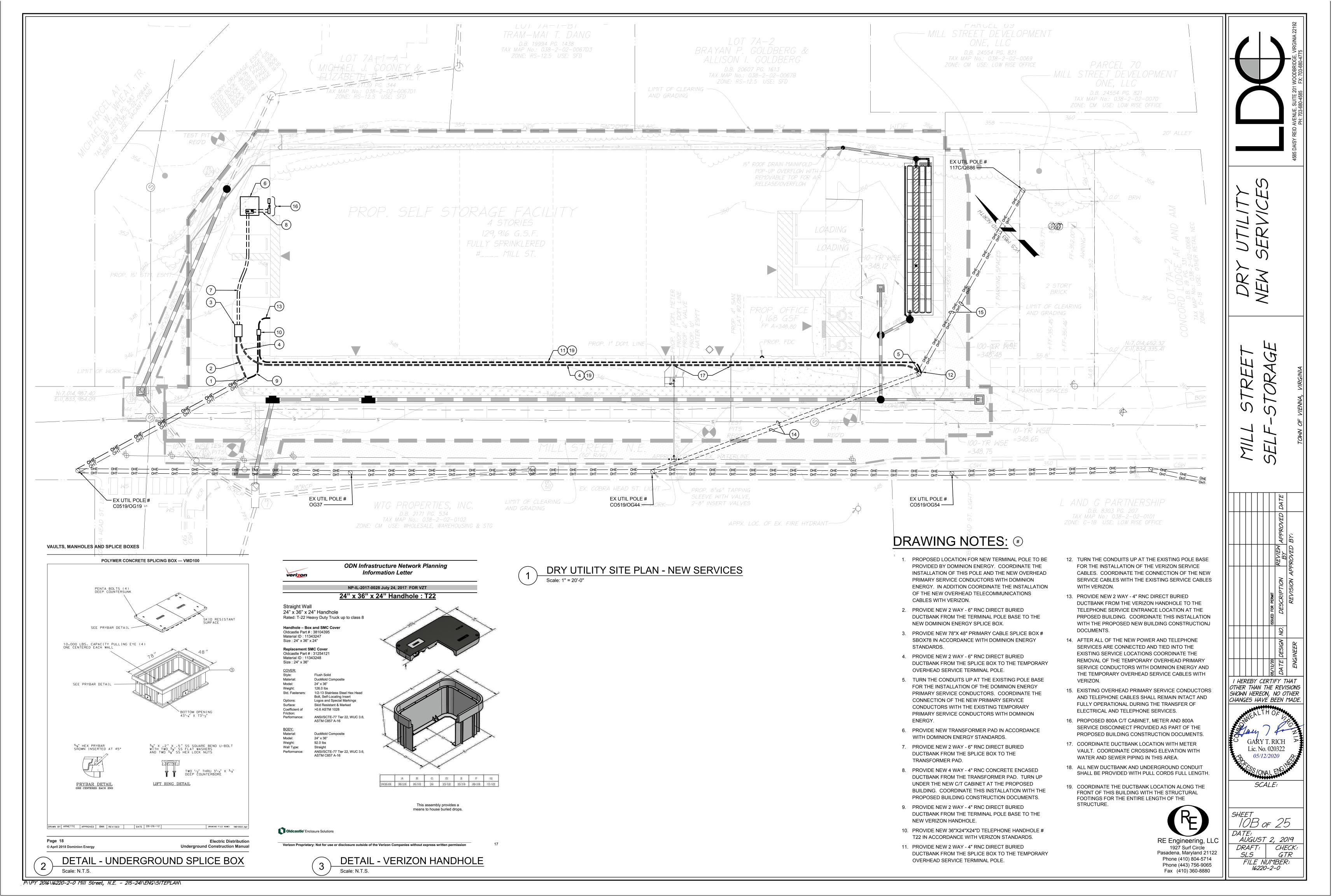
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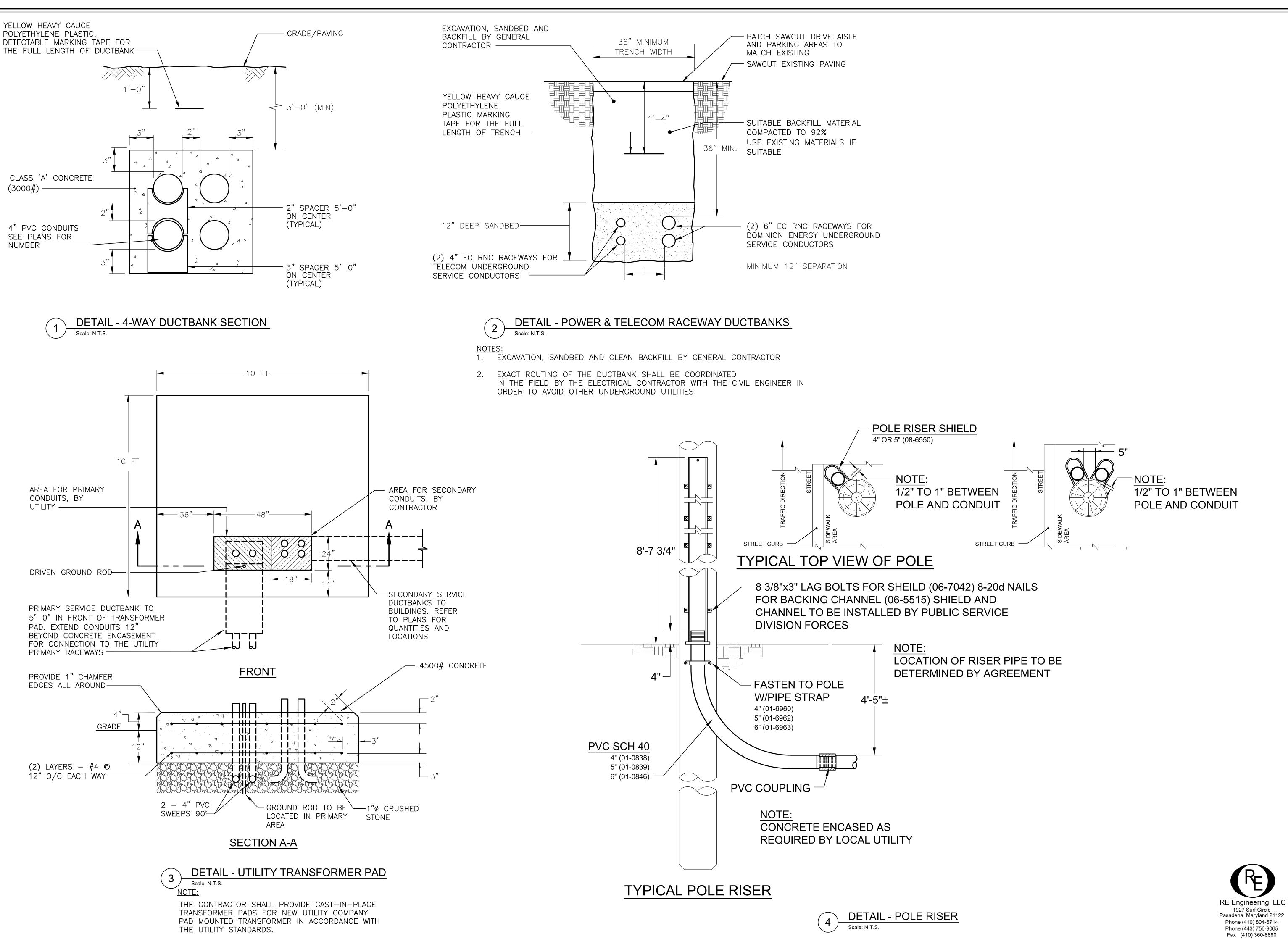
I HEREBY CERTIFY THAT

OTHER THAN THE REVISIONS

SHOWN HEREON, NO OTHER

CHANGES HAVE BEEN MADE.





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I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.

> GARY T. RICH Lic. No. 020322

05/12/2020

SCALE:

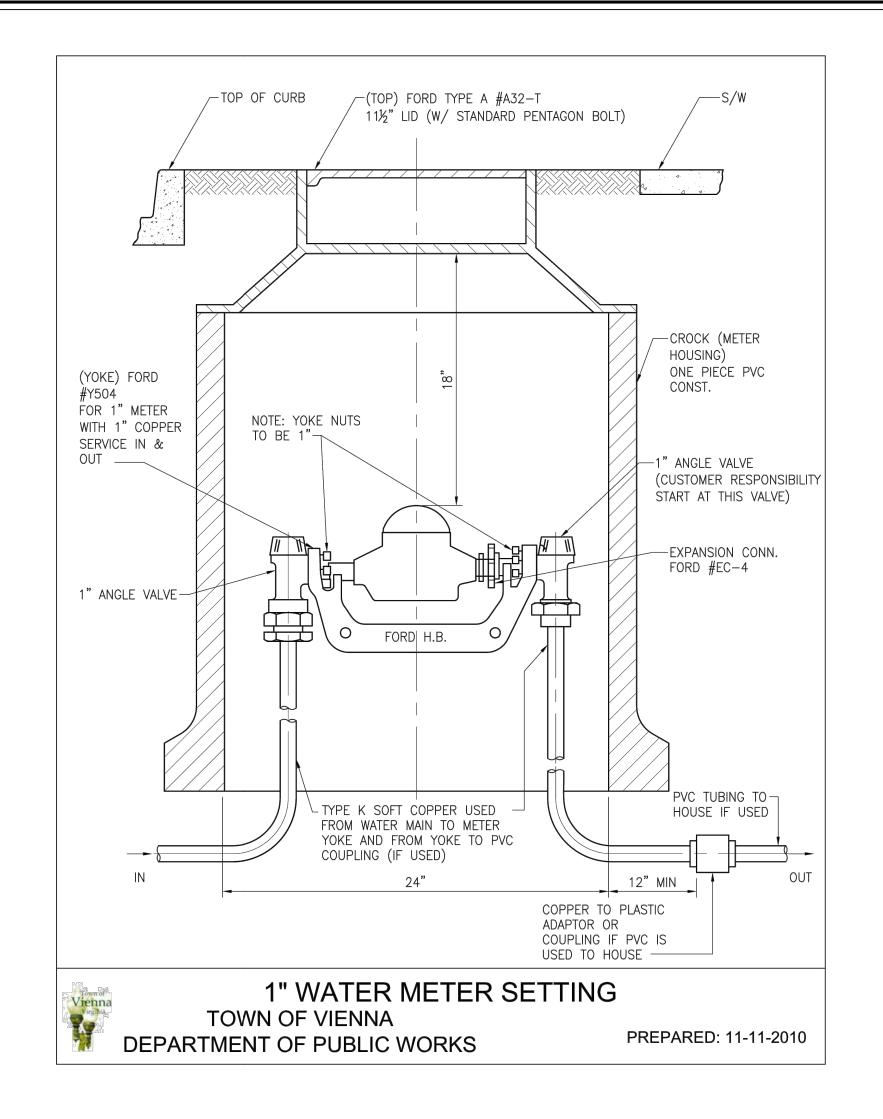
SHEET 10C of 25

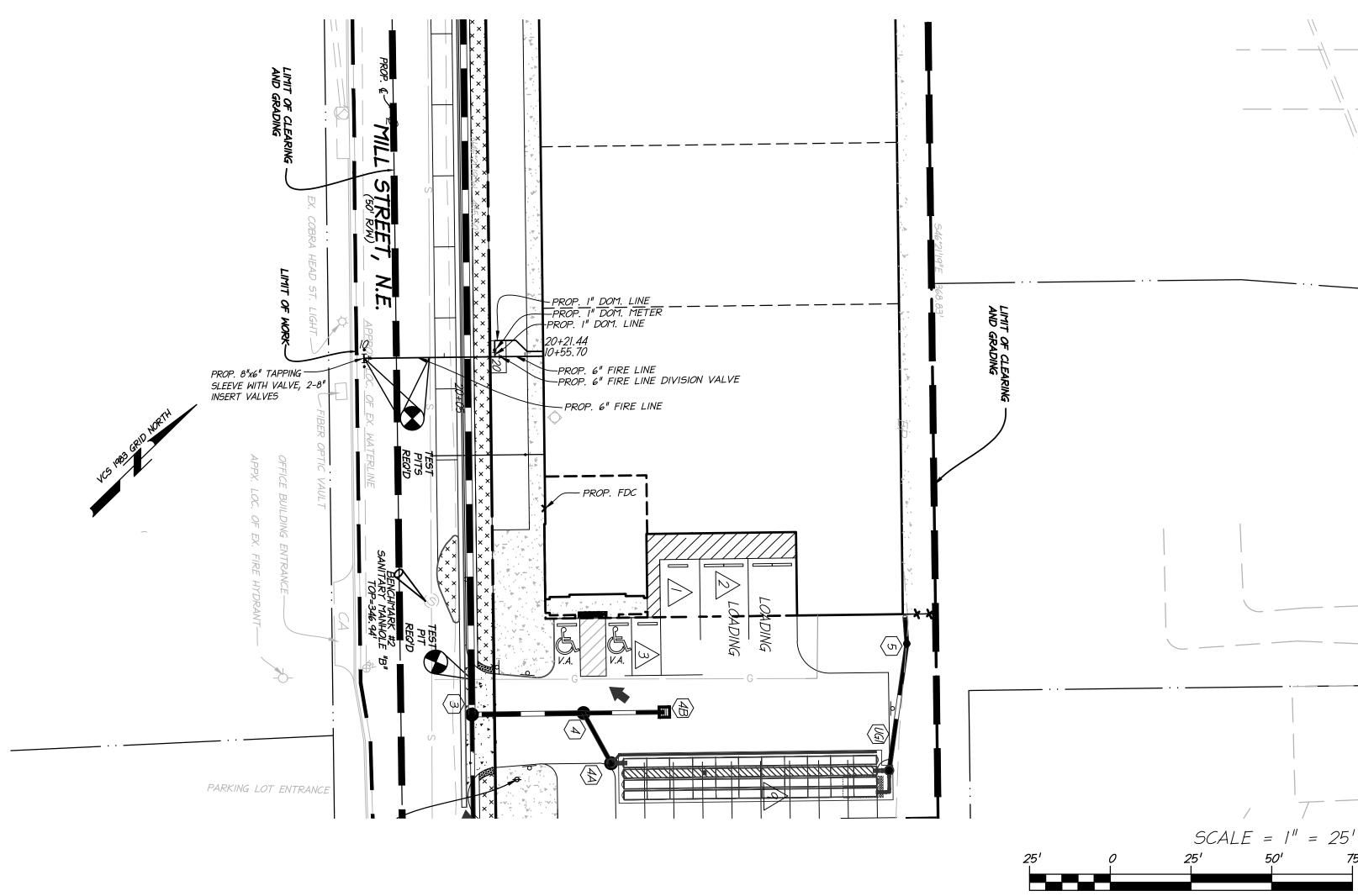
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### WATER SERVICE TABULATION SHEET

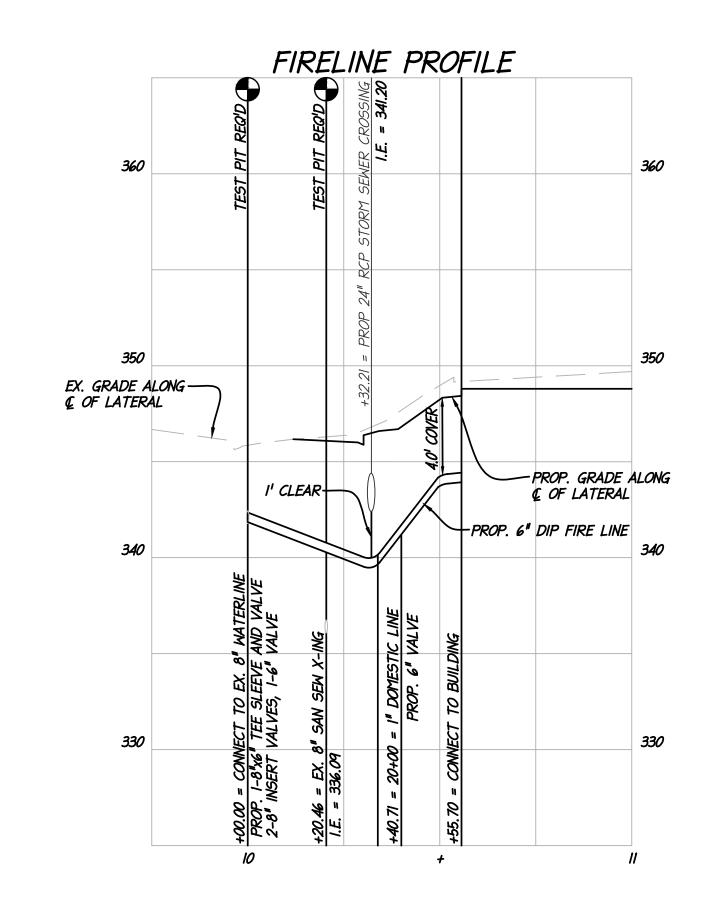
	Number		/alues, in		s	upply Fixture Unit	's
Name of Fixture	of		y Fixture				
	Fixtures (A)	Cold (B)	Hot (C)	Total (D)	Cold (B X A)	Hot (C X A)	Total (D X A)
Bathroom Group (Private, Flush Tank)	(~)	2.7	1.5	3.6	0.0	0.0	0.0
Bathroom Group (Private, Flush Valve)		6.0	3.0	8.0	0.0	0.0	0.0
Bathtub (Private)		1.0	1.0	1.4	0.0	0.0	0.0
Bathtub (Public)		3.0	3.0	4.0	0.0	0.0	0.0
Bidet (Private)		1.5	1.5	2.0	0.0	0.0	0.0
Combination Fixture (Private)		2.25	2.25	3.0	0.0	0.0	0.0
Dishwashing Machine (Private)		-	1.4	1.4	0.0	0.0	0.0
Drinking Fountain	1	0.25	-	0.25	0.3	0.0	0.3
Kitchen Sink (Private)		1.0	1.0	1.4	0.0	0.0	0.0
Kitchen Sink (Public)		3.0	3.0	4.0	0.0	0.0	0.0
Laundry Trays (1 to 3)		1.0	1.0	1.4	0.0	0.0	0.0
Lavatory (Private)		0.5	0.5	0.7	0.0	0.0	0.0
Lavatory (Public)	1	1.5	1.5	2.0	1.5	1.5	2.0
Service Sink	1	2.25	2.25	3.0	2.3	2.3	3.0
Hose bib	4	1	-	1.0	4.0		4.0
Shower Head (Public)		3.0	3.0	4.0	0.0	0.0	0.0
Shower Head (Private)		1.0	1.0	1.4	0.0	0.0	0.0
Urinal (Public, 1" Flush Valve)		10.0	-	10.0	0.0	0.0	0.0
Urinal (Public, ¾" Flush Valve)		5.0	-	5.0	0.0	0.0	0.0
Urinal (Public, Flush Tank)		3.0	-	3.0	0.0	0.0	0.0
Washing Machine (Private, 8lb)		1.0	1.0	1.4	0.0	0.0	0.0
Washing Machine (Public, 8lb)		2.25	2.25	3.0	0.0	0.0	0.0
Washing Machine (Public, 15lb)		3.0	3.0	4.0	0.0	0.0	0.0
Water Closet (Private, Flush Valve)		6.0	-	6.0	0.0	0.0	0.0
Water Closet (Private, Flush Tank)		2.2	-	2.2	0.0	0.0	0.0
Water Closet (Public, Flush Valve)	1	10.0	-	10.0	10.0	0.0	10.0
Water Closet (Public, Flush Tank)		5.0	-	5.0	0.0	0.0	0.0
Water Closet (Public or Private, Flushometer)		2.0	-	2.0	0.0	0.0	0.0
Total					18.0	3.8	19.3
Demand in GPM ———————————————————————————————————				<b>→</b>	33.4	0	34.2
Per Tables. Confirm type of closets.							
Sprinkler Demand in GPM							
(NFPA 13R Only)							
`							
Total Demand in GPM					33.4	0	34.2
					_		
Pipe Type (Copper K, Copper L or Smooth Steel)				<b>→</b> .			L
(K - Copper Type K, L - Copper Type L, Steel - Smoo	oth Steel)						
Dina siza (inchas)							1¼"
Pipe size (inches)  Refer to pipe sizing chart based on demand						-	1 74
Total to pipe orang entart based on definition							

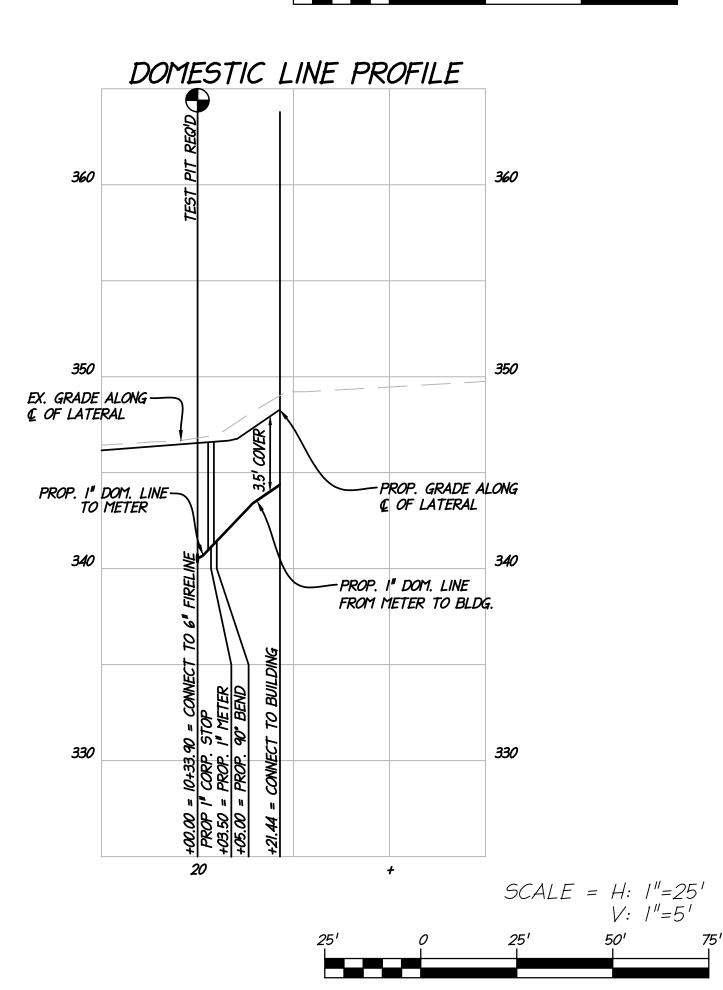
### TOWN OF VIENNA WATER MAIN CONSTRUCTION NOTES

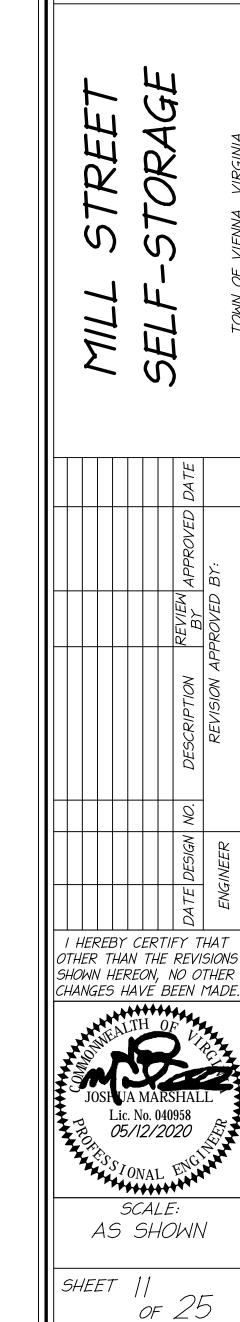
WATER MAIN CONSTRUCTION SHALL COMPLY WITH THE LATEST ISSUE OF THE TOWN OF VIENNA PUBLIC INFRASTRUCTURE MANUAL (PIM), VIRGINIA STATE WATERWORKS REGULATIONS, VDOT ROAD

# BRIDGE SPECIFICATIONS # STANDARDS, AND FAIRFAX COUNTY PUBLIC FACILITY MANUAL (PFM). 2. PRIOR TO COMMENCING THE WATER MAIN CONSTRUCTION, THE DEVELOPER/OWNER SHALL NOTIFY THE TOWN OF VIENNA, DEPARTMENT OF PUBLIC WORKS, WATER & SEWER DIVISION, *(703-255-6380).* 

- 3. THE DEVELOPER SHALL REQUEST INSPECTION BY THE PUBLIC WATER SUPPLY AGENCY THREE DAYS PRIOR TO COMMENCING CONSTRUCTION OF ANY WATER MAIN.
- 4. ALL BRANCH VALVES SHALL BE STRAPPED, AND ALL WATER MAINS SHALL HAVE A MINIMUM COVER OF FOUR (4) FEET, UNLESS OTHERWISE APPROVED BY THE TOWN OF VIENNA.
- 5. BACKFLOW PREVENTER IS ABSOLUTELY REQUIRED FOR TEMPORARY CONSTRUCTION, IRRIGATION SYSTEM OR VEHICLE WASH AREAS, AND WHERE AS REQUIRED BY THE STATE WATERWORKS REGULATIONS.
- 6. PRIOR TO DIGGING, NOTIFY MISS UTILITY @ I-800-552-7001 THREE (3) WORKING DAYS IN
- 7. ALL CONSTRUCTION MUST BE DONE IN COMPLIANCE WITH THE OCCUPATIONAL SAFETY AND HEALTH ACT (OSHA) OF 1970, AND ALL RULES AND REGULATIONS THERETO APPURTENANT.
- 8. THE DEVELOPER AGREES TO ACCEPT COMPLETE RESPONSIBILITY AND ALL COSTS FOR THE INSTALLATION OF WATER MAINS AND APPURTENANCES, INCLUDING ANY ADJUSTMENTS IN ALIGNMENT AND GRADE OR RELOCATION TO EXISTING WATER FACILITIES DUE TO THE DEVELOPMENT OF THIS PROPERTY, ANY REPAIR AND MAINTENANCE REQUIRED PRIOR TO FINISH GRADING AND SURFACING OF THE STREETS AND/OR EASEMENTS. FINAL ACCEPTANCE WILL NOT BE CONSIDERED OR GRANTED UNTIL AFTER THE STREETS HAVE BEEN SURFACED OR THE EASEMENTS FINALLY GRADED.
- 9. ALL WATER METERS SHALL BE PLACED BETWEEN THE BACK OF CURB AND PROPOSED SIDEWALK AND SHALL BE I" WATER METERS WITH 24" METER BOXES.
- 10. ALL SERVICE LINES ARE I" OFF WATER MAIN.
- II. ALL T'S AND VALVES NEED TO BE CONNECTED WITH MEGALUGS RETAINER GLANDS.
- 12. ALL T'S NEED BLOCKING.
- 13. BEFORE THE START OF CONSTRUCTION, THE DEVELOPER MUST PROVIDE THE FOLLOWING INFO AND/OR EVIDENCE OF COMPLIANCE WITH ALL APPLICABLE REGULATIONS AND LAWS: - IF ANY EASEMENTS ARE NEEDED, TWO COPIES OF THE RECORDED EASEMENT MUST BE PROVIDED, INCLUDING THE PLACE, DATE AND REFERENCE OF THE RECORDED EASEMENT.
- WRITTEN NOTICE OF THE TENTATIVE STARTING DATE OF CONSTRUCTION, WHICH MUST BE A MINIMUM OF ONE WEEK FOLLOWING THE DATE OF NOTICE, IS NEEDED. IN ADDITION, THE DEVELOPER MUST PROVIDE THE NAMES AND PHONE NUMBERS OF TWO EMERGENCY CONTACTS. 14. WATER VALVES SHALL ONLY BE OPERATED BY TOWN OF VIENNA PERSONNEL. THE CONTRACTOR
- IS REQUIRED TO PROVIDE 2 WORKING DAYS NOTICE OF ANY SHUT DOWN REQUIRED. WATER DISRUPTIONS TO EXISTING CUSTOMERS SHALL BE KEPT TO A MINIMUM WHICH MAY REQUIRE THE CONTRACTOR TO INSTALL INSERTING VALVES.
- 15. TOWN WORKING HOURS FOR WATER AND SEWER ARE MONDAY THRU FRIDAY (7:00 AM 3:30 PM) WITH THE EXCEPTION OF HOLIDAYS. ANY INSPECTIONS OR SHUT DOWNS OUTSIDE OF WORKING HOURS SHALL REQUIRE ADDITIONAL FEES.
- 16. ALL VALVE COVERS SHALL BE STAMPED WATER.
- 17. ALL FIRE HYDRANTS SHALL BE MUELLER.
- 18. DEVELOPER IS RESPONSIBLE FOR ALL ADJUSTMENTS DUE TO FIELD CONDITIONS.





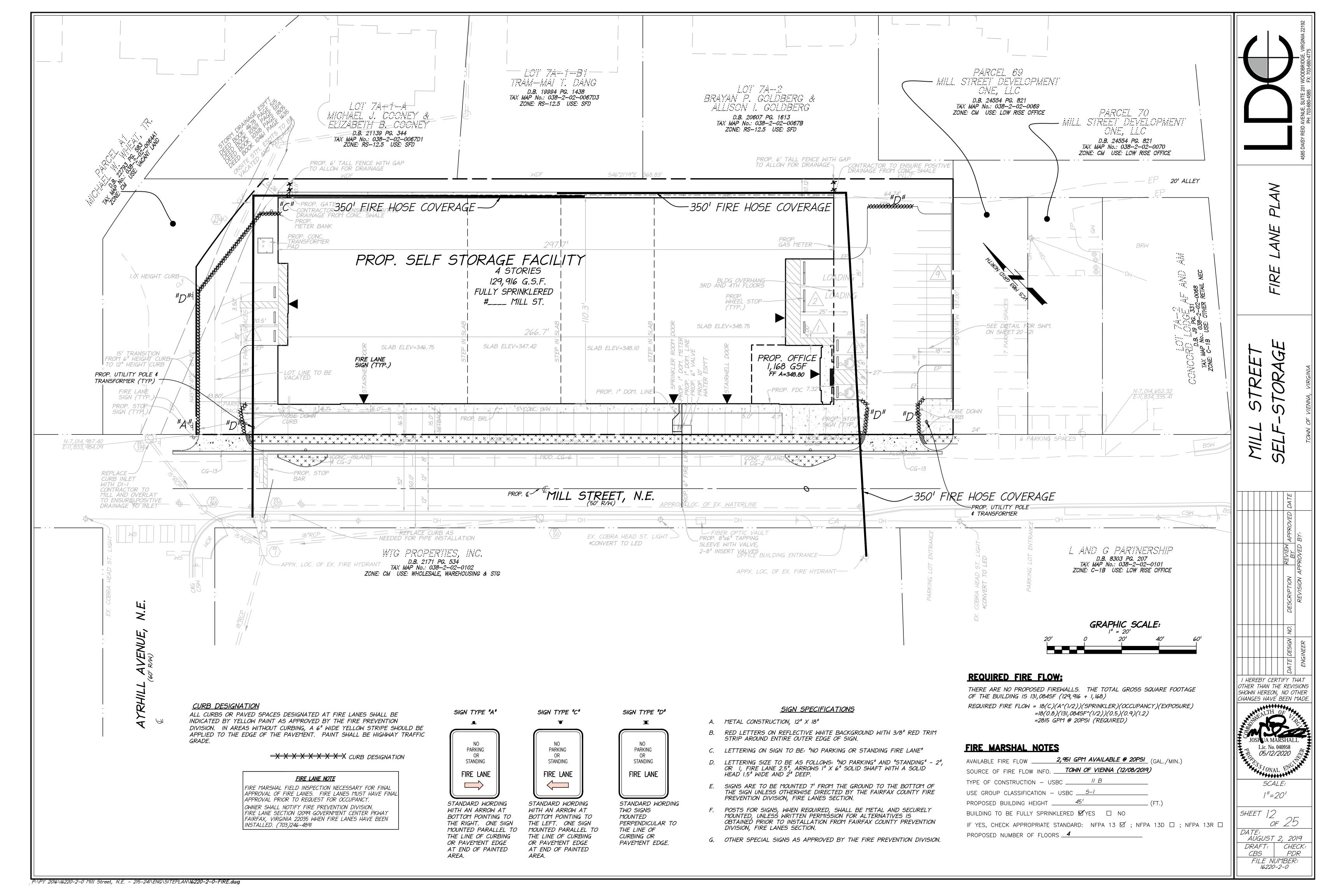


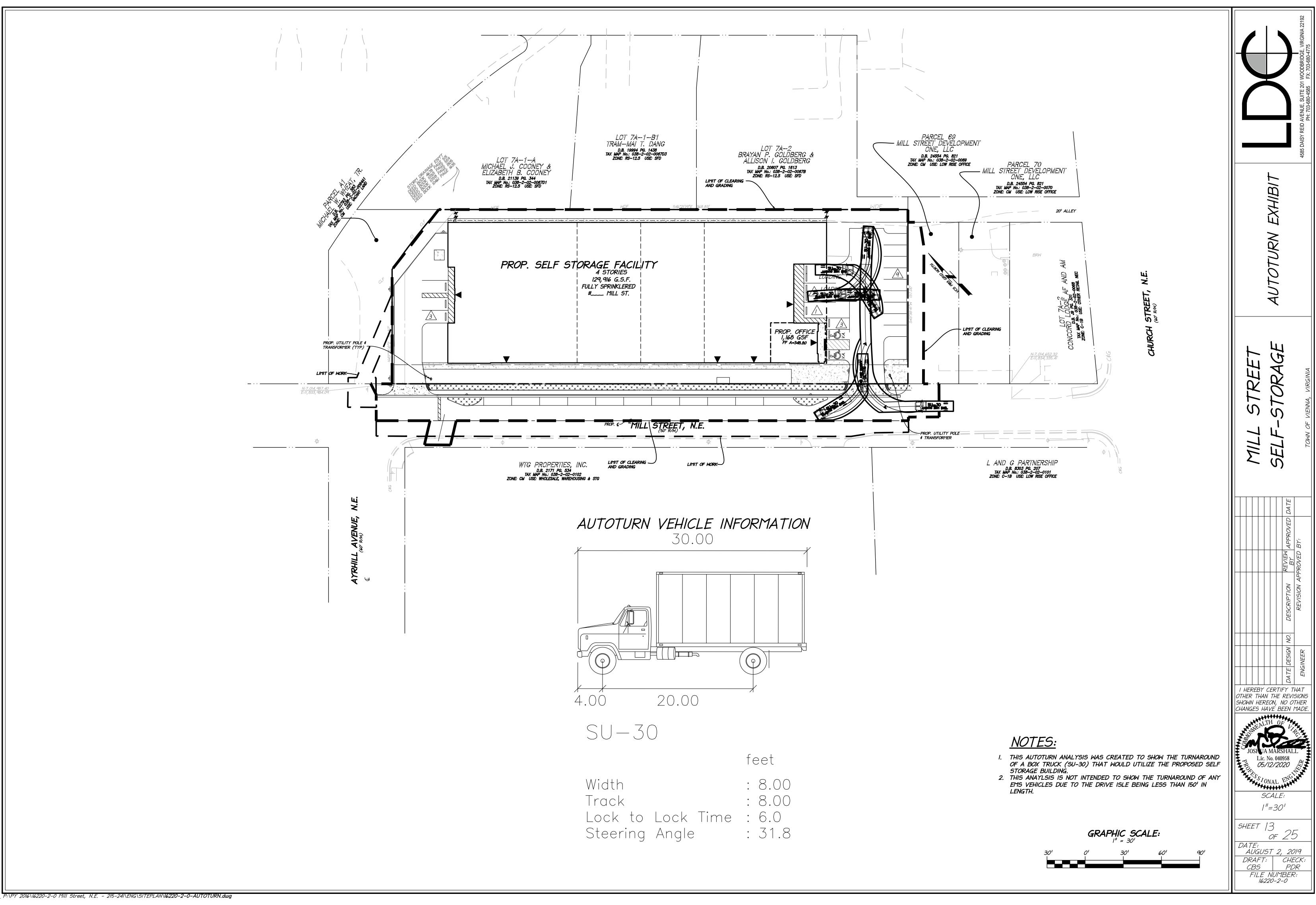
AUGUST 2, 2019

FILE NUMBER: 16220-2-0

CHECK: PDR

DRAFT:





I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.



385 DAISY REID AVENUE, SUITE 201 WOODBRIDGE, VIRGINIA

STORM SEWER PROFILES

MILL STREET SELF-STORAGE

TE DESIGN NO. DESCRIPTION REVIEW BY BY:

REVISION APPROVED BY:

I HEREBY CERTIFY THAT
OTHER THAN THE REVISIONS
SHOWN HEREON, NO OTHER
CHANGES HAVE BEEN MADE.

JOSHUA MARSHALL
Lic. No. 040958
05/12/2020

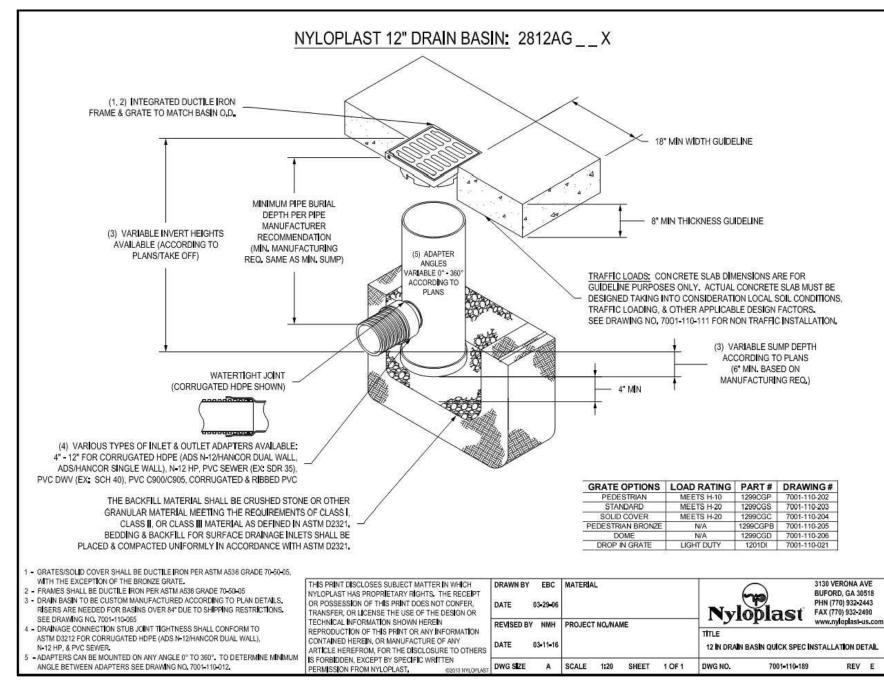
SCALE: AS SHOWN

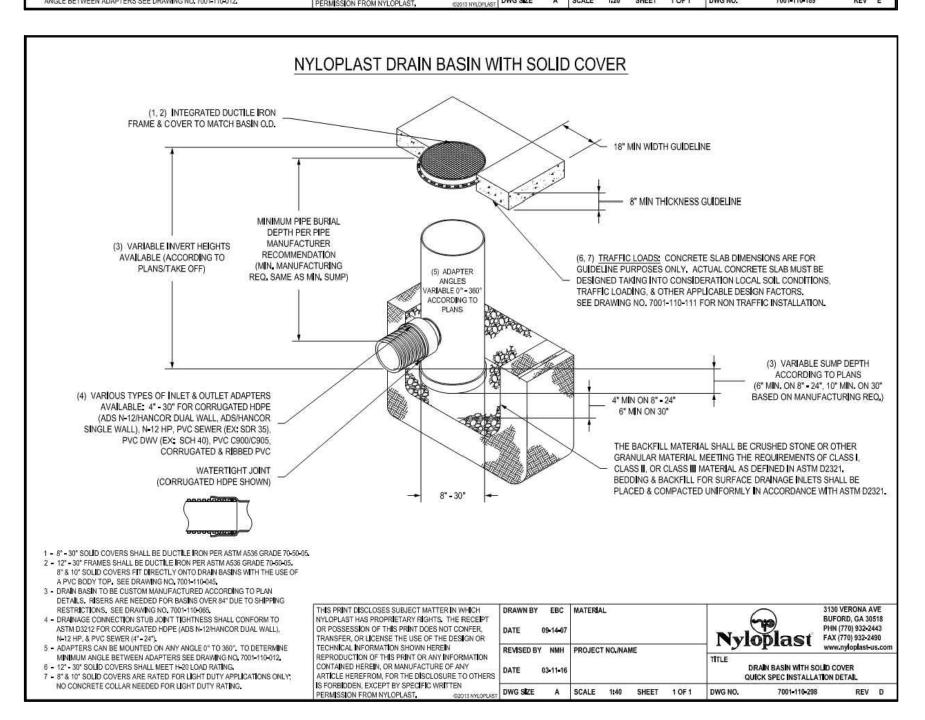
STRUCTURE #4B (ASSUME 50% CLOGGED)

 $Q_{10} = C*1*A*2(50% CLOGGED) = 0.45*6.77*1.54*2 = 9.38 CFS HEAD = 0.37$ GRATE = 347.75 10-YEAR WSE = 347.75 + 0.37 = 348.12



(866) 888-8479 / (770) 932-2443 • Fax: (770) 932-2490 © Nyloplast Inlet Capacity Charts June 2012





### STORM SEWER DESIGN COMPUTATIONS

FROM	ТО	DRAIN.	RUNOFF		A	INLET	RAINFALL	Q	(.0.000).000	T ELEV	LENGTH	SLOPE	DIA		CAPACITY	VELOCITY		
POINT	POINT	AREA	COEFF	INCREM	ACCUM	TIME	INTENSITY	RUNOFF		LOWER				n			TIME	REMARKS
		ACRES	С			MIN	IN/HR	CFS	FT	FT	FT	FT/FT	IN		CFS	FPS	SEC	
OFFSITE	EX629	4.04	0.73	2.95	2.95	5.00	6.77	19.97		į,								
EX629	EX339	0.00	0.00	0.00	2.95	5.00	6.77	19.97	343.55	341.57	117.59	1.68%	24	0.013	29.43	10.24	11.5	19"x30" ELLIPTICAL
EX339	EX868	0.21	0.70	0.15	3.10	5.00	6.77	20.97	341.57	340.55	47.95	2.13%	15	0.013	9.45	7.85	6.1	
EX868	EX1	0.00	0.00	0.00	3.10	5.00	6.77	20.97	340.45	339.67	35.50	2.20%	15	0.013	9.60	7.98	4.4	
RD1	5	0.76	0.90	0.68	0.68	5.00	6.77	4.63	348.20	348.00	8.13	2.46%	15	0.011	12.01	9.34	0.9	
5	UG1	0.00	0.00	0.00	0.68	5.00	6.77	4.63	348.00	347.00	39.66	2.52%	15	0.011	12.16	9.35	4.2	
4B	4	1.54	0.45	0.69	0.69	5.00	6.77	4.69	342.50	342.25	25.00	1.00%	15	0.011	7.65	6.65	3.8	
3A	3	0.35	0.70	0.25	0.25	5.00	6.77	1.66	342.40	341.85	52.86	1.04%	15	0.011	7.81	5.15	10.3	
EX511	EX1	0.76	0.90	0.68	0.68	5.00	6.77	4.63	340.62	339.64	148.58	0.66%	18	0.013	8.55	5.02	29.6	
4A	1	0.00	0.00	0.00	0.43	5.00	6.77	2.91	342.35	342.25	17.73	0.56%	15	0.011	5.75	4.78	3.7	10YR ROUTED FLOW
4	3	0.00	0.00	0.00	1.12	5.00	6.77	7.60	342.15	341.85	34.58	0.87%		0.013	9.81	6.24	5.5	TOTICIOOTEDTEOW
3	2	0.00	0.00	0.00	1.37	5.00	6.77	9.26	341.75	340.38	274.46	0.50%	+	0.013	16.03	5.37	51.1	
2	1	0.44	0.85	0.37	1.74	5.00	6.77	11.79	340.30	340.05	51.28	0.50%		0.013	15.96	5.64	9.1	
1	EX522	0.71	0.55	0.39	2.13	5.00	6.77	14.44	339.95	339.75	39.03	0.51%	24	0.013	16.24	5.94	6.6	
EX522	EX1	0.06	0.90	0.05	2.19	5.00	6.77	14.80	339.65	339.63	5.45	0.37%		0.013	24.91	5.38	1.0	

RM S	SEWER IN	LET DESI	GN COMP	UTATIONS	:																							DESIGN ST	ORM: 2 Y	R
	INLET				С	A [			(S)	(S	FT)	FT)							<u></u>			(FT)			(S	<u>(6</u>		Sag Inle	ets Only	
	TYPE	LENGTH (FT)	DRAINAGE AREA (A)	U	ON	CUM.	I (IN/HR)	Q INCR (CFS)	Q. CARRYOVER (CFS	Q <sub>1</sub> GUTTER FLOW (CF	S, GUTTER SLOPE (FT)	Sx, CROSS SLOPE (FT/	T, SPREAD (FT)	W (FT)	T/W	Sw (FT/FT)	S///Sx	В	a = 12W(S∾-Sx) + Loca Depression	S'w =a/(12W)	Se=Sx+S¹w(Eo) (FT/FT)	COMPUTED LENGTH, £(	L/Lī	E, EFFICIENCY	Q, INTERCEPTED (CF	Q₀, CARRYOVER (CF§	d (FT)	h (FT)	d/h	
	DI3-C	6	0.71	0.55	0.39	0.39	4.00	1.56	0.00	1.56	0.0010	0.0200	N/A	1.50	N/A	0.0833	N/A	N/A	N/A	N/A	N/A	N/A	-	-	1.56	0.00	0.17	0.46	0.37	
	DI-3C	3	0.36	0.55	0.20	0.20	4.00	0.78	0.00	0.78	0.0010	0.0200	8.10	1.50	0.19	0.0833	4.17	0.61	3.14	0.174	0.126	_	_	_	_	_				
	DI-3C	3	0.36	0.55	0.20	0.20	4.00	0.78	0.00	0.78	0.0010	0.0200	8.10	1.50	0.19	0.0833	4.17	0.61	3.14	0.174	0.126	<del></del>	-	_	_	-				
	DI3-C	6	0.44	0.85	0.37	0.37	4.00	1.50	0.00	1.50	0.0010	0.0200	N/A	1.50	N/A	0.0833	N/A	N/A	N/A	N/A	N/A	N/A	-	-	1.50	0.00	0.17	0.46	0.36	
	DI-3C	3	0.22	0.85	0.19	0.19	4.00	0.75	0.00	0.75	0.0010	0.0200	7.94	1.50	0.19	0.0833	4.17	0.61	3.14	0.174	0.127	-	-	-	-	-				$\perp$
	DI-3C	3	0.22	0.85	0.19	0.19	4.00	0.75	0.00	0.75	0.0010	0.0200	7.94	1.50	0.19	0.0833	4.17	0.61	3.14	0.174	0.127	_	-	_	_	-				

<b>HYDRAULIC GRADE LINE</b>	<b>COMPUTATIONS</b>

STORM SEWER GRATE INLET COMPUTATIONS:

INLET AREA RUNOFF GRATE 10 YR Q 10 YR GRATE 10 YR

EX339 0.21 0.7 DI-1 1.00 0.15 343.92 344.07

3A 0.35 0.7 DI-1 1.66 0.20 348.45 348.65

PROJECT:	Mill Street
COUNTY:	Town of Vienna
00110 011	

				0.013	(RCP)										OMP. BY:		Vicilia						
				0.013	(IXOI)							JUN	CTION	100000	JIVII . DT.	I DIX							
											,				2					FINAL	INLET	INLET	HGL
AT	OUTLET	OUTLET	OUTLET	OUTLET	FRIC-	FRIC-	VELOCITY	September Woulder	DISCHARGE	VELOCITY	MAX	INFLOW	MAX		00. T-4000000	MAX	TOTAL	INLET	SHAPE	HEAD	WATER	RIM	BELOW
INLET	WATER	PIPE	FLOW-	PIPE	TION	TION	OUTLET	CONTR.	INFLOW	INFLOW	INFLOW	VELOCITY	EXPAN.	MAX	LOSS	BEND	HEAD	CORRECT	CORRECT	LOSS	SURFACE	ELEV	RIM
#	SURFACE	DIAM	RATE	LENGTH	SLOPE	LOSS	PIPE	LOSS	PIPE	PIPE	PRODUCT	HEAD	LOSS	ANGLE	COEFF	LOSS	LOSS	FACTOR	FACTOR		ELEV		
	ELEV	Do	Qo	Lo	Sfo	Hf	Vo	Но	Qi	Vi	Qi * Vi	(Vi^2)/(2g)	Hi	Ai	K	Hb	Ht	1.3 Ht	0.5 Ht	Н	200-11-00 E-200	0000 950	April 1000 1000 1000
	(FT)	(IN)	(CFS)	(FT)	(%)	(FT)	(FPS)	(FT)	(CFS)	(FPS)	(FT <sup>4</sup> /S <sup>2</sup> )	(FT)	(FT)	(DEG)		(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)		(13)	(14)		(15)	(16)	(17)	(18)	(19)	(20)	(21)	
														and the state of t									
EX868	340.67	15	20.97	35.50	10.60%	3.763	8.0	0.247	20.97	7.9	164.66	0.9574	0.335	22	0.27	0.258	0.841	0.84	0.420	4.183	344.85	343.96	-0.89
EX339	344.85	15	20.97	47.95	10.60%	5.083	7.9	0.239	19.97	10.2	204.45	1.6267	0.569	64	0.58	0.943	1.752	1.75	0.876	5.959	350.81	343.75	-7.06
EX629	350.81	24	19.97	117.59	0.80%	0.941	10.2	0.407	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.407	0.53	0.264	1.205	352.02	354.20	2.18
5	348.00	15	4.63	39.66	0.50%	0.198	9.3	0.339	4.63	9.3	43.23	1.3536	0.474	11	0.14	0.192	1.005	1.01	0.503	0.701	348.70	350.50	1.80
4A	344.05	15	2.91	17.73	0.20%	0.035	4.8	0.089	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.089	0.12	0.058	0.093	344.14	349.00	4.86
							4							32	07								200
4B	344.05	15	4.69	25.00	0.50%	0.125	6.7	0.172	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.172	0.22	0.112	0.237	344.28	347.75	3.47
																		j i					
3A	343.56	15	1.66	52.86	0.10%	0.053	5.2	0.103	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.103	0.13	0.067	0.120	343.68	348.45	4.77
																			o'				
EX522	341.63	30	14.80	5.45	0.10%	0.005	5.4	0.112	14.44	5.9	85.73	0.5475	0.192	15	0.19	0.104	0.408	0.41	0.204	0.209	341.84	343.53	1.69
1	341.84	24	14.44	39.03	0.40%	0.156	5.9	0.137	11.79	5.6	66.48	0.4934	0.173	75	0.64	0.313	0.623	0.62	0.311	0.468	342.31	344.07	1.76
2	342.31	24	11.79	51.28	0.30%	0.154	5.6	0.123	9.26	5.4	49.70	0.4472	0.157	0	0.00	0.000	0.280	0.36	0.182	0.336	342.64	344.17	1.53
3	342.64	24	9.26	274.46	0.20%	0.549	5.4	0.112	7.60	6.2	47.44	0.6045	0.212	90	0.70	0.423	0.747	0.75	0.373	0.922	343.56	347.20	3.64
4	343.56	18	7.60	34.58	0.50%	0.173	6.2	0.151	2.91	4.8	13.91	0.3545	0.124	62	0.57	0.202	0.477	0.62	0.310	0.483	344.05	348.50	4.45
4A	344.05	15	2.91	17.73	0.20%	0.035	4.8	0.089	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.089	0.12	0.058	0.093	344.14	349.00	4.86

		<u>10</u>	00-YEAR O	VERLAND	RELIEF C	OMPUTATI	ONS		
STRUCTURE	Α	С	CA	I	Q <sub>100</sub>	L	Н	WEIR ELEV	100 OVERLAND
NUMBER	(Ac)			(in/hr)	(cfs)	(ft)	(ft)	(ft)	RELIEF ELEV.
3A	0.35	0.70	0.25	9.10	2.23	20	0.10	348.65	348.75
4B	1.54	0.45	0.69	9.10	6.31	25	0.18	348.30	348.48

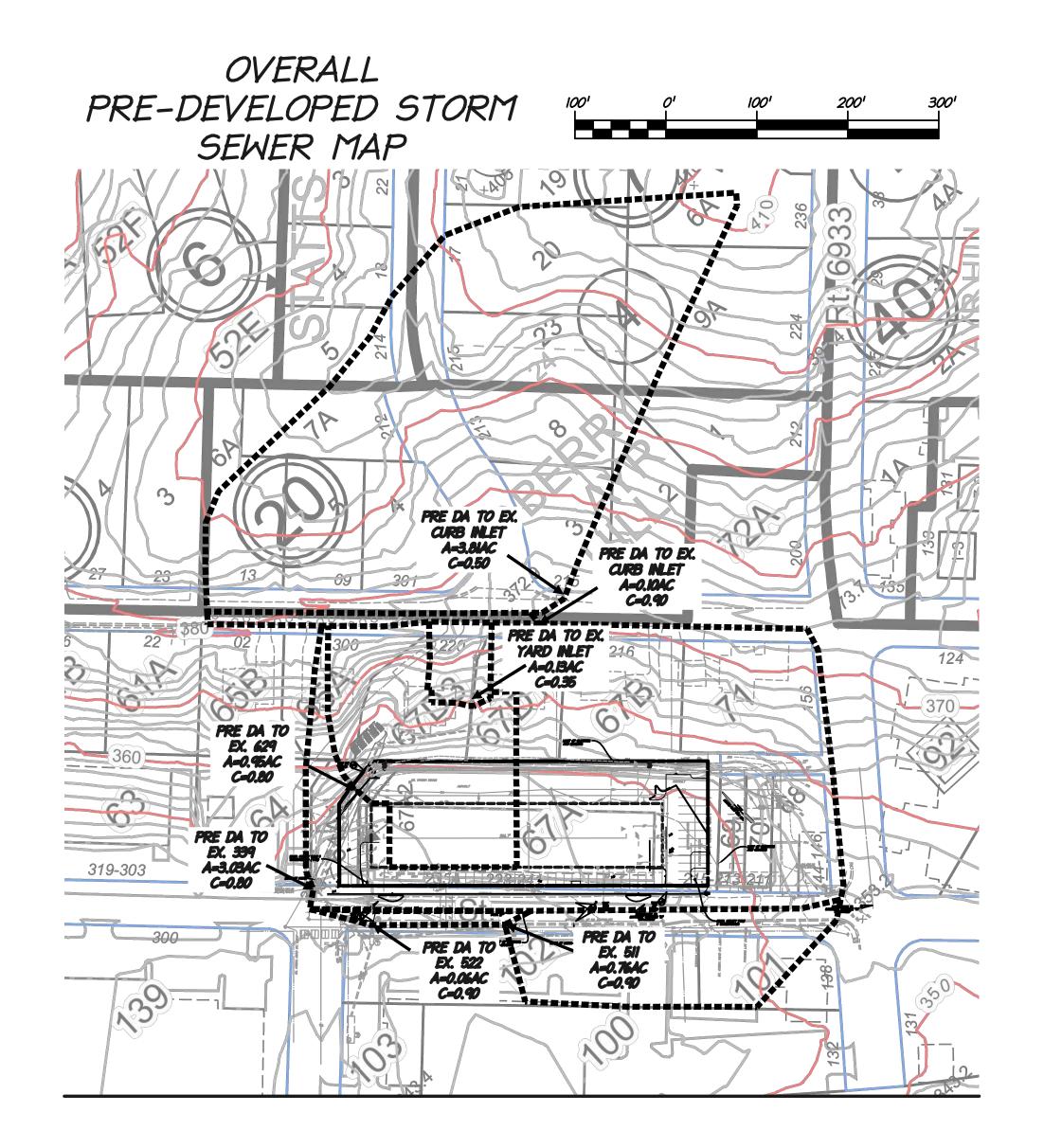
I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.

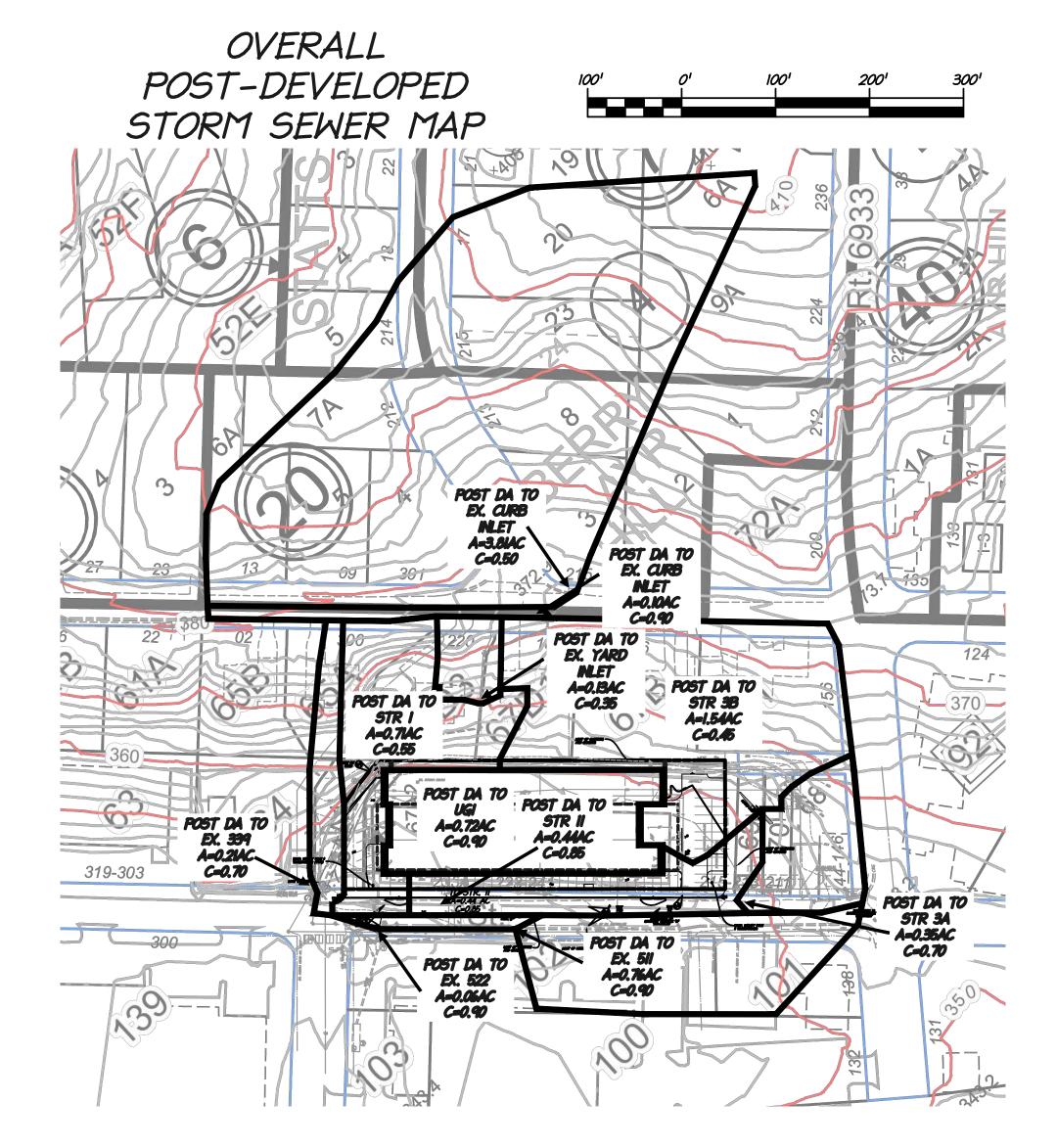


SCALE: N/A

SHEET 15 of 25 AUGUST 2, 2019 DRAFT: CHECK:

FILE NUMBER: 16220-2-0





# PRE-DEVELOPED STORM SEWER

### STORM SEWER DESIGN COMPUTATIONS

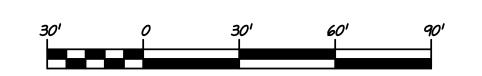
FROM	ТО	DRAIN.	RUNOFF	С	Α	INLET	RAINFALL	Q	INVER	T ELEV	LENGTH	SLOPE	DIA		CAPACITY	VELOCITY	FLOW	
POINT	POINT	AREA	COEFF	INCREM	ACCUM	TIME	INTENSITY	RUNOFF	UPPER	LOWER				n		:	TIME	REMARKS
		ACRES	С			MIN	IN/HR	CFS	FT	FT	FT	FT/FT	IN		CFS	FPS	SEC	
OFFSITE	EX629	4.04	0.73	2.95	2.95	5.00	6.77	19.97										
EX629	EX339	0.95	0.80	0.76	3.71	5.00	6.77	25.12	344.00	341.67	136.98	1.70%	15	0.013	8.45	7.02	19.5	
EX339	EX868	3.03	0.80	2.42	6.13	5.00	6.77	41.53	341.57	340.55	47.95	2.13%	15	0.013	9.45	7.85	6.1	
EX868	EX1	0.00	0.00	0.00	6.13	5.00	6.77	41.53	340.45	339.67	35.50	2.20%	15	0.013	9.60	7.98	4.4	
			0000															
EX511	EX1	0.76	0.90	0.68	0.68	5.00	6.77	4.63	340.62	339.64	148.58	0.66%	18	0.013	8.55	5.02	29.6	
EX522	EX1	0.06	0.90	0.05	0.05	5.00	6.77	0.37	339.65	339.63	5.45	0.37%	18	0.013	6.38	2.01	2.7	

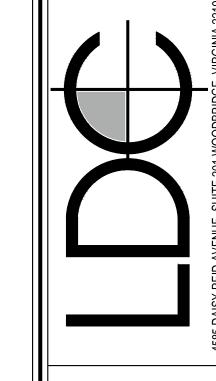
# PRE-DEVELOPED HYDRAULIC GRADE LINE COMPUTATIONS

### HYDRAULIC GRADE LINE COMPUTATIONS

COUNTY: Town of Vienn

																	COUNTY:	Town of	Vienna						
	MANNING	<b>ROUGHN</b>	ESS COEFFI	CIENT =		0.013	(RCP)									С	OMP. BY:	PDR							
	HIDE	HIDE												JUN	CTION	LOSS									
																						FINAL	INLET	INLET	HGL
AT	TO	OUTLET	OUTLET	OUTLET	OUTLET	OUTLET	FRIC-	FRIC-	VELOCITY		DISCHARGE	VELOCITY	MAX	INFLOW	MAX			MAX	TOTAL	INLET	SHAPE	HEAD	WATER	RIM	BELOW
INLET	INLET	INVERT	WATER	PIPE	FLOW-	PIPE	TION	TION	OUTLET	CONTR.	INFLOW	INFLOW	INFLOW	VELOCITY	EXPAN.	MAX	LOSS	BEND	HEAD	CORRECT	CORRECT	LOSS	SURFACE	ELEV	RIM
#	#	ELEV	SURFACE	DIAM	RATE	LENGTH	SLOPE	LOSS	PIPE	LOSS	PIPE	PIPE	PRODUCT	HEAD	LOSS	ANGLE	COEFF	LOSS	LOSS	FACTOR	FACTOR		ELEV		0' min-VDOT
			ELEV	Do	Qo	Lo	Sfo	Hf	Vo	Но	Qi	Vi	Qi * Vi	(Vi^2)/(2g)	Hi	Ai	K	Hb	Ht	1.3 Ht	0.5 Ht	Н			1' min-FFX
		(FT)	(FT)	(IN)	(CFS)	(FT)	(%)	(FT)	(FPS)	(FT)	(CFS)	(FPS)	(FT^4/S^2)	(FT)	(FT)	(DEG)		(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)	(FT)
(1)			(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)		(13)	(14)		(15)	(16)	(17)	(18)	(19)	(20)	(21)	
EX868	EX1	339.67	340.67	15	41.53	35.50	41.40%	14.697	8.0	0.247	41.53	7.9	326.10	0.9574	0.335	22	0.27	0.258	0.841	0.84	0.420	15.117	355.79	343.96	-11.83
EX339	EX868	340.55	355.79	15	41.53	47.95	41.40%	19.851	7.9	0.239	25.12	7.0	176.38	0.7655	0.268	64	0.58	0.444	0.951	1.24	0.618	20.470	376.26	344.25	-32.01
EX629	EX339	341.67	376.26	15	25.12	136.98	15.20%	20.821	7.0	0.191	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.191	0.25	0.124	20.945	397.20	348.14	-49.06
EX511	EX1	339.64	340,84	18	4.63	148.58	0.20%	0.297	5.0	0.098	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.098	0.13	0.064	0.361	341.20	344.65	3.45
EX522	EX1	339.63	340.83	18	0.37	5.45	0.00%	0.000	2.0	0.016	0.00	0.0	0.00	0.0000	0.000	0	0.00	0.000	0.016	0.02	0.010	0.010	340.84	343.53	2.69





VERALL PRE ANI POST STM SEWER

MILL STREET SELF-STORAGE

DATE DESIGN NO. DESCRIPTION BY:

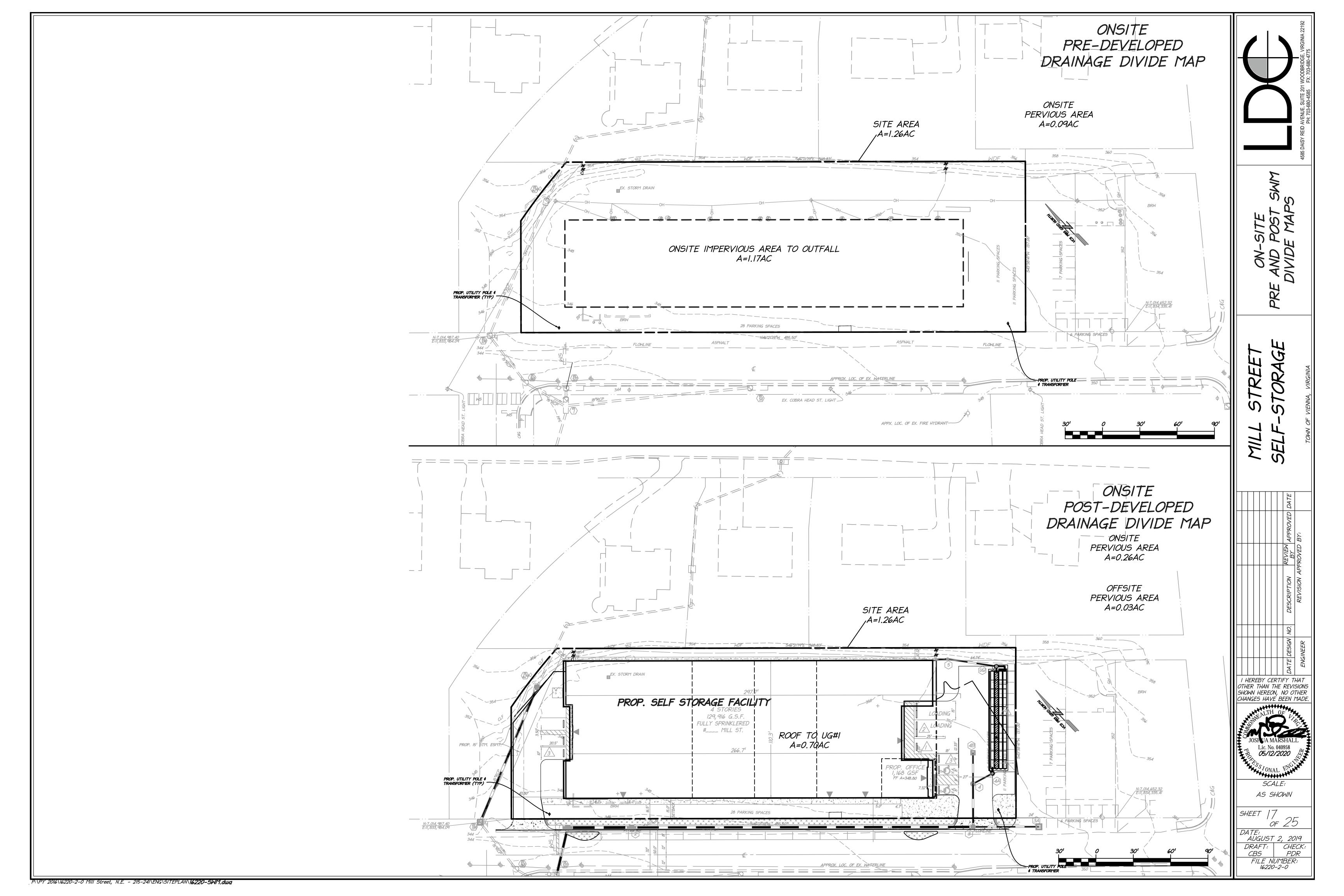
REVISION APPROVED BY:

JOSHUA MARSHALL
Lic. No. 040958
05/12/2020

SCALE: AS SHOWN

SHEET 16 OF 25 DATE: AUGUST 2, 2019

PAFT: CHECK: BS PDR FILE NUMBER: 16220-2-0



### PRE-DEVELOPED - AREA I (ONSITE)

MILL STREET ONSITE UNDETAINED

# VIENNA, VIRGINIA WATERSHED PEAK TABLE

		WATERSHED PEAR TABLE		
SUB-AREA	PEAK F	FLOW BY RAINFALL RETURN PL	RIOD	
AREA I	I-YR	2-YR II	O-YR	
	3.46 CFS	4.23 CF5 6	.59 CFS	
		AND USE AND CURVE NUMBER		
SUB-AREA	LAND USE	HYDROLOGIC SOIL GROUP	AREA(AC)	CN
AREA I	IMPERVIOUS OPEN SPACE	D	1.17 0.09	98 80
TO	TAL AREA/WEIGHTEI	D CURVE NUMBER	<u>1.26</u>	<u>97</u>

### POST-DEVELOPED CN

TR-55 METHODOLOGY WAS USED FOR CALCULATING THE COMPOSITE CN FACTORS FOR THE SITE. THE POST-DEVELOPED SITE CONDITION ARE OPEN SPACE AND PAVEMENT IN GOOD CONDITION WITH TYPE D SOIL. COMPOSITE CN CALCULATIONS CAN BE FOUND BELOW.

### <u>POST-DEVELOPED - AREA I (UNCONTROLLED-ONSITE) POST-DEVELOPED - AREA I (CONTROLLED-ONSITE)</u>

		MILL STREET ONSITE UNDETAINED VIENNA, VIRGINIA					MILL STREET ONSITE DETAINED VIENNA, VIRGINIA		
		WATERSHED PEAK TABLE					WATERSHED PEAK TABLE		
SUB-AREA	PEAK	FLOW BY RAINFALL RETURN	PERIOD		SUB-AREA	PEAK I	FLOW BY RAINFALL RETURN I	PERIOD	
AREA I	I-YR	2-YR	IO-YR		AREA I	I-YR	2-YR	10-YR	
	1.12 CFS		2.50 CFS			2.02 CFS	2.46 CFS	3.81 CFS	
	SUB-AREA	LAND USE AND CURVE NUMBE	R DETAILS			SUB-AREA L	AND USE AND CURVE NUMBE	R DETAILS	
SUB-AREA	LAND USE	HYDROLOGIC SOIL GROUF	AREA(AC)	CN	SUB-AREA	LAND USE	HYDROLOGIC SOIL GROUP	AREA(AC)	CN
AREA I	IMPERVIOUS OPEN SPACE	D	0.28 0.26	98 80	AREA I	IMPERVIOUS OPEN SPACE	D	0.70 0.00	98 80
το	PTAL AREA/WEIGHT!	ED CURVE NUMBER	<u>0.54</u>	<u>89</u>	<i></i>	TAL AREA/WEIGHTE	D CURVE NUMBER	<u>0.70</u>	<u>98</u> 

### TIME OF CONCENTRATION

DUE TO THE SIZE OF THE DRAINAGE AREA AND THE PREDOMINANTLY IMPERVIOUS SURFACE AREA OF THE SITE A TIME OF CONCENTRATION OF 0.10 HR WAS UTILIZED.

### OUTFALL NARRATIVE

THE SUBJECT PROPERTY IS LOCATED WITHIN THE DIFFICULT RUN WATERSHED. THE PRE-DEVELOPED LAND COVER OF THE SITE IS PREDOMINATLY IMPERVIOUS WITH AN EXISTING BUILDING AND PARKING LOT. THE POST-DEVELOPED CONDITION PROPOSES THE REMOVAL AND REPLACEMENT OF THE EXISTING BUILDING WITH ONE OF A LARGER FOOTPRINT AND THE REMOVAL OF A PORTION OF ONSITE AND OFFSITE PAVEMENT. THE ULTIMATE OUTFALL POINT FOR THIS PROJECT IS THE POTOMAC RIVER.

THE SITE IS BEING ANALYZED WITH ONE POINT OF INTEREST, QUANTITY CONTROL FOR AREA #I IS SATISFIED THROUGH DETENTION IN THE UNDERGROUND SWM FACILITY WITH ISOLATOR ROW. THE SITE DRAINS TO EXISTING INLETS THAT CONVERGE AT EXISTING STM #I. THE TAILWATER CONDITION FOR THE UNDERGROUND SWM FACILITY IS MODELED AS FREE FLOWING DUE TO THE SIGNIFICANT DIFFERENCE IN THE TIMING OF THE PEAK EVENT VERSES THE STORM SEWER PIPE CALCULATIONS.

### AREA I

IN THE PRE-DEVELOPED CONDITION AREA #I SHEET FLOWS TO THE SOUTHWEST AND IS COLLECTED BY EX. STORM STRUCTURES THAT CONVERGE AT EX. STORM #I. FOR THE POST-DEVELOPED CONDITION AREA #I UTILIZES A STORMWATER DETENTION VOLUME CONTROL STRUCTURE WHICH HAS BEEN DESIGNED TO REGULATE THE OUTFLOW FROM THE FACILITY IN ACCORDANCE WITH THE ALLOWABLE RELEASE RATES. SEE THIS SHEET FOR ALLOWABLE RELEASE RATE COMPUTATIONS. DISCHARGES FROM THE DETENTION FACILITY FOR I, 2 AND IO YEAR STORM EVENTS ARE LESS THAN THE ALLOWABLE RELEASE RATE COMPUTED FOR THE SUBJECT AREA. PLEASE REFER TO THE FLOW SUMMARY TABLE ON THIS SHEET.

ADEQUATE OUTFALL ANALYSIS DETERMINES THAT THE I-YR, 2-YR AND IO-YR POST-DEVELOPMENT RUNOFF GENERATED FLOWS FROM THE DISTURBED AREAS ARE LESS THAN THE ALLOWABLE RELEASE RATES. IN ACCORDANCE WITH VAESCH MS-I9(c)3, THE OUTFALL IS CONSIDERED ADEQUATE SINCE THE ENERGY BALANCE EQUATION HAS BEEN SATISFIED AND THE FLOWS HAVE BEEN REDUCED BELOW PRE-DEVELOPMENT CONDITIONS. GIVEN THE INFORMATION PROVIDED HEREIN IT IS THE ENGINEER'S OPINION THAT THE OUTFALL FOR THE SUBJECT PROPERTY IS ADEQUATE AND SATISFIES THE REQUIREMENTS AS SET FORTH IN STATE CODE 9VAC25-870-66 AS WELL AS MS-I9.

THE STORM WATER MANAGEMENT FACILITY WILL BE MAINTAINED BY THE OWNER.

### PRE-DEVELOPMENT / POST-DEVELOPMENT FLOW SUMMARY TABLE

		PRE-DEVELOPE	D ONSITE				
	AREA (AC)	IMP AREA (AC)	CN	Q1 (cfs)	Q2 (cfs)	Q10 (cfs)	TC (HR)
AREA 1	1.26	1.17	97	3.46	4.23	6.59	0.10
	PC	OST-DEVELOPED - U	NCONTROLL	.ED			
	TOTAL AREA (AC)	IMP AREA (AC)	CN	Q1 (cfs)	Q2 (cfs)	Q10 (cfs)	TC (HR)
AREA 1	0.54	0.28	89	1.12	1.45	2.50	0.10
	POST-DEVELO	OPMENT - CONTRO	LLED (PRIOR	TO ROUTI	NG)		
	TOTAL AREA (AC)	IMP AREA (AC)	CN	Q1 (cfs)	Q2 (cfs)	Q10 (cfs)	TC (HR)
AREA 1	0.72	0.72	98	2.02	2.46	3.81	0.10
	POST-DE	VELOPMENT - CON	TROLLED ALI	LOWABLE	000		
	TOTAL AREA (AC)	IMP AREA (AC)	CN	Q1 (cfs)	Q2 (cfs)	Q10 (cfs)	TC (HR)
AREA 1	0.72	0.72	98	1.41	2.78	4.09	0.10
	POST-D	EVELOPMENT - ACT	TUAL RELEAS	E RATES	(1)50		10010
	TOTAL AREA (AC)	IMP AREA (AC)	CN	Q1 (cfs)	Q2 (cfs)	Q10 (cfs)	TC (HR)
AREA 1	0.72	0.72	90	1.28	1.46	2.54	0.10

### CHANNEL PROTECTION

CHANNEL PROTECTION IS MET THROUGH
THE ACTUAL RELEASE RATE FROM THE
UNDERGROUND SYSTEM BEING LESS THAN
THE ALLOWABLE RATE CALCULATED ON
THIS SHEET FOR ENERGY BALANCE. SEE
RUNOFF VOLUME COMPUTATIONS FOR THE
I YR STORM EVENTS ON THIS SHEET
AND THE ACTUAL RELEASE RATES FOUND
IN THE FLOW SUMMARY ON THIS SHEET.

### <u>FLOOD PROTECTION</u>

FLOOD PROTECTION CALCULATIONS SHOW
THE IO-YEAR POST-DEVELOPED FLOW
RATE HAS BEEN REDUCED BELOW THE
PRE-DEVELOPED FLOW RATE FOR THE
SITE AND THEREFOR SATISFY THE
REQUIRED FLOOD PROTECTION
REQUIREMENTS.

RUNOFF VOLUME CO	OMPUTATIONS (	'EXISTI	NG CO	NDITION)
PER CHAPTER 5 - VIRG	INIA SWM HANDBO	OK VOLUI	ME II	
RAIN FALL DEPTH JURI	SDICTION: Town of	Vienna .		
DRAINA	AGE AREA: AREA 1			
			UNIT	FORMULA KEY
1. RAINFALL DEPTH (P)	1YR STORM	2.62	IN	P
2. HYDOLOGIC SOILS DRAINING T	O OUTFALL ARE:			
	SOIL GROUP A:	0.00	AC	
	SOIL GROUP B:	0.00	AC	
	SOIL GROUP C:	0.00	AC	
	SOIL GROUP D:	1.26	AC	
3. WEIGHTED CN FOR SITE GOOD	CONDITION:	97		CN
(SEE TR-55 CALCULATION FOR V	VEIGHTED CN)	37		Civ
4. TOTAL AREA DRAINING TO OU	TFALL	1.01	AC	Α
5. S=(1000/CN)-10		0.31		S
1 YEAR STORM EVENT				
6. RUNOFF $Q = ((P-0.2S)^2)/(P+0.8)$	RS)	2.28	IN	RQex
7. RUNOFF VOLUME (RVex) = A*R	Qex	0.192	AC-FT	RVex

### RUNOFF VOLUME COMPUTATIONS (DEVELOPED CONDITION)

			UNIT	<b>FORMULA KEY</b>
1. RAINFALL DEPTH (P)	1YR STORM	2.62	IN	P
2. HYDOLOGIC SOILS DRAINING T	O OUTFALL ARE:			
	SOIL GROUP A:	0.00	AC	
	SOIL GROUP B:	0.00	AC	
	SOIL GROUP C:	0.00	AC	
	SOIL GROUP D:	1.26	AC	
	~	(0)	20.5	**
1 YEAR STORM EVENT			-	•
3. ADJUSTED CN FOR SITE (SEE TR	-55 Calculations):	94		CN
4. TOTAL AREA DRAINING TO OU	TFALL	1.26	AC	Α
5. S=(1000/CN)-10		0.62		S
6. RUNOFF $Q = ((P-0.2S)^2)/(P+0.8)$	35)	2.00	IN	RQd
7. RUNOFF VOLUME (RVd) = A*RC	Qd	0.210	AC-FT	RVd

Reduction required for Channel Protection per VA State code section 9VAC25-870-66:

Qallow = I.F.\*Qex \* (RVex/RVd)

I.F. (Improvement Factor) equals 0.8 for sites > 1acre or 0.9 for sites < or = 1 acre

See Detention Release Rate Computations for Reductions

DETENTION RELEASE RATES
PRE DEVELOPMENT SITE CONDITIONS - DEVELOPMENT AR
**SEE PRE DEVELOPMENT DRAINAGE AREA MAP**

FOR 1-YEAR CHANNEL PROTECTION PER 9VAC25-870-66 USE: AREA = 1.01 AC TC= 0.10 HR CN= 97

 $\frac{1 \ YEAR \ STORM}{12\%} \qquad 12\% \ REDUCTION \ REQUIRED$   $SEE \ RUNOFF \ VOLUME \ COMPS$   $Qex = \qquad 3.46 \ CFS \quad (FROM \ TR-55 \ RESULTS)$   $Qallow = I.F. *Qex*(RVex/RVd) \qquad 2.53 \ CFS$ 

POSTDE	VELO	PM	ENT SITE CONDITIONS
			ENTION FOR THE 1, 2, & 10 YEAR STORM IE CONTROLLED AREA.
**SEE PO	ST DEVE	LOPN	MENT DRAINAGE AREA MAP**
POST DEVELOP	MENT SI	TE CC	ONDITIONS - UNCONTROLLED AREA:
AREA = 1 YEAR STORM	0.54	AC	TC= 0.10 HR   CN = 89
	1.12	CFS	(FROM TR-55 RESULTS)
<b>POST DEVELOPM</b>	ENT SITE	CON	DITIONS -OFFSITE CONTROLLED AREA:
!!THIS AREA	I THE TOURS OF STREET	120000	TROLLED TO OFFSET THE ON-SITE TROLLED AREA!!
**SEE PO	ST DEVE	LOPN	MENT DRAINAGE AREA MAP**

AREA = 0.00 AC TC= 0.00 HR CN= 0 1 YEAR STORM Q1 off site = 0.00 CFS (FROM TR-55 RESULTS)

TOTAL ALLOWABLE RELEASE FROM SWM SYSTEM

Q(SWM)allowable = Qallow - Quncontrolled + Qoffsite

1 YEAR STORM

Q1(SWM)allowable = 1.41 CFS

RESULT: \*\*CHANNEL PROTECTION REDUCTION PROVIDED PER

STATE CODE\*\*

DAISY REID AVENUE, SUITE 201 WOODBRIDGE, VIRGINIA 22

SWIT COMPUTATIONS RELEASE RATES

MILL STREET SELF-STORAGE

A DATE DESIGN NO. DESCRIPTION BY:

REVISION APPROVED BY:

JOSHUA MARSHALL
Lic. No. 040958
05/12/2020

OTHER THAN THE REVISIONS

SCALE: |" = 30'

SHEET 18 OF 25

AUGUST 2, 2019

DRAFT: CHECK:
CBS PDR

FILE NUMBER:
16220-2-0

\_\_\_\_\_\_

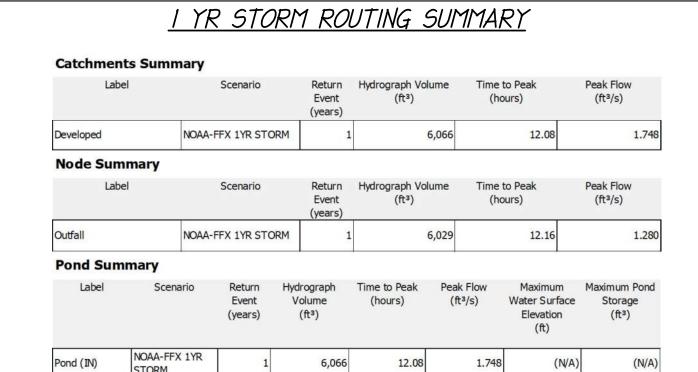
### **Outlet Connectivity** Structure Type Outlet ID Direction Orifice-Circular 342.35 343.95 344.68 Rectangular Weir Forward Tailwater Settings Structure ID: 1-YR orifice Structure Type: Orifice-Circular Number of Openings 342.35 ft Elevation Orifice Diameter 8.0 in Orifice Coefficient 0.600 Structure ID: Overflow Weir Structure Type: Rectangular Weir Number of Openings 343.95 ft Elevation 4.00 ft Weir Length 3.00 (ft^0.5)/s Weir Coefficient Structure ID: TW Structure Type: TW Setup, DS Channel Tailwater Type Free Outfall Convergence Tolerances Maximum Iterations 0.01 ft Tailwater Tolerance (Minimum) 0.50 ft Tailwater Tolerance (Maximum) Headwater Tolerance 0.01 ft (Minimum) Headwater Tolerance 0.50 ft (Maximum)

0.001 ft<sup>3</sup>/s

10.000 ft3/s

Flow Tolerance (Minimum)

Flow Tolerance (Maximum)



6,029

343.26

NOAA-FFX 1YR

Pond (OUT)

		low (Total In)	2S/t - O	2S/t + O	Infiltration	Flow (Outlet)	Volume	Elevation
(h	ours)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³/s)	(ft³)	(ft)
	9.04 9.12	0.045 0.047	0.386	0.472	0.000	0.043	61 61	342.47 342.47
	9.20	0.049	0.395	0.477	0.000	0.045	62	342.4
	9.28	0.051	0.401	0.495	0.000	0.047	63	342.4
	9.36	0.053	0.408	0.505	0.000	0.049	64	342.48
	9.44	0.055	0.415	0.516	0.000	0.051	66	342.48
	9.52	0.057	0.423 0.431	0.528 0.540	0.000	0.052 0.055	67	342.48
	9.68	0.062	0.431	0.552	0.000	0.057	68 70	342.48 342.49
	9.76	0.064	0.447	0.565	0.000	0.059	71	342.49
	9.84	0.066	0.455	0.577	0.000	0.061	72	342.49
	9.92	0.068	0.464	0.590	0.000	0.063	74	342.50
	10.00	0.071 0.073	0.472 0.481	0.603 0.616	0.000	0.065	75 77	342.50 342.50
	10.16	0.078	0.492	0.632	0.000	0.070	78	342.50
	10.24	0.083	0.505	0.653	0.000	0.074	81	342.5
	10.32	0.073	0.511	0.661	0.000	0.075	81	342.5
	10.40	0.080	0.513	0.664	0.000	0.076	82	342.5
	10.48	0.084	0.521 0.533	0.677 0.694	0.000	0.078	83 85	342.5 342.5
	10.64	0.096	0.549	0.718	0.000	0.085	88	342.5
	10.72	0.104	0.569	0.749	0.000	0.090	93	342.5
	10.80	0.113	0.594	0.787	0.000	0.096	98	342.5
	10.88	0.122 0.129	0.622 0.649	0.829 0.873	0.000	0.103 0.112	104 110	342.5
	11.04	0.129	0.675	0.873	0.000	0.112	115	342.5 342.5
	11.12	0.153	0.704	0.967	0.000	0.131	120	342.5
	11.20	0.170	0.739	1.027	0.000	0.144	127	342.5
	11.28	0.186	0.779	1.095	0.000	0.158	135	342.5
	11.36 11.44	0.201	0.820 0.861	1.165 1.235	0.000	0.173 0.187	143 151	342.6 342.6
	11.52	0.232	0.903	1.307	0.000	0.202	159	342.6
	11.60	0.325	0.989	1.460	0.000	0.235	176	342.6
	11.68	0.365	1.096	1.679	0.000	0.291	200	342.6
	11.76 11.84	0.429	1.200 1.328	1.891 2.170	0.000	0.345 0.421	223 252	342.7 342.7
	11.92	0.714	1.502	2.582	0.000	0.540	294	342.8
	12.00	1.122	1.788	3.338	0.000	0.775	369	342.9
	12.08	1.748	2.441	4.659	0.000	1.109	511	343.1
	12.16	1.554	3.183	5.744	0.000	1.280	643	343.2
	12.24 12.32	0.830 0.576	3.059 2.313	5.567 4.465	0.000	1.254 1.076	621 488	343.24 343.0
	12.40	0.430	1.782	3.320	0.000	0.769	367	342.9
	12.48	0.383	1.507	2.595	0.000	0.544	295	342.8
	12.56	0.322	1.346	2.213	0.000	0.433	256	342.7
	12.64 12.72	0.249 0.227	1.213 1.102	1.918 1.689	0.000	0.352 0.294	225 201	342.7 342.6
	12.80	0.210	1.028	1.538	0.000	0.255	185	342.6
	12.88	0.193	0.975	1.431	0.000	0.228	173	342.6
	12.96	0.178	0.926	1.347	0.000	0.210	164	342.6
	13.04	0.164	0.880	1.269	0.000	0.194	155	342.6
	13.12 13.20	0.148 0.138	0.836 0.795	1.192 1.122	0.000	0.178 0.164	146 138	342.6 342.6
	13.28	0.138	0.759	1.061	0.000	0.151	131	342.5
	13.36	0.121	0.729	1.009	0.000	0.140	125	342.5
	13.44	0.113	0.701	0.963	0.000	0.131	120	342.5
	13.52	0.105 0.096	0.676 0.651	0.919 0.876	0.000	0.122	115	342.50 342.50
	13.60 13.68	0.090	0.628	0.838	0.000	0.113 0.105	110 105	342.5
	13.76	0.089	0.608	0.808	0.000	0.100	101	342.5
	13.84	0.087	0.592	0.784	0.000	0.096	98	342.5
	13.92	0.085	0.579	0.764	0.000	0.093	95	342.5
	14.00 14.08	0.082	0.567 0.557	0.747 0.730	0.000	0.090 0.087	92 90	342.5 342.5
	14.16	0.080	0.557	0.730	0.000	0.087	88	342.5
	14.24	0.076	0.537	0.701	0.000	0.082	86	342.5
	14.32	0.074	0.528	0.687	0.000	0.079	84	342.5
	14.40	0.071	0.519	0.673	0.000	0.077	83	342.5
	14.48 14.56	0.069	0.510 0.501	0.660 0.646	0.000	0.075 0.073	81 80	342.5 342.5
	14.64	0.065	0.501	0.633	0.000	0.073	78	342.5
	14.72	0.063	0.484	0.620	0.000	0.068	77	342.50
	14.80	0.061	0.475	0.607	0.000	0.066	76	342.50
	14.88	0.058	0.467	0.594	0.000	0.064	74	342.50
	14 96	0.056	0.458	0.581	0.000	0.062	73	342 40

		14.72 14.80 14.88 14.96	0.063 0.061 0.058 0.056	0.484 0.475 0.467 0.458	0.620 0.607 0.594 0.581	0.000 0.000 0.000 0.000	0.068 0.066 0.064 0.062	77 76 74 73	342.50 342.50 342.50 342.49
	1.750					INFI OW=	1.75 CFS		
	1.500		-		-	114 LOFT-			
	1.250		-		-	OUTFLO	W=1.28 CFS		
(s/ <sub>E</sub> )	1.000								
Flow (H3/s)	0.750		-						
	0.500								
	0.250		-		1	1			
	0.000								
	0.00	2.00 4	1.00 6.0	0 8.00 1	Time (		16.00 18.0	00 20.00	22.00 24.00
		•					Flow (Tota		
		_	Po	na - NOAA	A-FFX 1YR	STORM -	Flow (Tota	(Out)	

Lab	el		Scenario	Return Event	Hydrograph Vo (ft³)	lume	Time to		Peak Flow (ft³/s)
Developed		NOAA-	FFX 2YR STO	(years)	,	7,455		12.08	2.12
Node Sun	nmarv				1	,,,,,,,		12.00	2.12
Lat			Scenario	Return Event (years)	Hydrograph Vo (ft³)	lume	Time to	The Contract	Peak Flow (ft³/s)
Outfall		NOAA-	FFX 2YR STO	RM 2	2	7,411		12.16	1.46
Pond Sun	nmary				1				
Label	Scen	ario	Return Event (years)	Hydrograph Volume (ft³)	Time to Peak (hours)	Peak Fl (ft³/s		Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft³)
Pond (IN)	NOAA-FFX STORM	C 2YR	2	7,455	12.08		2.128	(N/A)	(N/A
Pond (OUT)	NOAA-FF)	2YR	2	7,411	12.16		1.461	343.44	803

342.49 342.49 342.49 342.50 342.50

342.50 342.51

342.51

342.52 342.52 342.53

342.53 342.53 342.54 342.54 342.55 342.56 342.56 342.57

342.58 342.59 342.60 342.61 342.62 342.64 342.65 342.67 342.69 342.73 342.77

342.82 342.90 343.02 343.23 343.44 343.44 343.27

2 YR STORM ROUTING SUMMARY

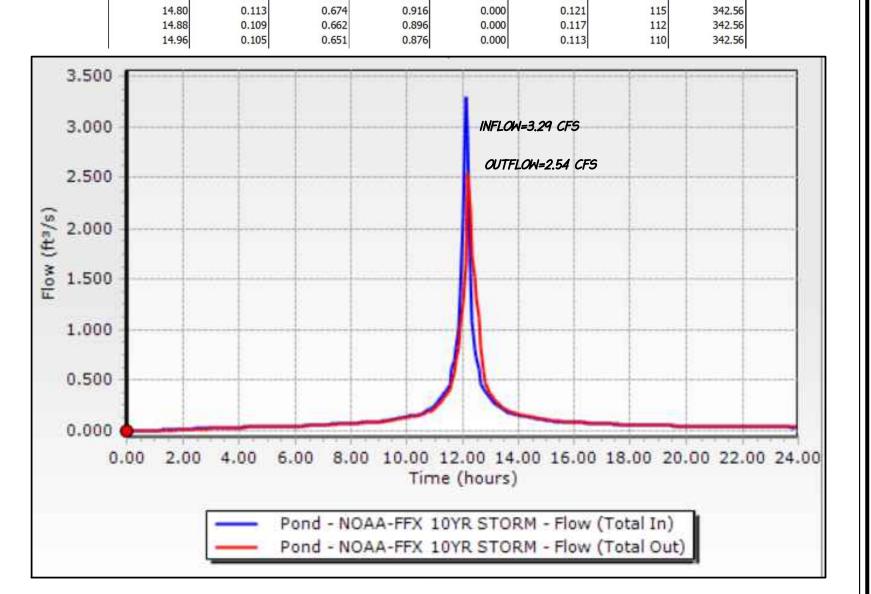
Time (hours)	Flow (Total In) (ft³/s)	2S/t - O (ft³/s)	2S/t + O (ft³/s)	Infiltration (ft³/s)	Flow (Outlet) (ft³/s)	Volume (ft³)
9.04		0.429	0.538	0.000	0.054	6
9.12		0.434	0.545	0.000	0.055	6
9.20 9.28		0.440 0.448	0.554 0.566	0.000	0.057	7
9.36		0.457	0.579	0.000	0.061	7
9.44		0.465	0.592	0.000	0.063	7
9.52	1	0.475	0.606	0.000	0.066	7
9.60		0.484	0.621	0.000	0.068	7
9.68		0.495	0.636	0.000	0.071	7
9.76	A 52.000000	0.505	0.652	0.000	0.073	8
9.84 9.92		0.515 0.525	0.667 0.682	0.000	0.076 0.079	8
10.00		0.535	0.698	0.000	0.081	8
10.08		0.546	0.714	0.000	0.084	8
10.16		0.559	0.734	0.000	0.087	9
10.24	0.103	0.576	0.759	0.000	0.092	9
10.32		0.583	0.769	0.000	0.093	9
10.40		0.585	0.773	0.000	0.094	9
10.48		0.595	0.788	0.000	0.097	9
10.56 10.64	1.07-101.030	0.609	0.809	0.000	0.100 0.105	10 10
10.72	225 5 48 25 2	0.651	0.876	0.000	0.113	11
10.80	1 200 (000)	0.676	0.920	0.000	0.122	11
10.88		0.704	0.966	0.000	0.131	12
10.96	0.160	0.731	1.013	0.000	0.141	12
11.04		0.759	1.062	0.000	0.151	13
11.12		0.793	1.119	0.000	0.163	13
11.20 11.28		0.835 0.883	1.191	0.000	0.178	14
11.26		0.883	1.273 1.358	0.000	0.195 0.213	15 16
11.44		0.981	1.443	0.000	0.231	17
11.52		1.023	1.529	0.000	0.253	18
11.60	0.398	1.110	1.706	0.000	0.298	20
11.68		1.231	1.954	0.000	0.362	22
11.76	1	1.342	2.203	0.000	0.430	25
11.84 11.92	1	1.479 1.667	2.528 3.010	0.000	0.525 0.672	28 33
12.00	1	2.006	3.906	0.000	0.950	42
12.08	1	3.014	5.502	0.000	1.244	61
12.16		4.109	7.031	0.000	1.461	80
12.24		4.091	7.006	0.000	1.458	79
12.32		3.222	5.799	0.000	1.288	64
12.40		2.300	4.444	0.000	1.072	48
12.48 12.56	-30000-000	1.770 1.520	3.287 2.626	0.000	0.759 0.553	36 29
12.64		1.347	2.213	0.000	0.433	25
12.72		1.216	1.924	0.000	0.354	22
12.80	0.254	1.129	1.746	0.000	0.308	20
12.88	1	1.067	1.618	0.000	0.276	19
12.96		1.018	1.518	0.000	0.250	18
13.04 13.12	397 5 5 5 5 5	0.976 0.931	1.433 1.355	0.000	0.228 0.212	17 16
13.20		0.886	1.278	0.000	0.196	15
13.28		0.846	1.210	0.000	0.182	14
13.36		0.810	1.149	0.000	0.169	14
13.44		0.778	1.094	0.000	0.158	13
13.52		0.748	1.042	0.000	0.147	12
13.60	I	0.718	0.991	0.000	0.137	12
13.68 13.76		0.691 0.671	0.945 0.910	0.000	0.127 0.120	11 11
13.84		0.655	0.884	0.000	0.114	11
13.92	100000000000000000000000000000000000000	0.644	0.864	0.000	0.110	10
14.00	0.100	0.633	0.846	0.000	0.107	10
14.08		0.623	0.831	0.000	0.104	10
14.16		0.613	0.815	0.000	0.101	10
14.24		0.603	0.800	0.000	0.099	10
14.32 14.40		0.592 0.582	0.784 0.769	0.000	0.096 0.093	9
14.48	1	0.571	0.769	0.000	0.093	9
14.56	1	0.561	0.737	0.000	0.088	9
14.64		0.551	0.721	0.000	0.085	8
14.72		0.540	0.706	0.000	0.083	8
14.80		0.530	0.690	0.000	0.080	8

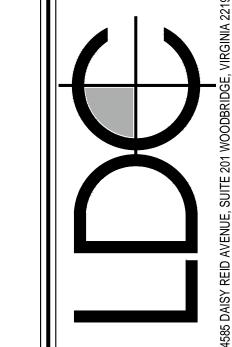
		12.32	0.523	2.300	4.444	0.000	1.072	486	343.27
		12.48 12.56	0.465 0.391	1.770 1.520	3.287 2.626	0.000	0.759 0.553	364 299	342.94 342.84
		12.64	0.302	1.347	2.213	0.000	0.433	256	342.77
		12.72	0.276	1.216	1.924	0.000	0.354	226	342.73
		12.80 12.88	0.254	1.129 1.067	1.746 1.618	0.000	0.308 0.276	207 193	342.70 342.68
		12.96	0.217	1.018	1.518	0.000	0.250	183	342.66
		13.04	0.199	0.976 0.931	1.433 1.355	0.000	0.228 0.212	173 165	342.65
		13.12 13.20	0.180 0.167	0.931	1.278	0.000	0.196	156	342.64 342.62
		13.28	0.156	0.846	1.210	0.000	0.182	148	342.61
		13.36	0.147	0.810	1.149	0.000	0.169	141	342.60
		13.4 <del>4</del> 13.52	0.137 0.127	0.778 0.748	1.094 1.042	0.000	0.158 0.147	135 129	342.59 342.58
		13.60	0.116	0.718	0.991	0.000	0.137	123	342.58
		13.68	0.111	0.691	0.945	0.000	0.127	118	342.57
		13.76 13.84	0.108	0.671 0.655	0.910 0.884	0.000	0.120 0.114	114 111	342.56 342.56
		13.92	0.103	0.644	0.864	0.000	0.110	109	342.55
		14.00	0.100	0.633	0.846	0.000	0.107	107	342.55
		14.08 14.16	0.097	0.623 0.613	0.831 0.815	0.000	0.104	104 102	342.55 342.54
		14.24	0.092	0.603	0.800	0.000	0.099	100	342.54
		14.32	0.089	0.592	0.784	0.000	0.096	98	342.54
		14.40 14.48	0.087	0.582 0.571	0.769 0.753	0.000	0.093	95 93	342.53 342.53
		14.56	0.081	0.561	0.737	0.000	0.088	91	342.53
		14.64 14.72	0.079	0.551 0.540	0.721 0.706	0.000	0.085	89 86	342.52 342.52
		14.80	0.078	0.530	0.690	0.000	0.083	85	342.52
		14.88	0.071	0.519	0.674	0.000	0.077	83	342.51
		14.96	0.068	0.509	0.658	0.000	0.075	81	342.51
	2.250			7			4		
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	2.000			-		INFLOV	N=2.13 CFS		
		1							1 1
	1.750						1,000		
	1.500			1		OUTF	FLOW=1.46 CFS		
Flow (ft3/s)	. 250								
#	1.250								
2	1.000								
0	1,000								
	0.750								
	01700								
	0.500								
	0.500					/ /			
	0.250								
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	0.000								
	CONTRACTOR OF THE PARTY OF THE	0 200	4.00	00 00	0 10 00	12.00.1	1 00 16 00	19 00 0	0.00.22.00
	0.0	0 2.00	4.00	5.00 8.0				18.00 2	0.00 22.00
					Lim	e (hours	1		
									_
				Pond - NO	DAA-FFX :	2YR STO	RM - Flow (	Total In)	
							RM - Flow (		

Label			Scenario		Return Event (years)	Hydrograph Vo (ft³)		e to Peak nours)	Peak Flow (ft³/s)
Developed		NOAA-F	FX 10YR ST	ORM	10	1	1,760	12.08	3.294
Node Sumi	mary						'		
Label		w-	Scenario		Return Event (years)	Hydrograph Vo (ft³)		e to Peak nours)	Peak Flow (ft³/s)
Outfall		NOAA-F	FX 10YR ST	ORM	10	1	1,702	12.16	2.543
Pond Sumr	mary								
Label	Scena	ario	Return Event (years)	Vo	rograph olume (ft³)	Time to Peak (hours)	Peak Flow (ft³/s)	Maximum Water Surface Elevation (ft)	Maximum Pond Storage (ft³)
Pond (IN)	NOAA-FFX	10YR	10		11,760	12.08	3.29	4 (N/A)	(N/A)

	-			Event (years)	(ft³)		(ho	ours)	(ft³/s)
Developed		NOAA-F	FX 10YR STO	DRM 1	0 :	11,760		12.08	3.29
Node Sun	nmary								
Lab	el	· 1	Scenario	Return Event (years)	Hydrograph Vo (ft³)	olume		to Peak ours)	Peak Flow (ft³/s)
Outfall		NOAA-F	FX 10YR STO	DRM 1	0 :	11,702		12.16	2.54
Pond Sun	nmary	•				•		•	
Label	Scena	ario	Return Event (years)	Hydrograph Volume (ft³)	Time to Peak (hours)		k Flow t³/s)	Maximum Water Surface Elevation (ft)	Maximum Pone Storage (ft³)
Pond (IN)	NOAA-FF) STORM	( 10YR	10	11,760	12.08		3.294	(N/A)	(N/A
Pond (OUT)	NOAA-FF) STORM	( 10YR	10	11,702	12.16		2.543	344.08	1,27

		Pond Ro	outing Calcu	lations (Tot	al Out)		
Time (hours)	Flow (Total In) (ft³/s)	2S/t - O (ft³/s)	2S/t + O (ft³/s)	Infiltration (ft³/s)	Flow (Outlet) (ft³/s)	Volume (ft³)	Elevation (ft)
9.04	0.092	0.563	0.740	0.000	0.088	91	342.53
9.12 9.20	0.095 0.100	0.570 0.580	0.750 0.765	0.000	0.090 0.093	93 95	342.53 342.53
9.28	0.104	0.592	0.783	0.000	0.095	98	342.54
9.36	0.108	0.605	0.804	0.000	0.099	100	342.54
9.44	0.112	0.619	0.825	0.000	0.103	103	342.55
9.52	0.116	0.633	0.846	0.000	0.107	107	342.55
9.60 9.68	0.120 0.124	0.647 0.660	0.869 0.892	0.000	0.111	109 112	342.55
9.76	0.124	0.672	0.892	0.000	0.116 0.120	112	342.56 342.56
9.84	0.132	0.684	0.932	0.000	0.124	116	342.57
9.92	0.137	0.695	0.952	0.000	0.129	119	342.57
10.00	0.141	0.707	0.973	0.000	0.133	121	342.57
10.08 10.16	0.145 0.156	0.719 0.735	0.993 1.020	0.000	0.137 0.143	123 126	342.58 342.58
10.16	0.164	0.755	1.054	0.000	0.143	130	342.59
10.32	0.145	0.761	1.064	0.000	0.152	131	342.59
10.40	0.158	0.760	1.063	0.000	0.152	131	342.59
10.48	0.165	0.772	1.084	0.000	0.156	134	342.59
10.56 10.64	0.175 0.189	0.789 0.813	1.113 1.154	0.000	0.162 0.170	137 142	342.60 342.60
10.72	0.205	0.813	1.207	0.000	0.170	148	342.61
10.80	0.222	0.882	1.271	0.000	0.194	155	342.62
10.88	0.237	0.923	1.341	0.000	0.209	163	342.63
10.96	0.252	0.965	1.412	0.000	0.224	171	342.65
11.04 11.12	0.270 0.298	1.003 1.043	1.487 1.570	0.000	0.242 0.263	179 188	342.66 342.67
11.20	0.329	1.092	1.671	0.000	0.289	199	342.69
11.28	0.359	1.146	1.781	0.000	0.317	211	342.71
11.36	0.387	1.201	1.893	0.000	0.346	223	342.72
11.44	0.414	1.255	2.002	0.000	0.374	234	342.74
11.52 11.60	0.446 0.622	1.305 1.414	2.114 2.373	0.000	0.404	246 273	342.76 342.80
11.68	0.698	1.564	2.734	0.000	0.585	309	342.85
11.76	0.820	1.693	3.081	0.000	0.694	344	342.91
11.84	1.028	1.865	3.540	0.000	0.837	388	342.97
11.92 12.00	1.355 2.123	2.171 3.115	4.248 5.649	0.000	1.039 1.267	462 631	343.07 343.25
12.08	3.294	5.192	8.533	0.000	1.670	988	343.67
12.16	2.921	6.321	11.407	0.000	2.543	1,276	344.08
12.24	1.558	6.418	10.799	0.000	2.191	1,244	344.02
12.32	1.079	5.570	9.055	0.000	1.743	1,053 857	343.76
12.40 12.48	0.806 0.718	4.419 3.324	7.455 5.943	0.000	1.518 1.309	667	343.50 343.29
12.56	0.604	2.432	4.646	0.000	1.107	510	343.12
12.64	0.466	1.851	3.502	0.000	0.826	384	342.97
12.72	0.425	1.567	2.742	0.000	0.588	310	342.86
12.80 12.88	0.392 0.362	1.418 1.329	2.384 2.172	0.000	0.483 0.421	274 252	342.80 342.77
12.96	0.334	1.266	2.025	0.000	0.380	237	342.75
13.04	0.306	1.207	1.906	0.000	0.349	224	342.73
13.12	0.277	1.151	1.791	0.000	0.320	212	342.71
13.20 13.28	0.258 0.241	1.100 1.057	1.686 1.599	0.000	0.293 0.271	201 191	342.69 342.68
13.36	0.241	1.021	1.525	0.000	0.271	183	342.67
13.44	0.211	0.989	1.459	0.000	0.235	176	342.65
13.52	0.195	0.955	1.396	0.000	0.220	169	342.64
13.60	0.179	0.916	1.329	0.000	0.207	162	342.63
13.68 13.76	0.171 0.166	0.879 0.850	1.266 1.216	0.000	0.193 0.183	154 149	342.62 342.61
13.84	0.163	0.828	1.179	0.000	0.175	144	342.61
13.92	0.159	0.811	1.149	0.000	0.169	141	342.60
14.00	0.154	0.795	1.123	0.000	0.164	138	342.60
14.08 14.16	0.150 0.146	0.781 0.768	1.099 1.077	0.000	0.159 0.154	135 133	342.59 342.59
14.16	0.146	0.768	1.077	0.000	0.154	133	342.59
14.32	0.138	0.745	1.036	0.000	0.146	128	342.58
14.40	0.133	0.733	1.016	0.000	0.142	126	342.58
14.48	0.129	0.721	0.995	0.000	0.137	124	342.58
14.56 14.64	0.125 0.121	0.709 0.697	0.975 0.956	0.000	0.133 0.129	121 119	342.57 342.57
14.72	0.121	0.686	0.936	0.000	0.125	117	342.57
14.80					0.121		







SHEET 19 OF 25





# MILL STREET TOWN OF VIENNA, VA

### **SC-310 STORMTECH CHAMBER SPECIFICATIONS**

CHAMBERS SHALL BE STORMTECH SC-310.

FOR IMPACT AND MULTIPLE VEHICLE PRESENCES.

POLYETHYLENE COPOLYMERS.

- CHAMBERS SHALL BE ARCH-SHAPED AND SHALL BE MANUFACTURED FROM VIRGIN, IMPACT-MODIFIED POLYPROPYLENE OR
- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2922 (POLETHYLENE) OR ASTM F2418-16a (POLYPROPYLENE), "STANDARD SPECIFICATION FOR CORRUGATED WALL STORMWATER COLLECTION CHAMBERS".
- CHAMBER ROWS SHALL PROVIDE CONTINUOUS, UNOBSTRUCTED INTERNAL SPACE WITH NO INTERNAL SUPPORTS THAT WOULD
- IMPEDE FLOW OR LIMIT ACCESS FOR INSPECTION. 5. THE STRUCTURAL DESIGN OF THE CHAMBERS. THE STRUCTURAL BACKFILL, AND THE INSTALLATION REQUIREMENTS SHALL ENSURE THAT THE LOAD FACTORS SPECIFIED IN THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS, SECTION 12.12, ARE MET FOR: 1) LONG-DURATION DEAD LOADS AND 2) SHORT-DURATION LIVE LOADS, BASED ON THE AASHTO DESIGN TRUCK WITH CONSIDERATION
- CHAMBERS SHALL BE DESIGNED, TESTED AND ALLOWABLE LOAD CONFIGURATIONS DETERMINED IN ACCORDANCE WITH ASTM F2787, "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION CHAMBERS". LOAD CONFIGURATIONS SHALL INCLUDE: 1) INSTANTANEOUS (<1 MIN) AASHTO DESIGN TRUCK LIVE LOAD ON MINIMUM COVER 2) MAXIMUM PERMANENT (75-YR) COVER LOAD AND 3) ALLOWABLE COVER WITH PARKED (1-WEEK) AASHTO DESIGN TRUCK.
- REQUIREMENTS FOR HANDLING AND INSTALLATION: TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING
- STACKING LUGS TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS
- . TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN
- SECTION 6.2.8 OF ASTM F2922 SHALL BE GREATER THAN OR EQUAL TO 400 LBS/IN/IN. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.
- 8. ONLY CHAMBERS THAT ARE APPROVED BY THE SITE DESIGN ENGINEER WILL BE ALLOWED. UPON REQUEST BY THE SITE DESIGN ENGINEER OR OWNER, THE CHAMBER MANUFACTURER SHALL SUBMIT A STRUCTURAL EVALUATION FOR APPROVAL BEFORE
- DELIVERING CHAMBERS TO THE PROJECT SITE AS FOLLOWS: THE STRUCTURAL EVALUATION SHALL BE SEALED BY A REGISTERED PROFESSIONAL ENGINEER.
- THE STRUCTURAL EVALUATION SHALL DEMONSTRATE THAT THE SAFETY FACTORS ARE GREATER THAN OR EQUAL TO 1.95 FOR DEAD LOAD AND 1.75 FOR LIVE LOAD, THE MINIMUM REQUIRED BY ASTM F2787 AND BY SECTIONS 3 AND 12.12 OF THE AASHTO LRFD BRIDGE DESIGN SPECIFICATIONS FOR THERMOPLASTIC PIPE.
- THE TEST DERIVED CREEP MODULUS AS SPECIFIED IN ASTM F2922 SHALL BE USED FOR PERMANENT DEAD LOAD DESIGN EXCEPT THAT IT SHALL BE THE 75-YEAR MODULUS USED FOR DESIGN.

CHAMBERS AND END CAPS SHALL BE PRODUCED AT AN ISO 9001 CERTIFIED MANUFACTURING FACILITY.

### IMPORTANT - NOTES FOR THE BIDDING AND INSTALLATION OF THE SC-310 SYSTEM

- 1. STORMTECH SC-310 CHAMBERS SHALL NOT BE INSTALLED UNTIL THE MANUFACTURER'S REPRESENTATIVE HAS COMPLETED A PRE-CONSTRUCTION MEETING WITH THE INSTALLERS.
- 2. STORMTECH SC-310 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
- CHAMBERS ARE NOT TO BE BACKFILLED WITH A DOZER OR AN EXCAVATOR SITUATED OVER THE CHAMBERS. STORMTECH RECOMMENDS 3 BACKFILL METHODS:
- STONESHOOTER LOCATED OFF THE CHAMBER BED
- BACKFILL AS ROWS ARE BUILT USING AN EXCAVATOR ON THE FOUNDATION STONE OR SUBGRADE.
   BACKFILL FROM OUTSIDE THE EXCAVATION USING A LONG BOOM HOE OR EXCAVATOR.
- THE FOUNDATION STONE SHALL BE LEVELED AND COMPACTED PRIOR TO PLACING CHAMBERS.

5. JOINTS BETWEEN CHAMBERS SHALL BE PROPERLY SEATED PRIOR TO PLACING STONE.

- 6. MAINTAIN MINIMUM 6" (150 mm) SPACING BETWEEN THE CHAMBER ROWS.
- EMBEDMENT STONE SURROUNDING CHAMBERS MUST BE A CLEAN, CRUSHED, ANGULAR STONE 3/4-2" (20-50 mm).
- 8. THE CONTRACTOR MUST REPORT ANY DISCREPANCIES WITH CHAMBER FOUNDATION MATERIALS BEARING CAPACITIES TO THE SITE DESIGN
- 9. ADS RECOMMENDS THE USE OF "FLEXSTORM CATCH IT" INSERTS DURING CONSTRUCTION FOR ALL INLETS TO PROTECT THE SUBSURFACE
- STORMWATER MANAGEMENT SYSTEM FROM CONSTRUCTION SITE RUNOFF.

CORRUGATION VALLEY.

PROVIDED BY ADS).

2. ALL SCHEDULE 40 FITTINGS TO BE SOLVENT CEMENTED (4" PVC NOT

- NOTES FOR CONSTRUCTION EQUIPMENT STORMTECH SC-310 CHAMBERS SHALL BE INSTALLED IN ACCORDANCE WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
- 2. THE USE OF CONSTRUCTION EQUIPMENT OVER SC-310 & SC-740 CHAMBERS IS LIMITED:
- NO EQUIPMENT IS ALLOWED ON BARE CHAMBERS NO RUBBER TIRED LOADERS, DUMP TRUCKS, OR EXCAVATORS ARE ALLOWED UNTIL PROPER FILL DEPTHS ARE REACHED IN ACCORDANCE
- WITH THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE" WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT CAN BE FOUND IN THE "STORMTECH SC-310/SC-740/DC-780 CONSTRUCTION GUIDE".
- 3. FULL 36" (900 mm) OF STABILIZED COVER MATERIALS OVER THE CHAMBERS IS REQUIRED FOR DUMP TRUCK TRAVEL OR DUMPING.

USE OF A DOZER TO PUSH EMBEDMENT STONE BETWEEN THE ROWS OF CHAMBERS MAY CAUSE DAMAGE TO THE CHAMBERS AND IS NOT AN ACCEPTABLE BACKFILL METHOD. ANY CHAMBERS DAMAGED BY THE "DUMP AND PUSH" METHOD ARE NOT COVERED UNDER THE STORMTECH

CONTACT STORMTECH AT 1-888-892-2694 WITH ANY QUESTIONS ON INSTALLATION REQUIREMENTS OR WEIGHT LIMITS FOR CONSTRUCTION EQUIPMENT.

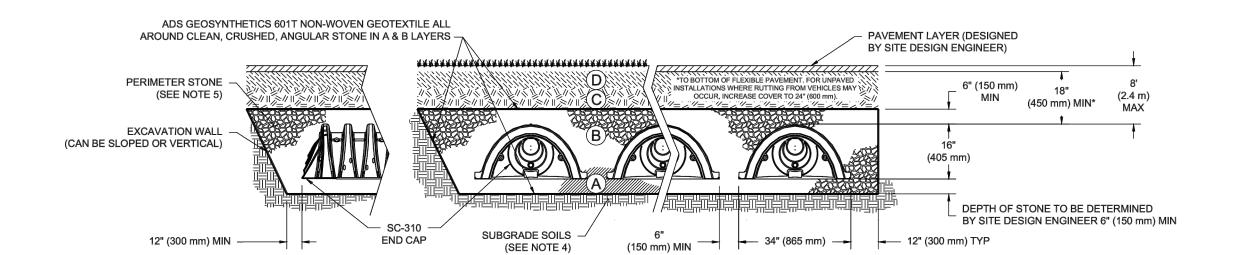
### **ACCEPTABLE FILL MATERIALS: STORMTECH SC-310 CHAMBER SYSTEMS**

	MATERIAL LOCATION	DESCRIPTION	AASHTO MATERIAL CLASSIFICATIONS	COMPACTION / DENSITY REQUIREMENT
D	FINAL FILL: FILL MATERIAL FOR LAYER 'D' STARTS FROM THE TOP OF THE 'C' LAYER TO THE BOTTOM OF FLEXIBLE PAVEMENT OR UNPAVED FINISHED GRADE ABOVE. NOTE THAT PAVEMENT SUBBASE MAY BE PART OF THE 'D' LAYER.	ANY SOIL/ROCK MATERIALS, NATIVE SOILS, OR PER ENGINEER'S PLANS. CHECK PLANS FOR PAVEMENT SUBGRADE REQUIREMENTS.	N/A	PREPARE PER SITE DESIGN ENGINEER'S PLANS. PAVED INSTALLATIONS MAY HAVE STRINGENT MATERIAL AND PREPARATION REQUIREMENTS.
С	INITIAL FILL: FILL MATERIAL FOR LAYER 'C' STARTS FROM THE TOP OF THE EMBEDMENT STONE ('B' LAYER) TO 18" (450 mm) ABOVE THE TOP OF THE CHAMBER. NOTE THAT PAVEMENT SUBBASE MAY BE A PART OF THE 'C' LAYER.	GRANULAR WELL-GRADED SOIL/AGGREGATE MIXTURES, <35% FINES OR PROCESSED AGGREGATE.  MOST PAVEMENT SUBBASE MATERIALS CAN BE USED IN LIEU OF THIS LAYER.	AASHTO M145 <sup>1</sup> A-1, A-2-4, A-3 OR AASHTO M43 <sup>1</sup> 3, 357, 4, 467, 5, 56, 57, 6, 67, 68, 7, 78, 8, 89, 9, 10	BEGIN COMPACTIONS AFTER 12" (300 mm) OF MATERIAL OVER THE CHAMBERS IS REACHED. COMPACT ADDITIONAL LAYERS IN 6" (150 mm) MAX LIFTS TO A MIN. 95% PROCTOR DENSITY FOR WELL GRADED MATERIAL AND 95% RELATIVE DENSITY FOR PROCESSED AGGREGATE MATERIALS. ROLLER GROSS VEHICLE WEIGHT NOT TO EXCEED 12,000 lbs (53 kN). DYNAMIC FORCE NOT TO EXCEED 20,000 lbs (89 kN).
В	EMBEDMENT STONE: FILL SURROUNDING THE CHAMBERS FROM THE FOUNDATION STONE ('A' LAYER) TO THE 'C' LAYER ABOVE.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43¹ 3, 357, 4, 467, 5, 56, 57	NO COMPACTION REQUIRED.
A	FOUNDATION STONE: FILL BELOW CHAMBERS FROM THE SUBGRADE UP TO THE FOOT (BOTTOM) OF THE CHAMBER.	CLEAN, CRUSHED, ANGULAR STONE	AASHTO M43¹ 3, 357, 4, 467, 5, 56, 57	PLATE COMPACT OR ROLL TO ACHIEVE A FLAT SURFACE. <sup>2,3</sup>

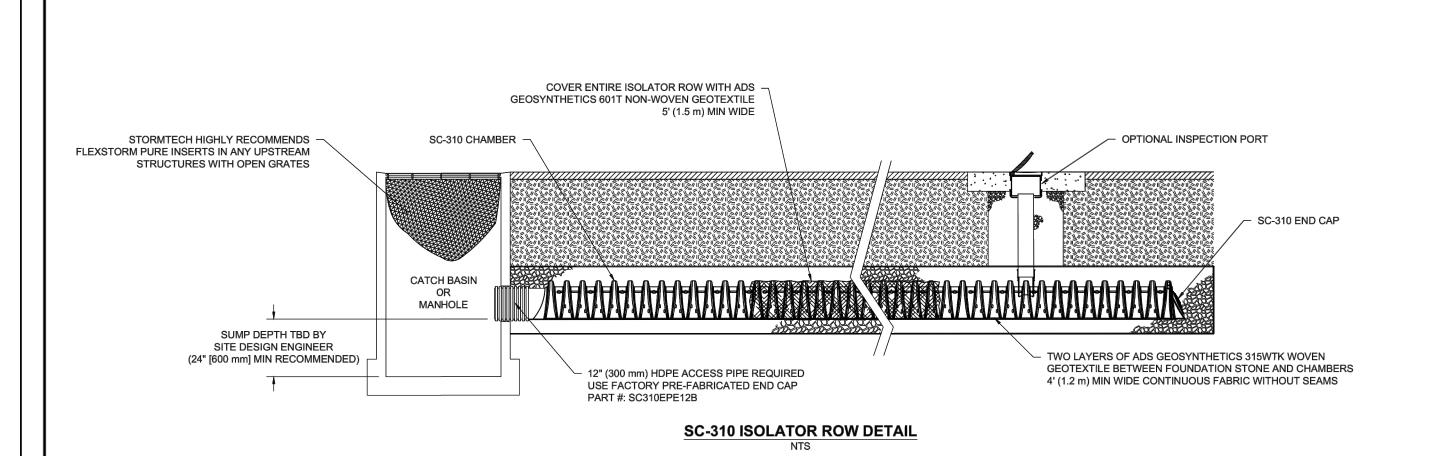
THE LISTED AASHTO DESIGNATIONS ARE FOR GRADATIONS ONLY. THE STONE MUST ALSO BE CLEAN, CRUSHED, ANGULAR. FOR EXAMPLE, A SPECIFICATION FOR #4 STONE WOULD STATE: "CLEAN, CRUSHED, ANGULAR NO. 4 (AASHTO M43) STONE".

STORMTECH COMPACTION REQUIREMENTS ARE MET FOR 'A' LOCATION MATERIALS WHEN PLACED AND COMPACTED IN 6" (150 mm) (MAX) LIFTS USING TWO FULL COVERAGES WITH A VIBRATORY COMPACTOR. WHERE INFILTRATION SURFACES MAY BE COMPROMISED BY COMPACTION, FOR STANDARD DESIGNS, CONTACT STORMTECH FOR COMPACTION REQUIREMENTS

ONCE LAYER 'C' IS PLACED, ANY SOIL/MATERIAL CAN BE PLACED IN LAYER 'D' UP TO THE FINISHED GRADE. MOST PAVEMENT SUBBASE SOILS CAN BE USED TO REPLACE THE MATERIAL REQUIREMENTS OF LAYER 'C' OR 'D' AT THE SITE DESIGN ENGINEER'S DISCRETION.



- CHAMBERS SHALL MEET THE REQUIREMENTS OF ASTM F2922 (POLETHYLENE) OR ASTM F2418-16a (POLYPROPYLENE), "STANDARD SPECIFICATION FOR CORRUGATED WALL STORMWATER COLLECTION
- SC-310 CHAMBERS SHALL BE DESIGNED IN ACCORDANCE WITH ASTM F2787 "STANDARD PRACTICE FOR STRUCTURAL DESIGN OF THERMOPLASTIC CORRUGATED WALL STORMWATER COLLECTION
- THE SITE DESIGN ENGINEER IS RESPONSIBLE FOR ASSESSING THE BEARING RESISTANCE (ALLOWABLE BEARING CAPACITY) OF THE SUBGRADE SOILS AND THE DEPTH OF FOUNDATION STONE WITH CONSIDERATION FOR THE RANGE OF EXPECTED SOIL MOISTURE CONDITIONS.
- PERIMETER STONE MUST BE EXTENDED HORIZONTALLY TO THE EXCAVATION WALL FOR BOTH VERTICAL AND SLOPED EXCAVATION WALLS.
- REQUIREMENTS FOR HANDLING AND INSTALLATION:
- TO MAINTAIN THE WIDTH OF CHAMBERS DURING SHIPPING AND HANDLING, CHAMBERS SHALL HAVE INTEGRAL, INTERLOCKING STACKING LUGS. TO ENSURE A SECURE JOINT DURING INSTALLATION AND BACKFILL, THE HEIGHT OF THE CHAMBER JOINT SHALL NOT BE LESS THAN 2".
- TO ENSURE THE INTEGRITY OF THE ARCH SHAPE DURING INSTALLATION, a) THE ARCH STIFFNESS CONSTANT AS DEFINED IN SECTION 6.2.8 OF ASTM F2922 SHALL BE GREATER THAN OR EQUAL TO 400 LBS/IN/IN. AND b) TO RESIST CHAMBER DEFORMATION DURING INSTALLATION AT ELEVATED TEMPERATURES (ABOVE 73° F / 23° C), CHAMBERS SHALL BE PRODUCED FROM REFLECTIVE GOLD OR YELLOW COLORS.

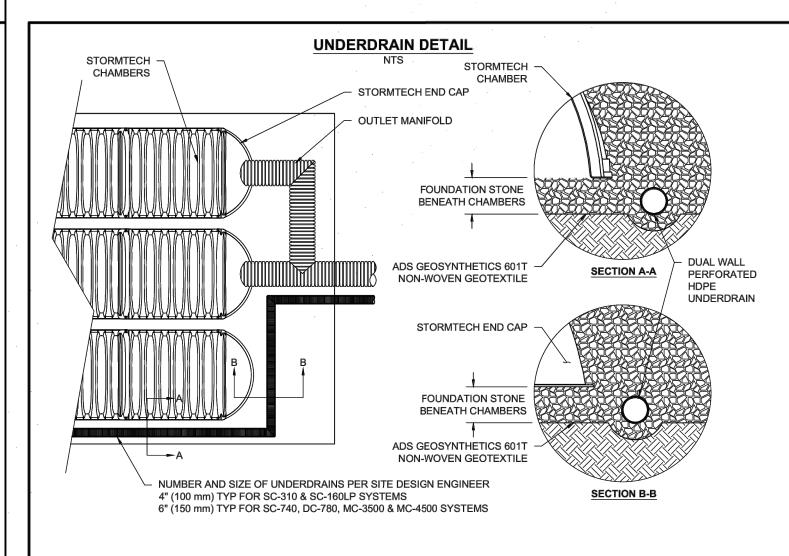


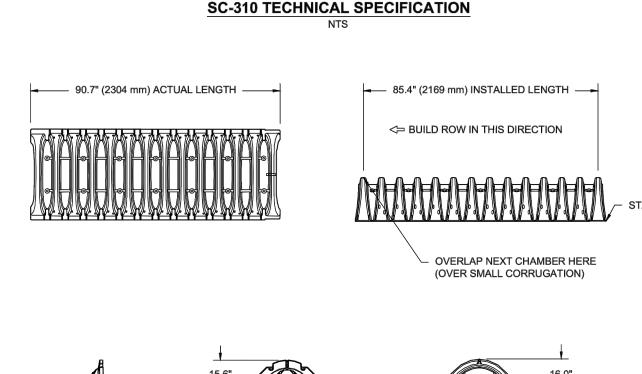
### **INSPECTION & MAINTENANCE**

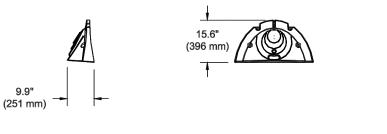
- STEP 1) INSPECT ISOLATOR ROW FOR SEDIMENT A. INSPECTION PORTS (IF PRESENT)
  - A.1. REMOVE/OPEN LID ON NYLOPLAST INLINE DRAIN A.2. REMOVE AND CLEAN FLEXSTORM FILTER IF INSTALLED
  - A.3. USING A FLASHLIGHT AND STADIA ROD, MEASURE DEPTH OF SEDIMENT AND RECORD ON MAINTENANCE LOG
  - A.4. LOWER A CAMERA INTO ISOLATOR ROW FOR VISUAL INSPECTION OF SEDIMENT LEVELS (OPTIONAL) A.5. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3. B. ALL ISOLATOR ROWS
  - B.1. REMOVE COVER FROM STRUCTURE AT UPSTREAM END OF ISOLATOR ROW B.2. USING A FLASHLIGHT, INSPECT DOWN THE ISOLATOR ROW THROUGH OUTLET PIPE
  - ) MIRRORS ON POLES OR CAMERAS MAY BE USED TO AVOID A CONFINED SPACE ENTRY i) FOLLOW OSHA REGULATIONS FOR CONFINED SPACE ENTRY IF ENTERING MANHOLE B.3. IF SEDIMENT IS AT, OR ABOVE, 3" (80 mm) PROCEED TO STEP 2. IF NOT, PROCEED TO STEP 3.
- STEP 2) CLEAN OUT ISOLATOR ROW USING THE JETVAC PROCESS A. A FIXED CULVERT CLEANING NOZZLE WITH REAR FACING SPREAD OF 45" (1.1 m) OR MORE IS PREFERRED
  - . APPLY MULTIPLE PASSES OF JETVAC UNTIL BACKFLUSH WATER IS CLEAN VACUUM STRUCTURE SUMP AS REQUIRED
- STEP 3) REPLACE ALL COVERS, GRATES, FILTERS, AND LIDS; RECORD OBSERVATIONS AND ACTIONS.
- STEP 4) INSPECT AND CLEAN BASINS AND MANHOLES UPSTREAM OF THE STORMTECH SYSTEM.

- INSPECT EVERY 6 MONTHS DURING THE FIRST YEAR OF OPERATION. ADJUST THE INSPECTION INTERVAL BASED ON PREVIOUS OBSERVATIONS OF SEDIMENT ACCUMULATION AND HIGH WATER ELEVATIONS.
- CONDUCT JETTING AND VACTORING ANNUALLY OR WHEN INSPECTION SHOWS THAT MAINTENANCE IS NECESSARY.
- 12" (300 mm) MIN WIDTH CONCRETE COLLAR NOT REQUIRED FOR UNPAVED APPLICATIONS CONCRETE COLLAR 8" NYLOPLAST INSPECTION PORT PAVEMENT BODY (PART# 2708AG4IPKIT) OR TRAFFIC RATED BOX W/SOLID LOCKING COVER SCHED 40 PVC CONCRETE SLAB 4" (100 mm) SCHED 40 PVC 6" (150 mm) MIN THICKNESS SCHED 40 PVC COUPLING STORMTECH CHAMBER -SCHED 40 PVC CORE 4.5" (114 mm) Ø HOLE IN CHAMBER (4.5" HOLE SAW REQ'D) ANY VALLEY LOCATION CONNECTION DETAIL 1. INSPECTION PORTS MAY BE CONNECTED THROUGH ANY CHAMBER

4" PVC INSPECTION PORT DETAIL

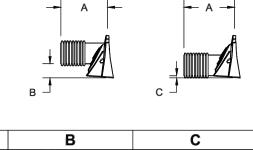






34.0" X 16.0" X 85.4" (864 mm X 406 mm X 2169 mm) CHAMBER STORAGE 14.7 CUBIC FEET MINIMUM INSTALLED STORAGE 31.0 CUBIC FEET (0.88 m<sup>3</sup>) 35.0 lbs. (16.8 kg)

\*ASSUMES 6" (152 mm) ABOVE, BELOW, AND BETWEEN CHAMBERS

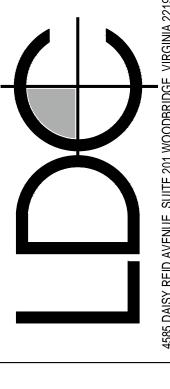


STUBS AT BOTTOM OF END CAP FOR PART NUMBERS ENDING WITH "B" STUBS AT TOP OF END CAP FOR PART NUMBERS ENDING WITH "T"

PART#	STUB	Α	В	С
SC310EPE06T / SC310EPE06TPC	6" (150 mm)	9.6" (244 mm)	5.8" (147 mm)	
SC310EPE06B / SC310EPE06BPC	0 (130 11111)	9.0 (244 11111)		0.5" (13 mm)
SC310EPE08T / SC310EPE08TPC	8" (200 mm)	11.9" (302 mm)	3.5" (89 mm)	
SC310EPE08B / SC310EPE08BPC	0 (200 111111)	11.9 (302 11111)		0.6" (15 mm)
SC310EPE10T / SC310EPE10TPC	10" (250 mm)	12.7" (323 mm)	1.4" (36 mm)	
SC310EPE10B / SC310EPE10BPC	10" (250 mm)	12.1 (323 11111)		0.7" (18 mm)
SC310EPE12B	12" (300 mm)	13.5" (343 mm)		0.9" (23 mm)

ALL STUBS, EXCEPT FOR THE SC310EPE12B ARE PLACED AT BOTTOM OF END CAP SUCH THAT THE OUTSIDE DIAMETER OF THE STUB IS FLUSH WITH THE BOTTOM OF THE END CAP. FOR ADDITIONAL INFORMATION CONTACT STORMTECH AT

\* FOR THE SC310EPE12B THE 12" (300 mm) STUB LIES BELOW THE BOTTOM OF THE END CAP APPROXIMATELY 0.25" (6 mm). BACKFILL MATERIAL SHOULD BE REMOVED FROM BELOW THE N-12 STUB SO THAT THE FITTING SITS LEVEL. NOTE: ALL DIMENSIONS ARE NOMINAL



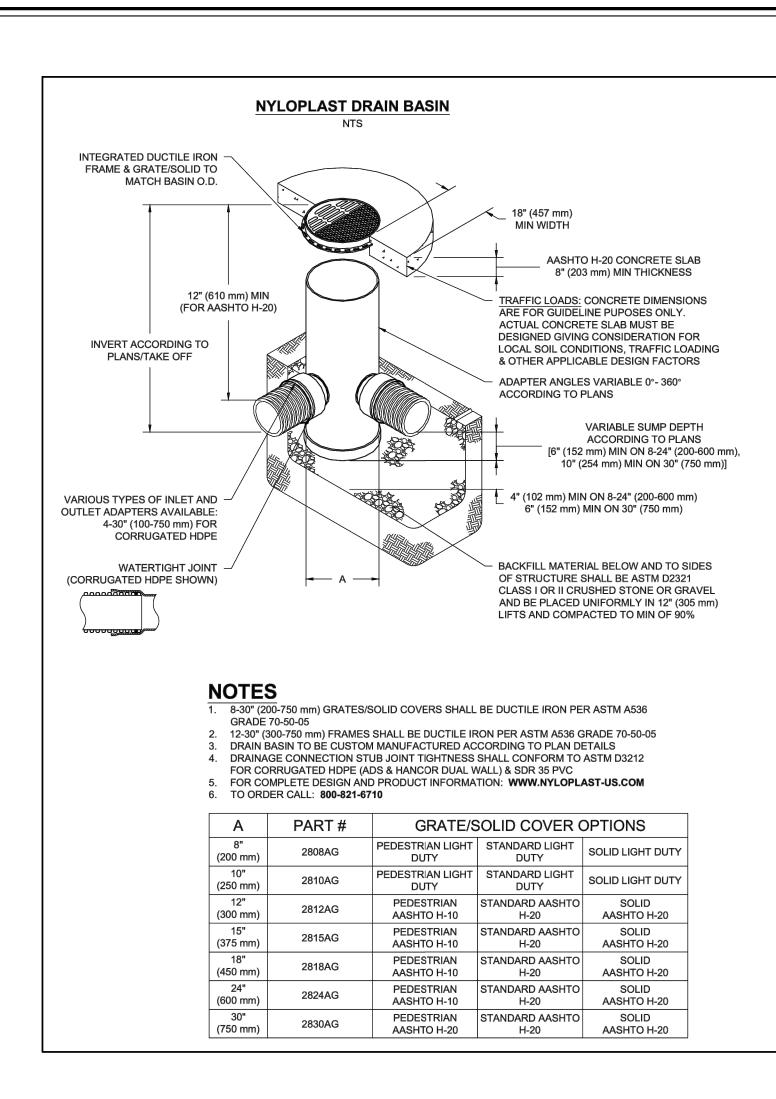
I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS

SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.

SCALE.

OF 25 AUGUST 2, 2019

DRAFT: CHECK: FILE NUMBER: 16220-2-0



### NYLOPLAST DRAIN BASINS

THE ROOF DRAIN MANIFOLDS SHALL UTILIZE NYLOPLAST DRAIN BASINS OR APPROVED EQUAL. THE DRAIN BASINS AT THE REAR OF THE BUILDING ARE NOT INTENDED TO CAPTURE SURFACE RUNOFF SO THEY SHALL HAVE A SOLID COVER. TRAFFIC RATED TOPS ARE ONLY REQUIRED IN THE PAVED AREAS.

### CONSTRUCTION INSPECTION

INSPECTIONS ARE NEEDED DURING CONSTRUCTION TO ENSURE THAT THE FACILITIES ARE BUILT IN ACCORDANCE WITH THE APPROVED PLANS AND DESIGN SPECIFICATIONS. THE DEVELOPER/OWNER SHALL PROVIDE FOR PERIODIC INSPECTIONS OF THE FACILITY DURING CONSTRUCTION. DETAILED INSPECTION CHECKLISTS SHALL BE USED THAT INCLUDE SIGN-OFFS BY A LICENSED LAND SURVEYOR OR PROFESSIONAL ENGINEER REGISTERED IN VIRGINIA AT CRITICAL STAGES OF CONSTRUCTION, TO ENSURE THAT THE CONTRACTOR'S INTERPRETATION OF THE PLAN IS CONSISTENT WITH THE DESIGNER'S INTENT. THE ACTUAL INSPECTIONS MAY BE PERFORMED BY AN INDIVIDUAL UNDER THE DIRECT SUPERVISION OF THE LICENSED PROFESSIONAL.

### AS-BUILT CERTIFICATION

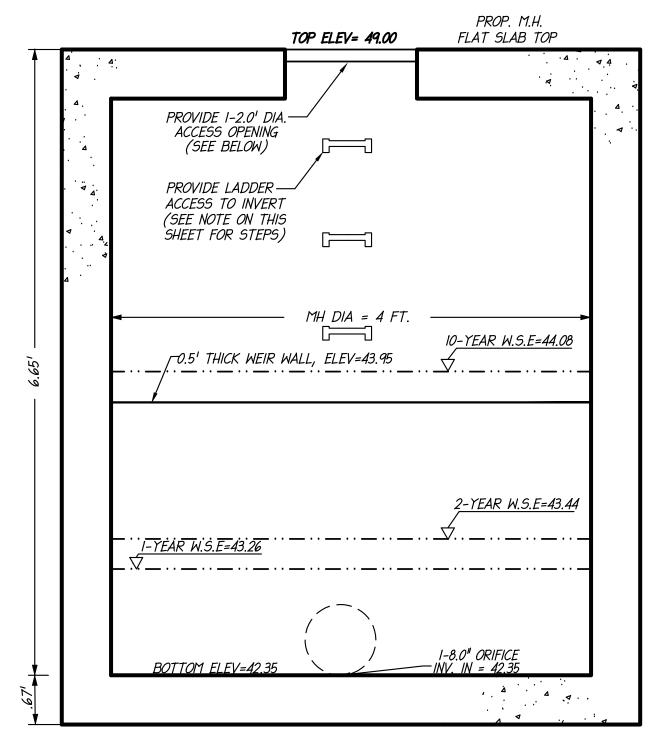
AFTER THE FACILITY HAS BEEN CONSTRUCTED, THE DEVELOPER SHALL HAVE AN AS-BUILT CERTIFICATION CONDUCTED BY A LICENSED LAND SURVEYOR OR PROFESSIONAL ENGINEER REGISTERED IN VIRGINIA AND SUBMITTED TO THE TOWN OF VIENNA ALONG WITH THE AS-BUILT CHECKLIST AND AS-BUILT PLAN. THE AS-BUILT CERTIFICATION VERIFIES THAT THE FACILITY WAS INSTALLED AS DESIGNED AND APPROVED. THE FOLLOWING COMPONENTS SHALL BE ADDRESSED IN THE AS-BUILT CERTIFICATION:

- A. ELEVATIONS PER PLAN
- B. PRETREATMENT STRUCTURES ARE PROPERLY INSTALLED (I.E. ISOLATOR ROW)
- C. OBSERVATION WELLS ARE INSTALLED AND WORKING EFFECTIVELY
- D. DIGITAL PHOTOGRAPHS DOCUMENTING CONSTRUCTION AND SHOWING
  THE SITE BEFORE CONSTRUCTION, THE EXCAVATION'S WALLS AND
  BOTTOM BEFORE ANY BACKFILL, PLACEMENT OF EACH MATERIAL
  LAYER, PLACEMENT OF UNDERDRAIN SYSTEM, OBSERVATION WELLS,
  AND OUTLET WORKS.
- E. GPS COORDINATES FOR THE FACILITY

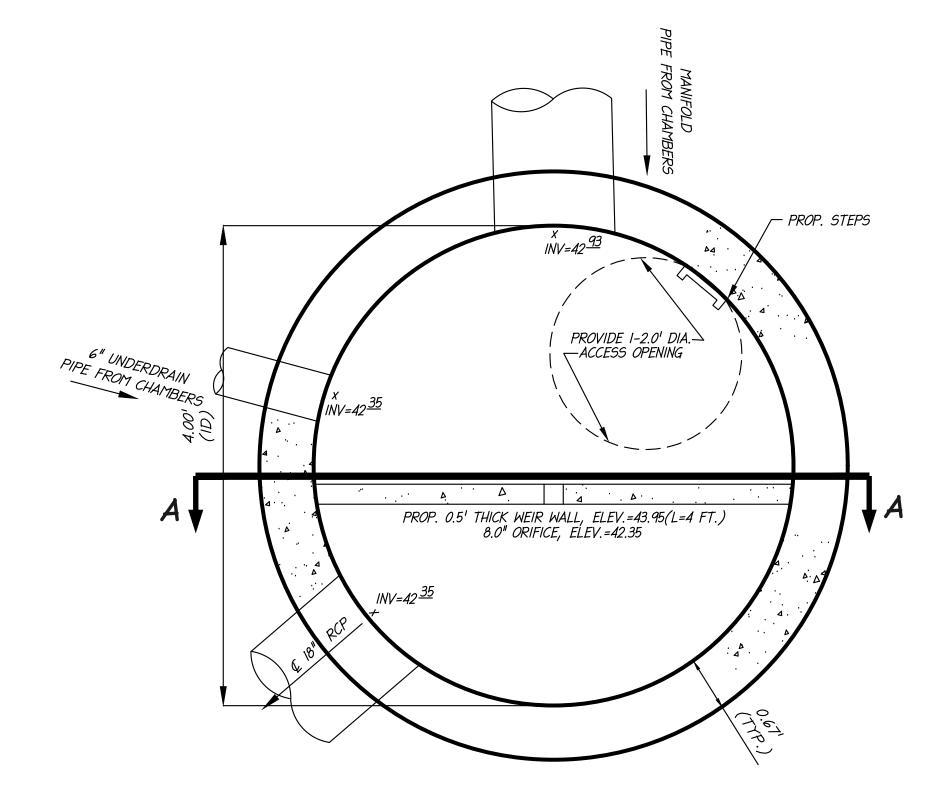
PROPOSED LAYOUT	PROPOSED ELEVATIONS					ABOVE BASE	OF CHAMBI
44 STORMTECH SC-310 CHAMBERS	MAXIMUM ALLOWABLE GRADE (TOP OF PAVEMENT/UNPAVED):	352.18	PART TYPE	ITEM ON LAYOUT	. DESCRIPTION	INVERT*	MAX FLOV
8 STORMTECH SC-310 END CAPS 6 STONE ABOVE (in)	MINIMUM ALLOWABLE GRADE (ÙNPAVED WITH TRAFFIC): MINIMUM ALLOWABLE GRADE (UNPAVED NO TRAFFIC):	346.18 345.68	PREFABRICATED END CAP	А	12" BOTTOM PREFABRICATED END CAP/TYP OF ALL 12" BOTTOM CONNECTIONS AND ISOLATOR ROWS	0.90"	
6 STONE BELOW (in) 40 % STONE VOID	MINIMUM ALLOWABLE GRADE (TOP OF RIGID CONCRETE PAVEMENT): MINIMUM ALLOWABLE GRADE (BASE OF FLEXIBLE PAVEMENT):	345.68 345.68	MANIFOLD		10" X 10" TOP, MOLDED FITTINGS	1.40"	
INSTALLED SYSTEM VOLUME (CF)	TOP OF STONE:	344.68	PIPE CONNECTION NYLOPLAST (INLET W/ ISO ROW)		12" BOTTOM CONNECTION 30" DIAMETER (24" SUMP MIN)	0.90"	1.3 CFS I
(COVER STONE INCLUDED) (COVER STONE INCLUDED)	TOP OF SC-310 CHAMBER: 10" x 10" TOP MANIFOLD INVERT:	344.18 342.97	NYLOPLAST (INLET W/ ISO ROW)		30" DIAMETER (24 SOME MIN) 30" DIAMETER (DESIGN BY ENGINEER)		2.0 CFS O
(BASE STONE INCLUDED) 294 SYSTEM AREA (SF)	12" ISOLATOR ROW INVERT: 12" BOTTOM CONNECTION INVERT:	342.93 342.93	UNDERDRAIN INSPECTION PORT		6" ADS N-12 DUAL WALL PERFORATED HDPE UNDERDRAIN 4" SEE DETAIL		
0.65 SYSTEM PERIMETER (ft)	BOTTOM OF SC-310 CHAMBER: UNDERDRAIN INVERT:	342.85 342.35	INSPECTION PORT	<u> </u>	14" SEE DETAIL		
	BOTTOM OF STONE:	342.35					
	B A D				E		

# STORMTECH CHAMBERS OUTLET CONTROL STRUCTURE STORM STR #4A

PROP. MANHOLE (MOD. MH-2, 4' DIA.)



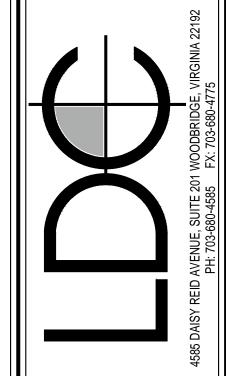
SECTION A-A



### <u>PLAN VIEW</u>

### <u>NOTES</u>

- I. STAIRS ARE TO BE PROVIDED WITHIN ALL ACCESS STRUCTURES. THESE STAIRS SHALL BE PROVIDED I' ON CENTER AND SHOULD BE LOCATED ALONG AN INTERIOR WALL AT THE DISCRETION OF THE MANUFACTURER.
- 2. VIRGINIA RUNOFF REDUCTION METHOD AND SCS METHODS WERE UTILIZED TO GENERATE THE HYDROLOGY AND THE HYDRAULICS FOR THE WEIR AND ORIFICE EQUATIONS AND PONDPACK USED FOR THE SWM ROUTINGS COMPUTATIONS.
- 3. A 6" DIA. PVC UNDERDRAIN SHALL BE INSTALLED AT BOTTOM OF STONE ELEVATION AS SHOWN ON THE ADS DETAILS.



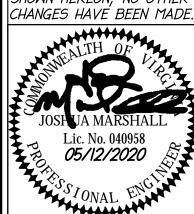
STORMTECH

IILL SIKEEI ELF-STORAGE

DATE DESIGN NO. DESCRIPTION REVIEW APPROVED DATE

REVISION APPROVED BY:

I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVE BEEN MADE.



SCALE: |" = 30'

SHEET 21 OF 25 DATE:

### **Site Information**

### Post-Development Project (Treatment Volume and Loads)

Enter Total Disturbed Area (acres) → 1.71

20%	Maximum reduction required:
0	The site's net increase in impervious cover (acres) is:
0.44	Post-Development TP Load Reduction for Site (lb/yr):

Linear Development Project?

### Check: BMP Design Specifications List: 2013 Draft Stds & Specs Linear project? No Land cover areas entered correctly? Total disturbed area entered?

calculation cells

final results

### Pre-ReDevelopment Land Cover (acres) A Soils **B** Soils C Soils D Soils

Forest/Open Space (acres) undisturbed,			0.00
protected forest/open space or reforested land		0.00	0.00
Managed Turf (acres) disturbed, graded for			0.00
yards or other turf to be mowed/managed		0.09	0.09
Impervious Cover (acres)		1.62	1.62
			1.71
			2

Post-Development Land Cover (acres)					
	A Soils	B Soils	C Soils	D Soils	Totals
Forest/Open Space (acres) undisturbed, protected forest/open space or reforested land				0.00	0.00
Managed Turf (acres) disturbed, graded for yards or other turf to be mowed/managed				0.26	0.26
Impervious Cover (acres)				1.45	1.45
Area Check	OK.	OK.	OK.	OK.	1.71

43
1.00
0.26
1.86
0.41
0.90

Runoff Coefficients	(Rv) A Soils	B Soils	C Soils	D Soils
			Cours	
Forest/Open Space	0.02	0.03	0.04	0.05
Managed Turf	0.15	0.20	0.22	0.25
Impervious Cover	0.95	0.95	0.95	0.95

Totals

Land Cover Summary-Pre						
Pre-ReDevelopment	Listed	Adjusted <sup>1</sup>				
Forest/Open Space Cover (acres)	0.00	0.00				
Weighted Rv(forest)	0.00	0.00				
% Forest	0%	0%				
Managed Turf Cover (acres)	0.09	0.09				
Weighted Rv(turf)	0.25	0.25				
% Managed Turf	5%	5%				
Impervious Cover (acres)	1.62	1.62				
Rv(impervious)	0.95	0.95				
% Impervious	95%	95%				
Total Site Area (acres)	1.71	1.71				
Site Pv	0.91	0.01				

Site Rv	0.91							
Treatment Volume and Nutrient Load								
Pre-ReDevelopment Treatment Volume (acre-ft)	0.1301	0.1301						
Pre-ReDevelopment Treatment Volume (cubic feet)	5,668	5,668						
Pre-ReDevelopment TP Load (lb/yr)	3.56	3.56						
Pre-ReDevelopment TP Load per acre (lb/acre/yr)	2.08							
Baseline TP Load (lb/yr)  (0.41 lbs/acre/yr applied to pre-redevelopmen pervious land proposed for new impervio	0.70							

<sup>1</sup> Adjusted Land Cover Summary:
Pre ReDevelopment land cover minus pervious land cover (forest/open space or
managed turf) acreage proposed for new impervious cover.

Adjusted total acreage is consistent with Post-ReDevelopment acreage (minus acreage of new impervious cover).

Column I shows load reduction requriement for new impervious cover (based on new development load limit, 0.41 lbs/acre/year).

<b>Land Cover Summary</b>	-Post (Final)	Land Cover Summ	ary-Post	Land Cover Summo	ary-Post		
Post ReDev. & New I	mpervious	Post-ReDevelop	ment	Post-Development New	Post-Development New Impervious		
Forest/Open Space Cover (acres)	0.00	Forest/Open Space Cover (acres)	0.00				
Weighted Rv(forest)	0.00	Weighted Rv(forest)	0.00				
% Forest	0%	% Forest	0%				
Managed Turf Cover (acres)	0.26	Managed Turf Cover (acres)	0.26				
Weighted Rv (turf)	0.25	Weighted Rv (turf)	0.25				
% Managed Turf	15%	% Managed Turf	15%				
mpervious Cover (acres)	1.45	ReDev. Impervious Cover (acres)	1.45	New Impervious Cover (acres)	0.00		
Rv(impervious)	0.95	Rv(impervious)	0.95	Rv(impervious)			
% Impervious	85%	% Impervious	85%				
Final Site Area (acres)	1.71	Total ReDev. Site Area (acres)	1.71				
Final Post Dev Site Rv	0.84	ReDev Site Rv	0.84				

		Treatment Volume and N	Nutrient Load		
Final Post-Development Treatment Volume (acre-ft)	0.1202	Post-ReDevelopment Treatment Volume (acre-ft)	0.1202	Post-Development Treatment Volume (acre-ft)	
Final Post-Development Treatment Volume (cubic feet)	5,236	Post-ReDevelopment Treatment Volume (cubic feet)	5,236	Post-Development Treatment Volume (cubic feet)	
Final Post- Development TP Load (lb/yr)	3.29	Post-ReDevelopment Load (TP) (lb/yr)*	3.29	Post-Development TP Load (lb/yr)	
Final Post-Development TP Load per acre (lb/acre/yr)	1.92	Post-ReDevelopment TP Load per acre (lb/acre/yr)	1.92		
		Max. Reduction Required (Below Pre-	20%		

ReDevelopment Load)			
TP Load Reduction Required for Redeveloped Area (lb/yr)	0.44	TP Load Reduction Required for New Impervious Area (lb/yr)	0

### **Post-Development Requirement for Site Area**

TD Load Dadvetion Descriped (lb // m)	0.44
TP Load Reduction Required (lb/yr)	0.44

### Nitrogen Loads (Informational Purposes Only)

Pre-ReDevelopment TN Load (lb/yr)	25.48	Final Post-Development TN Load (Post-ReDevelopment & New Impervious) (lb/yr)	23.54
<del></del>	20	•	

### nage Area A

### ige Area A Land Cover (acres)

	A Soils	B Soils	C Soils	D Soils	Totals	Land Cover Rv
Forest/Open Space (acres)					0.00	0.00
Managed Turf (acres)				0.00	0.00	0.00
Impervious Cover (acres)				0.72	0.72	0.95
		· · · · · · · · · · · · · · · · · · ·		Total	0.72	

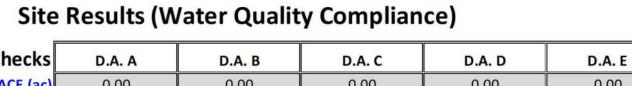
### CLEAR BMP AREAS

0.00

Total Phosphorus Available for Removal in D.A. A (lb/yr)	1.56
Post Development Treatment Volume in D.A. A (ft <sup>3</sup> )	2,483

nwater Best Management Practices (RR = Runoff Reduction)

Select from dropdown lists-													
Practice	Runoff Reduction Credit (%)	Managed Turf Credit Area (acres)	Impervious Cover Credit Area (acres)	Volume from Upstream Practice (ft <sup>3</sup> )	Runoff Reduction (ft <sup>3</sup> )	Remaining Runoff Volume (ft <sup>3</sup> )	Total BMP Treatment Volume (ft <sup>3</sup> )	Phosphorus Removal Efficiency (%)	Phosphorus Load from Upstream Practices (lb)	Untreated Phosphorus Load to Practice (lb)	Phosphorus Removed By Practice (lb)	Remaining Phosphorus Load (lb)	Downstream Practice to be Employed
nufactured Treatment Devices (	no RR)												
. Manufactured Treatment Device- Hydrodynamic	0			0	0	0	0	20	0.00	0.00	0.00	0.00	
nufactured Treatment Device-Filtering	0	0.00	0.72	0	0	2,483	2,483	40	0.00	1.56	0.62	0.93	
inufactured Treatment Device-Generic	0			0	0	0	0	20	0.00	0.00	0.00	0.00	



Area Checks	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	AREA CHECK
FOREST/OPEN SPACE (ac)	0.00	0.00	0.00	0.00	0.00	OK.
IMPERVIOUS COVER (ac)	0.72	0.00	0.00	0.00	0.00	OK.
IMPERVIOUS COVER TREATED (ac)	0.72	0.00	0.00	0.00	0.00	OK.
MANAGED TURF AREA (ac)	0.00	0.00	0.00	0.00	0.00	OK.
MANAGED TURF AREA TREATED (ac)	0.00	0.00	0.00	0.00	0.00	OK.
AREA CHECK	OK.	OK.	OK.	OK.	OK.	

Site Treatment Volume (ft<sup>3</sup>) 5,236

### Runoff Reduction Volume and TP By Drainage Area

	D.A. A	D.A. B	D.A. C	D.A. D	D.A. E	TOTAL
RUNOFF REDUCTION VOLUME ACHIEVED (ft <sup>3</sup> )	0	0	0	0	0	0
TP LOAD AVAILABLE FOR REMOVAL (lb/yr)	1.56	0.00	0.00	0.00	0.00	1.56
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.62	0.00	0.00	0.00	0.00	0.62
TP LOAD REMAINING (lb/yr)	0.94	0.00	0.00	0.00	0.00	0.94

0.00

### NITROGEN LOAD REDUCTION ACHIEVED (lb/yr) 0.00

### **Total Phosphorus**

FINAL POST-DEVELOPMENT TP LOAD (lb/yr)	3.29
TP LOAD REDUCTION REQUIRED (lb/yr)	
TP LOAD REDUCTION ACHIEVED (lb/yr)	0.62
TP LOAD REMAINING (lb/yr):	2.67

REMAINING TP LOAD REDUCTION REQUIRED (lb/yr): 0.00 \*\* \*\* TARGET TP REDUCTION EXCEEDED BY 0.18 LB/YEAR \*\*

## Total Nitrogen (For Information Purposes)

POST-DEVELOPMENT LOAD (lb/yr)	23.54
NITROGEN LOAD REDUCTION ACHIEVED (lb/yr)	0.00
REMAINING POST-DEVELOPMENT NITROGEN LOAD (lb/yr)	23.54

P0000,		
AD (lb/yr)	23.54	
D (lb/yr)	0.00	
1 D (III /)	22.54	

# BMP NARRATIVE

THE SITE IS BEING ANALYZED AS RE-DEVELOPMENT FOR WATER QUALITY PURPOSES. THE PRE-DEVELOPED LAND COVER OF THE SITE IS PREDOMINANTLY IMPERVIOUS WITH AN EXISTING BUILDING AND PARKING LOT. THE POST-DEVELOPED CONDITION PROPOSES THE REMOVAL AND REPLACEMENT OF THE EXISTING BUILDING WITH ONE OF A LARGER FOOTPRINT AND THE REMOVAL OF A PORTION OF ONSITE AND OFFSITE PAVEMENT.

WATER QUALITY REQUIREMENTS ARE SATISFIED THROUGH THE REDUCTION IN ONSITE IMPERVIOUS AREA AND TREATMENT OF THE ROOF AREA THAT IS DIRECTED TO THE ISOLATOR ROW IN THE UNDERGROUND SWM FACILITY. PLEASE SEE VRRM CALCULATIONS ON THIS SHEET.

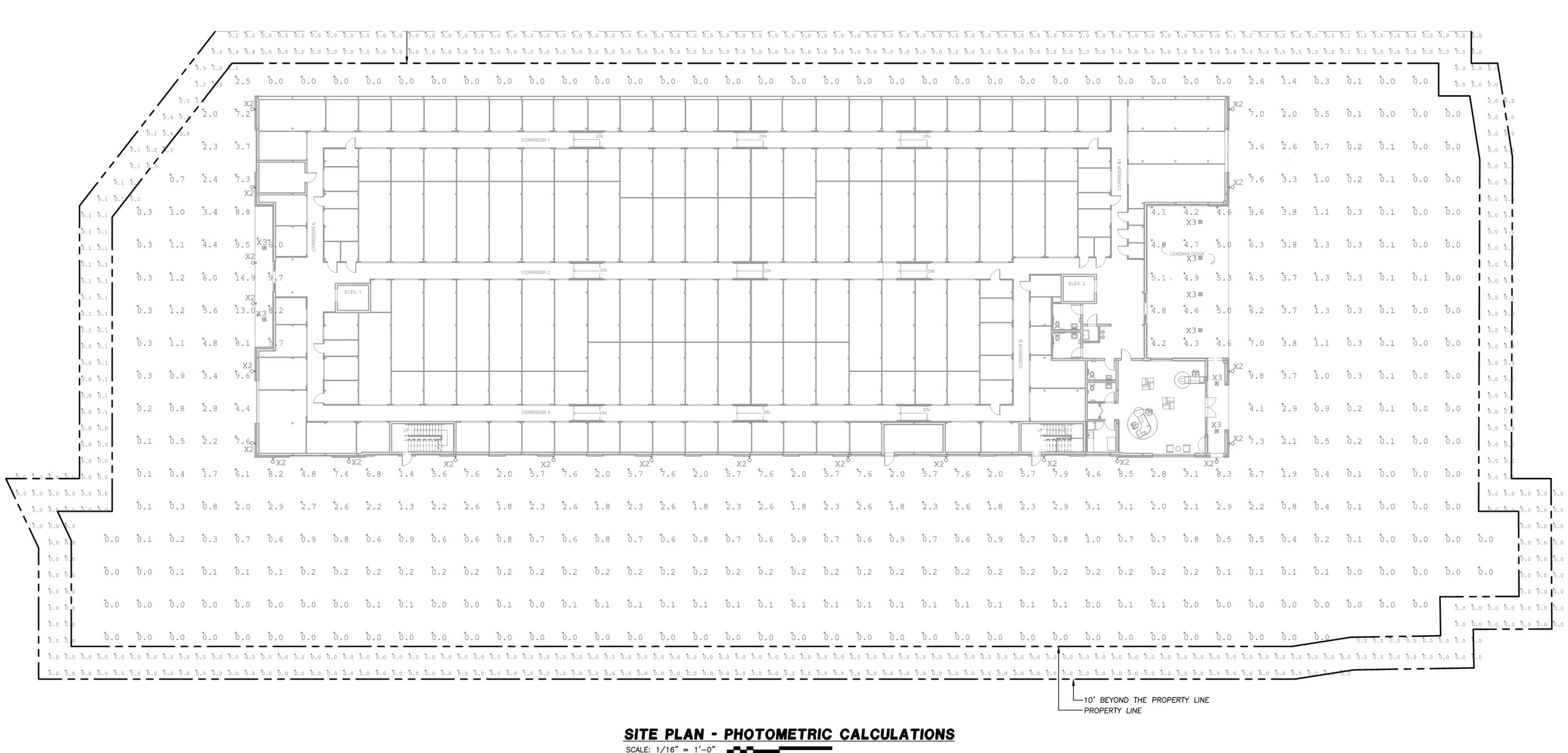
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I HEREBY CERTIFY THAT OTHER THAN THE REVISIONS SHOWN HEREON, NO OTHER CHANGES HAVÉ BEEN MADE.



AS NOTED

SHEET 22 OF 25

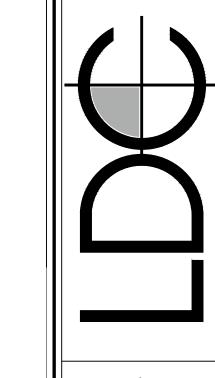


# SCALE: 1/16" = 1'-0" 0 4' 8' 16' 32'

Luminaire Schedule								
Symbol	Qty	Label	Arrangement	LLF	Description	Lum. Watts	Lum. Lumens	
·O	21	X2	SINGLE	0.900	LIGMAN TA-31861-T4-W40	37.8	2777	
•	6	Х3	SINGLE	0.900	DECO D533-LED-40-50-U-PRO	42	4830	

Calculation Summary						
Label	CalcType	Units	Avg	Max	Min	Max/Min
CUBE SMART SITE	Illuminance	Fc	1.58	14.9	0.0	N.A.
10 FOOT BEYOND PROPERTY LINE	Illuminance	Fc	0.04	2.0	0.0	N.A.

P:\PY 2016\16220-2-0 Mill Street, N.E. - 215-241\ENG\SITEPLAN



OTOMETRIC PLAI

ILL STREET

4TE DESIGN NO. DESCRIPTION REVIEW APPROVED DATE BY:

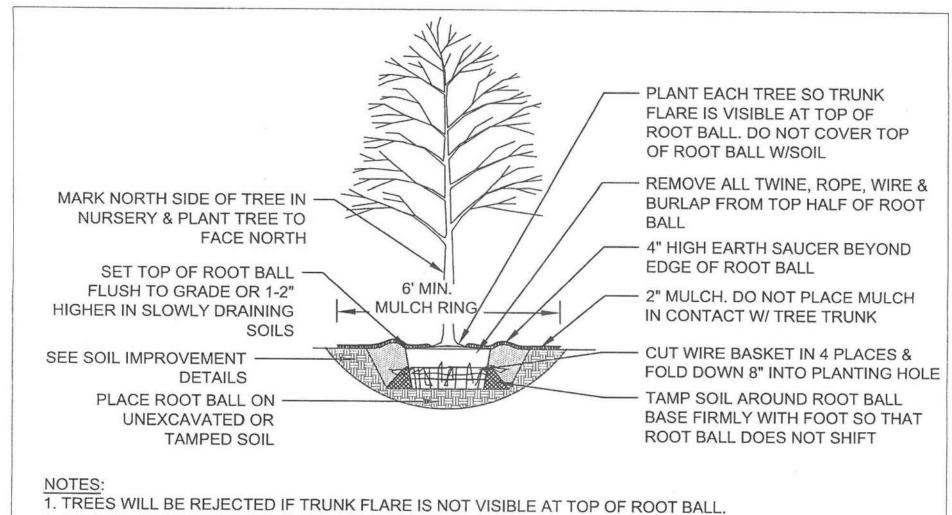
I HEREBY CERTIFY THAT
OTHER THAN THE REVISIONS
SHOWN HEREON, NO OTHER
CHANGES HAVE BEEN MADE.

JOSHUA MARSHALL
Lic. No. 040958

SCALE: AS SHOWN

SHEET 23 OF 25

- 2. ALL PLANT MATERIAL SHALL CONFORM THE <u>AMERICAN STANDARD FOR NURSERY,</u> LATEST EDITION, PUBLISHED BY THE AMERICAN NURSERY AND LANDSCAPE ASSOCIATION. ALL PLANTS MUST BE FREE FROM INJURY, INSECT INFESTATIONS AND DISEASE.
- 3. TREES SHALL BE HEALTHY AND VIGOROUS, AND MEET ALL ANSI STANDARDS. ALL NEWLY INSTALLED TREES THAT ARE REQUIRED BY THE TREE CONSERVATION PLAN, WHICH IN THE OPINION OF THE TOWN ARBORIST ARE DEAD OR ARE NOT HEALTHY, SHALL BE REPLACED BY THE CONTRACTOR.
- 4. ALL PLANT MATERIAL MUST BEAR ORIGINAL NURSERY TAGS INDICATING THE GENUS, SPECIES AND IF APPLICABLE, CULTIVARS AND VARIETY. ALL TAGS SHALL BE REMOVED AFTER THE SITE INSPECTOR HAS INSPECTED THE PLANT MATERIAL.
- 5. TEST SOIL DRAINAGE BEFORE PLANTING. DIG A HOLE AS DEEP AS YOUR PLANTING HOLE AND FILL WITH WATER. IF WATER DRAINS AT A RATE LESS THAN ONE INCH PER HOUR, INSTALL DRAINAGE TO CARRY WATER AWAY FROM THE PLANTING HOLE BASE, OR MOVING OR RAISING THE PLANTING SITE.
- 6. TREE PITS SHALL BE A MINIMUM OF TWO (2) AND A HALF (1/2) TIMES THE WIDTH OF THE ROOT BALL AND NO DEEPER THAN THE HEIGHT OF THE ROOT BALL. ON BALLED AND BURLAPED TREES, REMOVE PINNING NAILS OR ROPE LACING, THEN CUT AWAY THE WRAPPING AND THEN BACKFILL. REMOVE THE TOP 12" OF THE WIRE BASKET. REMOVE ALL ROPE, WHETHER JUTE OR NYLON, FROM TRUNKS. FOR CONTAINER MATERIALS, REMOVE THE CONTAINER COMPLETELY. IF ROOTS ARE CIRCLING AROUND THE ROOT BALL EXTERIOR OF CONTAINER PLANTS (TREES, SHRUBS OR PERENNIALS) CUT THROUGH THE ROOTS AND SOIL IN A FEW PLACES. CONTAINER TREE WITH MULTIPLE CIRCLING ROOTS WILL BE REJECTED. PLACE SHRUBS AND PERENNIALS AT THE SAME DEPTH THEY WERE IN THE CONTAINERS.
- 7. WHEN HALF OF THE BACKFILL HAS BEEN RETURNED TO THE PLANTING HOLE, WATER SHALL BE APPLIED TO PROVIDE SETTLEMENT AND ELIMINATE AIR POCKETS. THE TREE SHALL BE THOROUGHLY WATERED AGAIN AFTER THE REMAINING SOIL HAS BEEN PLACED IN THE PLANTING PIT. A THREE (3) TO FOUR (4) INCH DAM OF SOIL SHALL BE CONSTRUCTED AROUND THE PLANTING PIT.
- 8. TWO (2) TO THREE (3) INCHES OF MULCH SHALL BE PLACED OVER THE TREE-PLANTING PIT, BUT SHALL BE KEPT THREE (3) TO FOUR (4) INCHES AWAY FROM THE TRUNK OF THE TREE OR CROWNS OF SHRUBS. DO NOT ALLOW MULCH TO TOUCH THE TRUNKS OF TREES OR CROWNS OF SHRUBS. USE MULCH THAT IS COMPATIBLE WITH THE TYPE OF PLANT USED. IN MULCHING PERENNIALS, USE NO MORE THAN 1-2".
- 9. TREES SHALL BE PLANTED AT THE HEIGHT OF THE SURROUNDING GRADE WITH ROOT FLARES VISIBLE. SHOULD SOIL HAVE BEEN PILED OVER THE ROOT FLARE DURING THE DIGGING PROCESS, THIS SOIL SHALL BE REMOVED SO THAT THE FLARE IS SLIGHTLY ABOVE GRADE.
- IO. PRUNING AT THE TIME OF PLANTING SHALL BE DONE ONLY TO REMOVE BROKEN BRANCHES LEADERS.
- II. REMOVE TAGS AND LABELS FROM TREES AND SHRUBS TO PREVENT GIRDLING BRANCHES AND TRUNKS.
- 12. STAKES SHALL BE USED ONLY IN AREA OF HIGH TRAFFIC OR HIGHLY WINDY LOCATIONS. A TREE-STAKING DIAGRAM SHOULD BE PROVIDED IF STAKING IS NECESSARY. STAKE FOR MAXIMUM OF ONE YEAR. ALLOW TREES A SLIGHT AMOUNT OF FLEX RATHER THAN HOLDING THEM RIGIDLY IN PLACE. USE GUYING OR ATTACHING THAT WON'T DAMAGE THE BARK. TO PREVENT TRUNK GIRDLING, REMOVE ALL GUYING MATERIAL AFTER ONE YEAR.
- 13. IF GUYING WILL BE CARRIED OUT, THEN THE MATERIAL SHALL ONLY BE A WIDE FABRIC TAPE SUCH AS ARBORTIE OR EQUIVALENT, INSTALLED PER MANUFACTURES INSTRUCTIONS.
- 14. PLANTING SEASON PLANTING SHALL BE DONE ONLY WITHIN THE FOLLOWING DATES: A. DECIDUOUS TREES - MARCH IS TO MAY 30 OR SEPTEMBER IS TO DECEMBER IS (OAKS AND BLACK GUM TO BE SPRING DUG AND PLANTED ONLY). B. EVERGREEN TREES - MARCH I TO MAY IS OR SEPTEMBER IS TO NOVEMBER IS.
- 15. ALL PLANT MATERIAL SHALL BE GUARANTEED BY THE CONTRACTOR FOR ONE YEAR FROM THE DATE OF ACCEPTANCE TO BE IN GOOD, HEALTHY AND FLOURISHING CONDITION. IN THE EVENT THAT A PLANT DIES OR IN THE JUDGMENT OF THE TOWN ARBORIST, FAILS TO FLOURISH; THE CONTRACTOR SHALL REPLACE IN ACCORDANCE WITH THE ABOVE NOTED SPECIFICATIONS.
- 16. THE CONTRACTOR SHALL BE RESPONSIBLE FOR THE MAINTENANCE OF THE PLANTS DURING THIS ONE-YEAR WARRANTY PERIOD. THIS MAINTENANCE SHALL INCLUDE PROVIDING WATER ON A WEEKLY BASIS WHEN NATURAL RAINFALL IS LESS THAN ONE INCH A WEEK. DRIP IRRIGATION SYSTEMS AND WATER RESERVOIR DEVICES CAN FACILITATE WATERING. ROOT BALLS OF TREES SHOULD BE SLOWLY AND THOROUGHLY SOAKED AT TIME OF WATERING. FOR PLANTING BEDS (I.E., TREES, SHRUBS AND PERENNIALS), WATER SLOWLY AND DEEPLY PUTTING DOWN I"-2" OF WATER IN A 6-12 HOUR PERIOD. THIS SHOULD GIVE A PENETRATION OF 12-18" DEPTH.
- 17. PLANT MATERIAL SHALL NOT BE STORED ON SITE FOR A PERIOD LONGER THAN 3 DAYS FROM THE TIME OF DELIVERY.
- 18. ALL PLANT MATERIAL SHALL BE PROTECTED FROM DRYING DURING TRANSPORTATION AND DURING STORAGE ON SITE. ANY PLANT THAT IS NOT PLANTED ON THE DAY OF DELIVERY WILL BE PLACED IN A HOLDING AREA. THE TREE OR SHRUB WILL BE STORED VERTICALLY AND ITS ROOTS WILL BE COVERED WITH A MOISTURE HOLDING MEDIUM (WOOD CHIPS, SAW-DUST, ECT.) UNTIL PLANTED.
- 19. THE CONTRACTOR SHALL BE RESPONSIBLE FOR ANY DISCREPANCIES BETWEEN THE PLANT LIST AND THE PLANTING



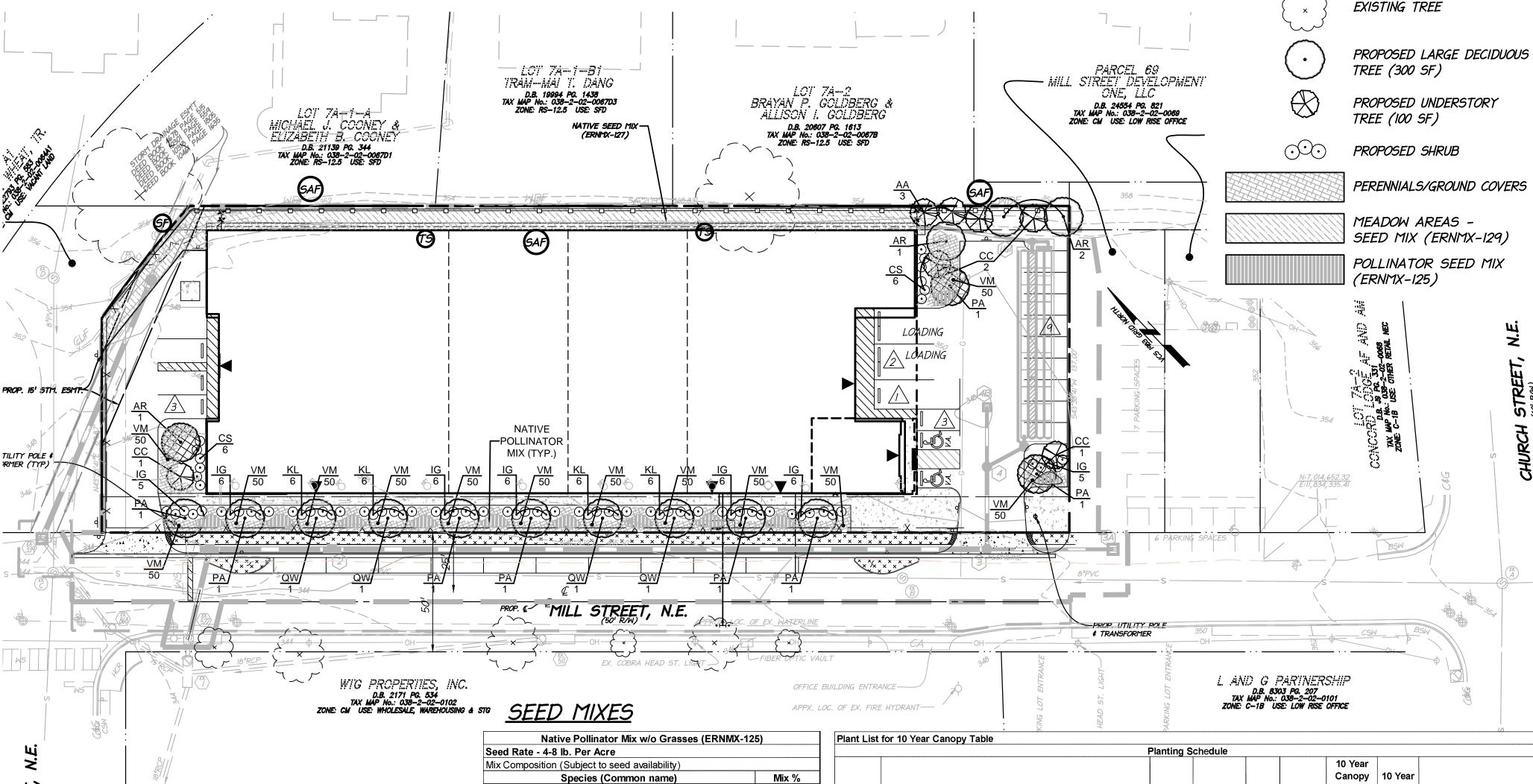
- 2. DO NOT HEAVILY PRUNE TREE AT PLANTING. PRUNE ONLY CROSSING LIMBS, CO-DOMINANT LEADERS & BROKEN OR DEAD BRANCHES. SOME INTERIOR TWIGS & LATERAL BRANCHES MAY BE PRUNED, BUT DO NOT REMOVE TERMINAL BUDS OF BRANCHES THAT EXTEND TO THE EDGE OF CROWN.
- 3. STAKE OR WRAP TREE ONLY UPON APPROVAL OF LANDSCAPE ARCHITECT.
- 4. THIS DETAIL ASSUMES THAT PLANTING SPACE IS LARGER THAN 8 SQUARE FEET, OPEN TO SKY, AND NOT COVERED BY PAVING OR GRATING.
- 5. THIS PLANTING DETAIL WAS BASED ON ISA (INTERNATIONAL SOCIETY OF ARBORICULTURE) STANDARD TREE PLANTING DETAIL. FOR MORE INFORMATION SEE www.isa-arbor.com OR CALL (217) 355-9411.

TREE PLANTING DETAIL B&B TREES IN ALL SOIL TYPES

### LEGEND:

- LIMITS OF CLEARING AND

GRADING



15.0%

12.0%

12.0%

10.0%

8.0%

6.0%

6.0%

6.0%

5.0%

4.0%

3.5%

2.4%

1.6%

30.0%

30.0%

20.0%

10.0%

10.0%

Total 100.0%

### FRONT YARD LANDSCAPING TABULATION:

TOTAL FRONT YARD AREA: 6,484 SF (0.15 AC) TOTAL LANDSCAPED AREA IN FRONT YARD: 2,978 SF (0.07 AC)

PERCENTAGE OF FRONT YARD LANDSCAPED:

TREE CANOPY COVER	REQUIR	<u>REMENTS</u>
GROSS SITE AREA DEDUCTION OF R.O.W. DEDICATION ADJUSTED SITE AREA	±0 SF	(1.26 AC) (0.00 AC) (1.26 AC)
ZONING:		CM
TREE CANOPY COVER REQUIRED (54,716 S EXISTING TREE CANOPY CREDIT (0 SF X I TREE CANOPY COVER PROVIDED:		5,472 SF 0 SF
LANDSCAPE TO BE PROVIDED  TREE SAVE AREA		5,500 SF 0 SF
	TOTAL	5,500 SF

### Aster umbellatus (Flat Topped White Aster) 1.5% Eupatorium perfoliatum (Boneset) 1.5% Monarda fistulosa (Wild Bergamot) 1.5% Geum canadense(White Avens) 1.0% 1.0% Tradescantia virginiana (Virginia Spiderwort) Total 100.0% 2,978/6,484 = 46% Conservation Shade Mix (ERNMX-129) Seed Rate - 4 lb. Per 1,000 Square Feet Mix Composition (Subject to seed availability) Species (Common name)

estuca rubra (Creeping Red Fescue)

Lolium multiflorum (Annual Ryegrass)

Poa trivialis (Rough Bluegrass)

Echinacea purpurea (Purple Coneflower)

Chamaecrista fasciculata (Partridge Pea)

Asclepias tuberosa (Butterfly Milkweed)

Heliopsis helianthoides (Oxeye Sunflower)

Tradescantia virginiana (Virginia Spiderwort)

Penstemon digitalis (Tall White Beardtongue)

Pycnanthemum tenuifolium (Narrowleaf Mountainmint)

Aster novae-angliae (New England Aster)

Solidago nemoralis (Gray Goldenrod)

Rudbeckia hirta, (Blackeyed Susan)

Verbena hastata, (Blue Vervain)

Zizia aurea(Golden Alexanders)

Senna hebecarpa (Wild Senna)

Coreopsis lanceolata (Lanceleaf Coreopsis)

# SEEDING NARRATIVE

Festuca rubra ssp. commutata (Chewings Fescue)

Poa pratensis, 'Kelly' (Kentucky Bluegrass, 'Kelly')

- I. SEEDING SHALL BE DONE IN THE DESIGNATED AREAS AS NOTED ON THIS PLAN.
- 2. BEFORE SEEDING, REMOVE ALL DEAD VEGETATION FROM THE
- 3. SEEDING SHALL BE DONE BY HAND, BROADCAST SPREADER OR
- HYDROSEEDING.
- 4. THE LANDSCAPE CONTRACTOR SHALL FOLLOW THE SEED DISTRIBUTOR'S RECOMMENDED TIME FOR SEEDING.
- 5. NO APPLICATION OF FERTILIZER SHALL BE USED DURING THE
- ESTABLISHMENT OF THE SEED MIXTURE. 6. SEEDING RATE OF ERNMX-129 IS 4 LB./ 1,000 S.F. A COVER SEED OF GRAIN OATS IS RECOMMENDED FROM JAN. I - AUG. I , AT A RATE OF 30LB./ I AC. AND A COVER SEED OF GRAIN RYE IS RECOMMENDED FROM AUG. I - DEC. 31, AT A RATE OF 30LB./ I
- 7. MONITORING OF WEEDS AND EVASIVE PLANT MATERIAL SHALL BE DONE SEASONALLY.

		Planting S	ichedule					
Cumbal	Species (Common name)	Occaptitus	Planting	<b>T</b>	Specina	10 Year Canopy Coverage	10 Year Credit	Comments
Symbol	Species (Common name)	Quantity	Size	Туре	Spacing	(sq. ft.)	total	Comments
Overstory AR	Acer rubrum (Red maple)	4	2-2.5" Cal.	B&B	As Shown	300	1200	Full, single stem
PA	Platanus × acerifolia (London planetree)	8	2-2.5" Cal.		As Shown	300		Full, single stem
QW	Quercus phellos (Willow Oak)	4	2-2.5" Cal.		As Shown	300		Full, single stem
Understo	ry Trees							
AA	Amelanchier arborea (Downey serviceberry)	3	2-2.5" Cal.	B&B	As Shown	100	300	Full to ground, single stem
CC	Cercis canadensis (Redbud)	4	2-2.5" Cal.	B&B	As Shown	100	400	Full to ground, single stem
Shrubs								
CS	Cornus stolonifera 'Arctic Fire' ('Arctic Fire' Red-Osier dogwood)	12	18-24" Ht.	cont	As Shown	n/a		Full to ground
IG	llex glabra (Inkberry)	40	18-24" Ht.	cont	As Shown	n/a		Full to ground
KL	Kalmia latifolia 'Elf' (Dwarf Mountain Laurel)	24	18-24" Ht.	cont	As Shown	n/a		Full to ground
Perennial	S							
VM	Periwinkle(Vinca minor)	650	2" PP	cont	12" O.C.	n/a		Full to ground
	Totals(Sq. Ft.)					Sub-total	5500	

# NOTE:

NO TREES BEYOND R.O.W. DUE TO CONFLICT WITH PIPE

# TREE PRESERVATION NOTE:

NO TREES ARE BEING PRESERVED ONSITE. THE EXISTING VEGETATION CONSISTS OF BAMBOO ALONG THE REAR PROPERTY LINE.

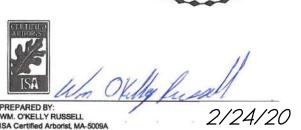
### SIGHT DISTANCE NOTE:

STREET TREES WILL HAVE THEIR LOWER BRANCHES LIMBED TO A HEIGHT OF 5.5' TO MAINTAIN A CLEAR SIGHT DISTANCE

# ZONING WAIVER & MODIFICATION

SEE ZONING ORDINANCE WAIVER & MODIFICATION LETTER ON SHEET 2 FOR THE MASONRY SCREEN WALL AND TREE CANOPY COVER.





GRAPHIC SCALE:



Q

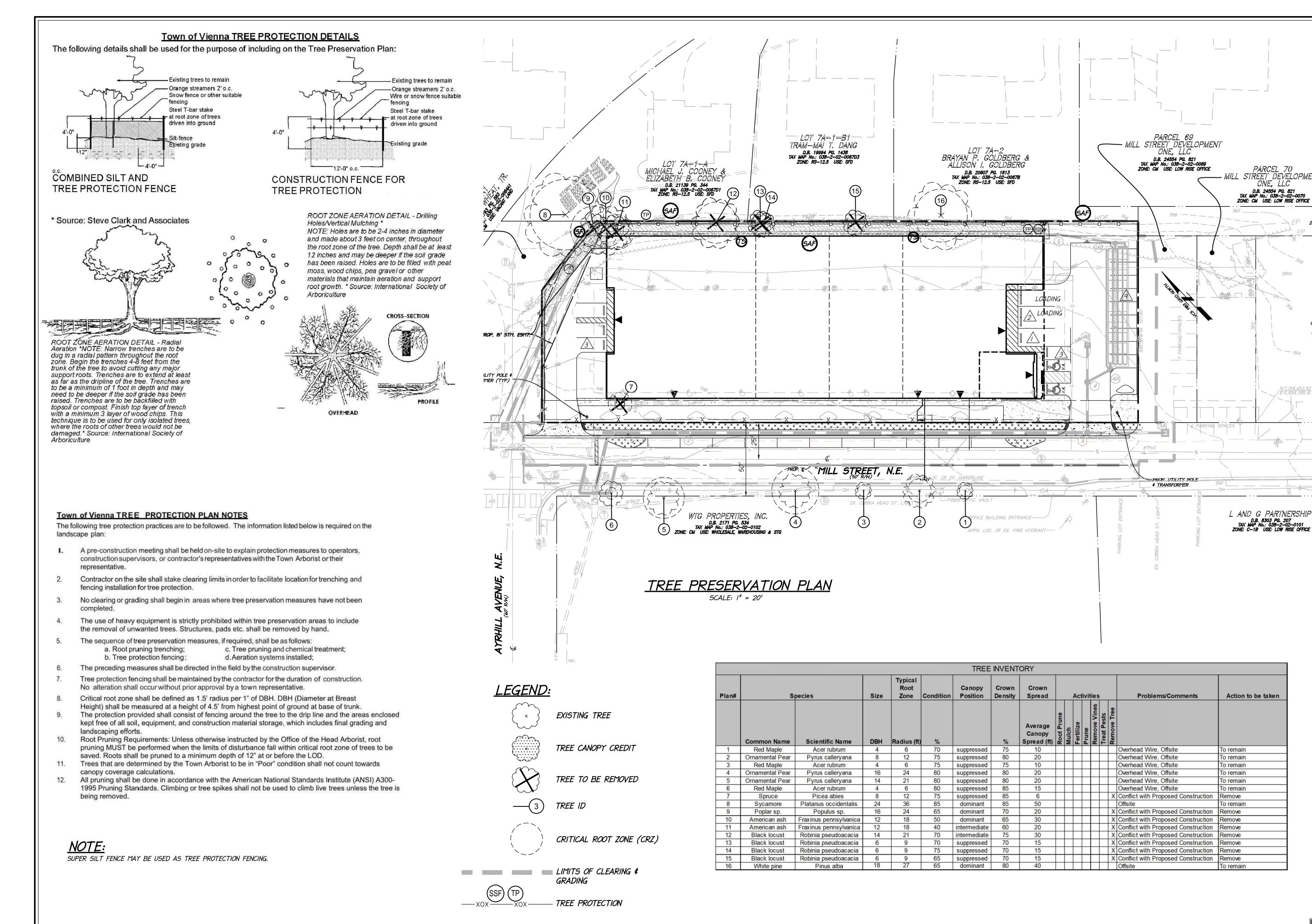
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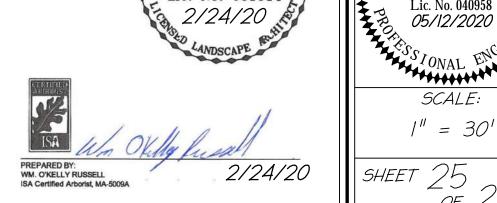
SCALE: I'' = 30'

SHEET 24 of 25



AREA TO BE MULCHED





20' ALLEY

AUGUST 2, 2019 DRAFT: | CHECK: PDRCBS FILE NUMBER: 16220-2-0

RE

I HEREBY CERTIFY THAT

OTHER THAN THE REVISIONS

SHOWN HEREON, NO OTHER

CHANGES HAVE BEEN MADE.

Lic. No. 040958

05/12/2020

SCALE:

of 25

SHEET 25